

AGENDA

CALL TO ORDER

Pledge Allegiance to the Flag (ES) Moment of Silence

ADOPTION OF AGENDA

APPROVAL OF THE MINUTES

1. Approval of the Minutes of the January 14, 2025 Town Council meeting, the January 14, 2025 Closed Session (Sealed), and the January 27, 2025 Work Session.

AWARDS AND RECOGNITION

2. Proclamation for Carolina Sporting Arms

BOARD UPDATES

- 3. Downtown Merchants (Sara Longstreet)
- 4. Pineville Library (Cameron Smith)

CONSENT AGENDA

5. Resolution 2025-02 in Support of HB 24 to Restore Authority to Local Government to Initiate Downzoning

PUBLIC COMMENT

PUBLIC HEARING

- 6. Dorman Road Development (Travis Morgan)
- 7. Industrial Warehouse (Travis Morgan)

OLD BUSINESS

Town Council - February 11, 2025

NEW BUSINESS

- 8. Dorman Road Development (Travis Morgan) ACTION ITEM
- 9. Industrial Warehouse (Travis Morgan) ACTION ITEM
- 10. Pay Plan Policies (Linda Gaddy) ACTION ITEM
- 11. Award for Purchase of Underground Cables (David Lucore) ACTION ITEM

MANAGER'S REPORT

MONTHLY STAFF REPORTS

12. Public Works

HR

PCS

Parks & Rec

PD

Planning & Zoning

CALENDARS FOR COUNCIL

13. March

CLOSED SESSION

ADJOURN

If you require any type of reasonable accommodation as a result of physical, sensory, or mental disability in order to participate in this meeting, please contact Lisa Snyder, Clerk of Council, at 704-889-2291 or lsnyder@pinevillenc.gov. Three days' notice is required.



TOWN COUNCIL REGULAR MEETING TOWN HALL COUNCIL CHAMBERS TUESDAY, JANUARY 14, 2025, AT 6:30 PM

MINUTES

CALL TO ORDER

Mayor David Phillips called the meeting to order @ 6:30 pm.

Mayor: David Phillips Mayor Pro Tem: Ed Samaha Council Members: Amelia Stinson-Wesley, Chris McDonough, Danielle Moore Town Manager: Ryan Spitzer Town Clerk: Lisa Snyder Town Attorney: Janelle Lyons

PLEDGE ALLEGIANCE TO THE FLAG

Mayor David Phillips led everyone in the Pledge of Allegiance.

MOMENT OF SILENCE

Mayor David Phillips asked for a moment of silence for three life-long Pineville residents who passed away in the past month, Glenn Stewart, Bobby Howington, and Bill Phillips, and keep them in your prayers and thoughts, and also our first responders, police officers and firefighters, as well.

ADOPTION OF AGENDA

Mayor Phillips announced that we need to add one item to the Agenda which is a Closed Session regarding a personnel matter pursuant to NCGS 143-318-11 (6). Mayor Pro Tem Samaha made a motion to adopt the agenda with said adjustment and a second was made by Council Member Moore. All ayes. (*Approved 4-0*).

APPROVAL OF MINUTES

The Minutes of the Closed Session on November 25, 2024, and Town Council meeting on December 10, 2024, were submitted for approval. Mayor Pro Tem Samaha moved to approve the minutes as presented with a second made by Council Member Stinson-Wesley. All Ayes. (*Approved 4-0*)

AWARDS AND RECOGNITION

Sergeant Corey Copley introduced our newest Police Officer, Tyler Griswold, and his parents. He comes to us from Flint, Michigan. New Police Officer Griswold was sworn in by Mayor Phillips.

BOARD REPORT

There were no Board Reports.

Town Council Regular Meeting - January 14, 2025

There were no Consent Agenda items.	CONSENT AGENDA
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There were no public comments.	
	PUBLIC HEARING
There was no Public Hearing.	
	OLD BUSINESS

There was no old business.

NEW BUSINESS

Board Appointments and Reappointments (Lisa Snyder). Town Clerk Snyder shared the list of board appointments that need reviewed and approval. A new application was received from Pineville resident, Dusty Gilvin, to the Board of Adjustment opening. Pineville resident, Nick Gallo, applied for the newly created board, Public Transit Advisory Committee.

Mayoral appointments include Centralina (COG) (Amelia Stinson-Wesley), CRTPO (Amelia Stinson-Wesley), Electricities (Ryan Spitzer with David Phillips as the alternate), MTC (David Phillips), Municipal Education Advisory (Ed Samaha), and Public Transit Advisory (Nick Gallo)

Planning Board member Dusty Gilvin was reappointed until January 1, 2027.

Board of Adjustment (Jim Knowles, Thomas White, Justin Musick) were reappointed until January 1, 2027. Dusty Gilvin was approved as the newest BOA member with his term expiring January 1, 2028.

There were no changes to the Telephone Board members or terms.

Council Member Stinson-Wesley would like to have an Airport Roundtable representative attend a future meeting to provide any updates pertaining to the airport and allow residents to express any concerns to them. In addition, she asked about the booklet that Town Clerk Snyder created that outlines the requirements of all our boards/committees. Town Clerk Snyder replied that the booklets will be available at all council meetings and in the lobby of Town Hall for anyone who would like a copy.

Pro Tem Samaha moved to approve the reappointments and to approve the applications of Dusty Gilvin and Nick Gallo with a second provided by Council Member Moore. (*Approved 4-0*)

Employee Handbook Update (Linda Gaddy). HR Director, Linda Gaddy, advised that a new state statute (NCGS 143-805) was adopted that requires local governments to add new definitions and restrictions on the use of pornography on government networks. A review and adoption of the revised Personnel Policies Handbook is needed to comply with the new law requirements. Ms. Gaddy will discuss the update with all employees and obtain signed handbook acknowledgments. Council Member Moore moved to approve the Employee Handbook update with a second made by Mayor Pro Tem Samaha. All ayes. (*Approved 4-0*)

Resolution 2025-01 Appointing Town Attorney (Ryan Spitzer). Town Manager Spitzer advised that each year, as part of the Town Charter, Council is required to appoint a Town Attorney. Council Member McDonough moved to approve Resolution 2025-01 with Janelle Lyons serving as our Town Attorney, for one year, followed by Council Member Moore providing a second. All ayes. (*Approved 4-0*)

Item 1.

MANAGER'S REPORT

Town Manager Spitzer gave a brief update on the McCullough bridge. The engineers approved the redesign so they won't have to remove the piers. Johnston Road is working on the underground on phase II and they are currently reviewing detours for the traffic in this area while additional work is being done in that area.

CLOSED SESSION

Mayor Pro Tem Samaha made a motion to go into Closed Session followed by a second made by Council Member Stinson-Wesley. Pursuant to NCGS 143-318.11 (6), a personnel matter.

Council Member Stinson-Wesley made a motion to leave Closed Session with a second provided by Council Member Moore. All ayes.

ADJOURNMENT

Mayor Pro Tem Samaha made a motion to adjourn followed by a second made by Council Member McDonough. All ayes. The meeting was adjourned at 7:20 pm.

David Phillips, Mayor

ATTEST:

Lisa Snyder, Town Clerk



WORK SESSION MINUTES MONDAY, JANUARY 27, 2025 @ 6:00 PM TOWN HALL COUNCIL CHAMBERS

The Town Council of the Town of Pineville, NC, met in a Work Session on Monday, January 27, 2025 @ 6:02 p.m.

ATTENDANCE

Mayor: David Phillips Mayor Pro Tem: Ed Samaha Council Members: Amelia Stinson-Wesley, Chris McDonough, Danielle Moore Town Manager: Ryan Spitzer Town Clerk: Lisa Snyder Finance Director: Chris Tucker Planning Director: Travis Morgan HR Director: Linda Gaddy

CALL TO ORDER.

Mayor David Phillips called the meeting to order at 6:02 p.m.

DISCUSSION ITEMS:

Text Amendment for Owner-occupied homes (Travis Morgan). Mr. Morgan reviewed the text amendment to the Pineville Ordinance for owner-occupied homes. He added that Michael Stumpf is requesting Council's consideration for a text amendment to revise the Ordinance owner-occupied restrictions for secondary dwelling units (also known as mother-in-law suites or accessory dwelling units). The Planning Board expressed concern over having both primary and accessory dwellings being for rent and recommended that Council also discuss this with legal counsel about possible options and legal compliance that was available.

Mr. Morgan noted that we received the town attorney's letter late today, so the Planning Board has not seen it yet. It looks like it would not be in the Town's best interest to pursue this. Town Manager Spitzer advised that Council can rule on the information in the text and that the Planning Board deferred legal counsel to Council. They did, however, recommend a change to the text, modifying the letter "f."

Mr. Morgan summarized that another meeting and a public hearing are still needed. Council Member Stinson-Wesley would like to hear from the legislature/league and hear what our town attorney's thoughts are on this matter.

Industrial Conditional Zoning Permit (Travis Morgan). Planning Director Morgan advised that the applicant is Turner Fortin, on behalf of Iconic Equities. He is seeking consideration and approval for a new warehouse within the prior 2018 conditional zoning Industrial subdivision. It is a new 194,382 square foot warehouse on Lot 4. This is the last remaining unbuilt parcel in that subdivision. The prior subdivision plan had a traffic study and road improvements

consisting of additional turn lane stacking from North Polk back westward to the railroad tracks. Updates were included and appear to be within the scope of the prior traffic study. Turner Fortin, the applicant, indicated that the majority of the traffic would be headed toward Westinghouse and minimal traffic through the town. A public hearing will need to be scheduled prior to any vote by council.

<u>New Employee Handbook Pay Policies (Linda Gaddy).</u> Ms. Gaddy gave a presentation to Council on the new pay plans that were approved based upon the compensation study that was recently conducted. Policies surrounding the new plans and the changes need to be revised to match within the handbook. We have two basic types of plans for three types of staff: General employees, sworn police officers and firefighters.

Lower end of pay ranges have increased and are now very competitive with the surrounding areas. The Pay Policy Review will have to be added to the bottom of the Police Pay Plan where they are used to seeing them listed and changed in the appropriate sections of the Employee Handbook. Her department will incorporate the policy changes into the Employee Handbook text before the February 2025 Council meeting. New policies will take effect on March 5th along with the new pay plans. Council will vote at the February meeting.

Finance Report for FY25 and FY26 Budget Calendar (Chris Tucker). Mr. Tucker presented the Finance Report for FY25, second quarter. Council will receive a quarterly financial report showing year-to-date revenues and expenditures and comparing each amount to the budget, as amended.

Scheduling of the budget meetings was discussed among Council. Each year, to kick off the budget process, staff should present a budget calendar to lay out the timeline for the Council's budget engagements. They are to send their availability to Chris and Ryan so dates can be confirmed. Mr. Tucker will be bringing in Davenport, once again, to talk to Council.

<u>Coyotes' discussion.</u> Mayor Phillips said that he has received concerns from residents about coyotes in the area. They have been sent to Wildlife Commission to address. There was general discussion on what to do about this in the future if the problem increases.

Parking on Town Roads discussion. Mr. Morgan wanted to check in with Council regarding the Town's on street parking and the Town Code. He added that there are narrow road issues and wide road issues. Some roads have curb and gutter. Council Member Stinson-Wesley asked about staggered parking on collector roads. Town Manager Spitzer noted that all new developments have cut outs.

Council Member Moore moved to adjourn the Work Session followed by a second made by Council Member McDonough. All ayes.

The Mayor adjourned the meeting at 8:20 pm.

ATTEST:

David Phillips, Mayor

Lisa Snyder, Town Clerk



A Proclamation of the Town of Pineville, NC From the Office of Mayor David Phillips

WHEREAS, David Drummond, owner of Carolina Sporting Arms, has provided significant free services to the Pineville Police Department since 2015, greatly enhancing the department's ability to serve our community; and

WHEREAS, the Pineville Police Department has utilized Carolina Sporting Arms' 5,000 square foot indoor shooting range for the qualification of new police applicants; and

WHEREAS, the Pineville Police Department has also used Carolina Sporting Arms' indoor shooting range for the annual qualification of police officers; and

WHEREAS, the Training Division of the Pineville Police Department has benefited from Carolina Sporting Arms' indoor shooting range as a site for combat shooting training; and

WHEREAS, the Training Division of the Pineville Police Department has used the range for transitioning to new weapon systems; and

WHEREAS, Carolina Sporting Arms has been a trusted vendor for weapon acquisitions for the Pineville Police Department; and

WHEREAS, for the past nine years, Carolina Sporting Arms has demonstrated goodwill towards the Pineville Police Department, the Town of Pineville, and the community we serve.

NOW, THEREFORE, I, Mayor David Phillips, Mayor of the Town of Pineville, North Carolina, do hereby recognize **Carolina Sporting Arms** for their selfless service and exemplary partnership with the Pineville Police Department, and for helping to build a stronger, safer, and more vibrant town for all residents.

IN WITNESS WHEREOF, I have hereunto set my hand and caused the Great Seal of Pineville to be affixed this _____ day of _____ in the year of our Lord two thousand and twenty-five.

Mayor David Phillips



TOWN OF PINEVILLE

RESOLUTION 2025-02 IN SUPPORT OF HB24

AN ACT TO RESTORE THE AUTHORITY FOR LOCAL GOVERNMENTS TO INITIATE DOWNZONING

WHEREAS, the North Carolina General Assembly will be considering House Bill 24, which provides for Section 3K.1 of State Law 2024-57 to be repealed; and

WHEREAS, provisions within this legislation would be supportive of local municipal governments to make their own zoning decisions based on the will of the voters that elected them;

WHEREAS, Session Law 2024-57 "AN ACT TO MAKE MODIFICATIONS TO AND PROVIDE ADDITIONAL APPROPRIATIONS FOR DISASTER RECOVERS; TO MAKE TECHNICAL, CLARIFYING, AND OTHER MODIFICATIONS TO THE CURRENT OPERATIONS APPROPRIATIONS ACT OF 2023; AND TO MAKE VARIOUS CHANGES TO THE LAW", became law on the 11th of December 2024, notwithstanding the objections of the Governor; and

WHEREAS, Subpart III-K, Section 3K.1 of State Law 2024-57 reads "NO LOCAL GOVERNMENT INITIATED DOWN-ZONING WITHOUT CONSENT OF THE AFFECTED PROPERTY OWNER"; and

WHEREAS, the passage of Session Law 2024-57 will severely affect the Town's authority to adopt zoning text amendments and regulations related to residential zoning districts. The new law allows an override of the will of a governing board; and

WHEREAS, all North Carolina local governments continue to face challenges presented by growth and development. Each local government's zoning authority has historically provided a means to balance those challenges with the needs of the community based upon the will of the citizens as determined through elections. The downzoning provisions of SL 2024-57 effectively freeze local government zoning and greatly diminish the role of local government granted by the Constitution of the State of North Carolina to manage growth and change consisted with the needs of its jurisdiction

NOW, THEREFORE, BE IT RESOLVED that the TOWN OF PINEVILLE Town Council requests that the elected representatives for the Town of Pineville in Mecklenburg County who serve in the North Carolina General Assembly secure the passage of the proposed House Bill 24 To Restore the Authority for Local Governments to Initiate Downzoning.

Adopted this the 11th day of February 2025.

Mayor David Phillips

Attest:

Town Clerk Lisa Snyder

PUBLIC HEARING



Item 6.

To: Town Council

From: Travis Morgan

Date: 2/11/2025

Re: Blue Heel Dorman Road Townhomes (Public Hearing/Action Item)

REQUEST:

Blue Heel Development seeks your consideration on a site plan specific conditional rezoning. The proposal seeks to rezone the Melinda Earnheart property from R-44 to RMX(CD) to allow for 75 townhome units.

UPDATE:

Street trees added street lights shall be required. Note architectural standards. Recommend note about widow grids and arrangement. Roll out trash containers stored in the garage eliminate one parking space in the garage. Updated paved open space path and two amenities noted. Revegetated 20 foot buffer along some of the property boundary has been added. Construction town specifications added. NCDOT and Dorman Rd improvement lanes added as shown. NCDOT comment was stated as:

"We heard back from NCDOT and they stated that a left turn lane with 100' storage, 50' decel and appropriate taper is to be provided into the proposed access. A 3-lane section is to be constructed from the approach of Huntley Glen Dr and the approach from the Carolina Sportsplex drive to avoid a "hourglass" effect pending R/W investigation. They also requested a 100' internal stem protection, which we currently meet."

DEVELOPMENT SUMMARY:

Parcel number:	22110102
Acreage:	+/-15.28
Units:	75
Density:	4.91 units/acre
Min. Parking:	244 (3.25/unit)
Parking Provided:	252 spaces (150 spaces 2per driveway, 75 spaces 1per garage, and 27 onstreet)
Trash:	Public rollouts

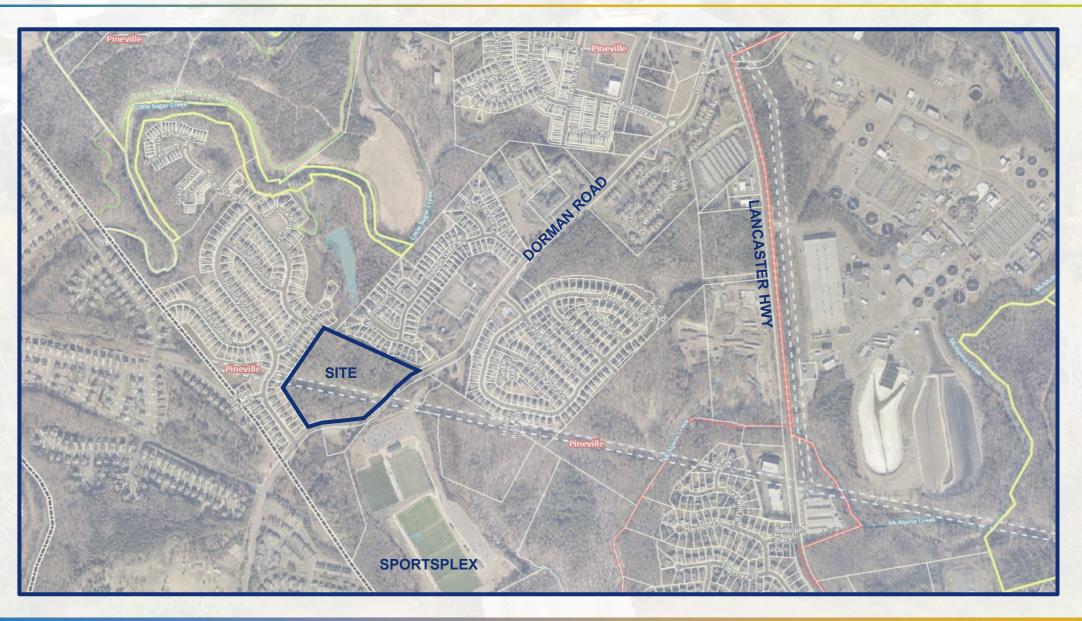
STAFF COMMENT:

Plan progresses with added information. Recommended additional building notes such as porch railings and window grids as shown in the renderings. Also recommended is for Townhouse only use since RMX allows for other uses. Plan is recommended with these notes and final approval with NCDOT, fire, and trash turn radius review. Plan is found to be consistent with adopted plans such as the comprehensive plan regarding housing, open space, and walkability.

ACTION:

This the public hearing to hear the request and updates to the plans and gain community input. This follows regular legislative process, consideration and consistency determination to adopted plans and goals. A public hearing is needed before any final vote.

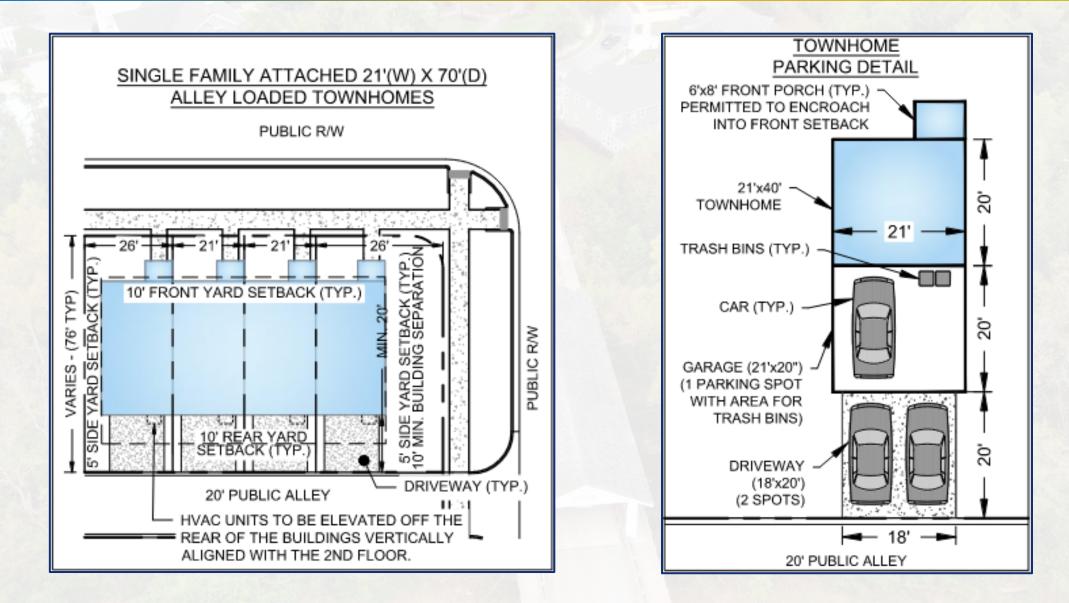
Community Location



Community Vision



Dimensional Standards



Elevations

ELEVATIONS ARE REPRESENTATIONAL ON V AND ARE NOT FINAL DESIGNS. FINAL HOM Item 6. ELEVATION DESIGNS ARE SUBJECT TO CHANGE.



Elevations

ELEVATIONS ARE REPRESENTATIONAL ON V AND ARE NOT FINAL DESIGNS. FINAL HOM *Item 6.* ELEVATION DESIGNS ARE SUBJECT TO CHANGE.



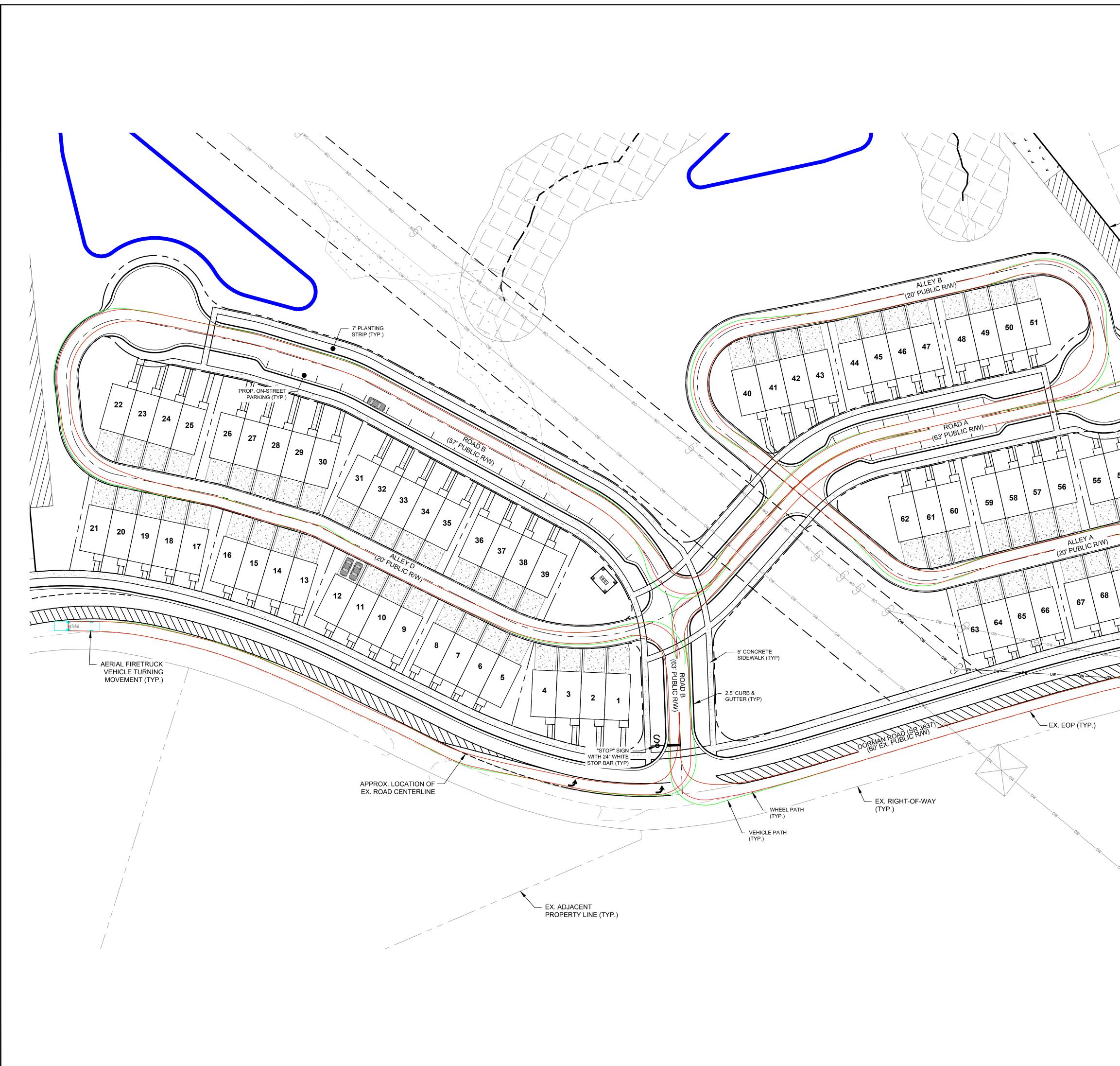
Elevations

ELEVATIONS ARE REPRESENTATIONAL ONLY AND ARE NOT FINAL DESIGNS. FINAL HOL *Item 6.* ELEVATION DESIGNS ARE SUBJECT TO CHANGE.











1213 W. MOREHEAD STREET, SUITE 300 CHARLOTTE, NC 28208 (t) (704) 334-0078 (f) (704) 334-5348 WWW.WKDICKSON.COM NC LICENSE NO.F-0374



PROJECT:

SHEET TITLE:

- PROJECT BOUNDARY (TYP.)

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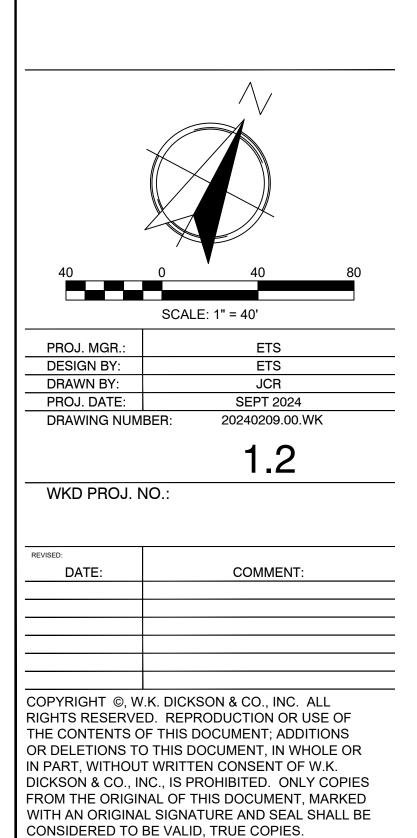
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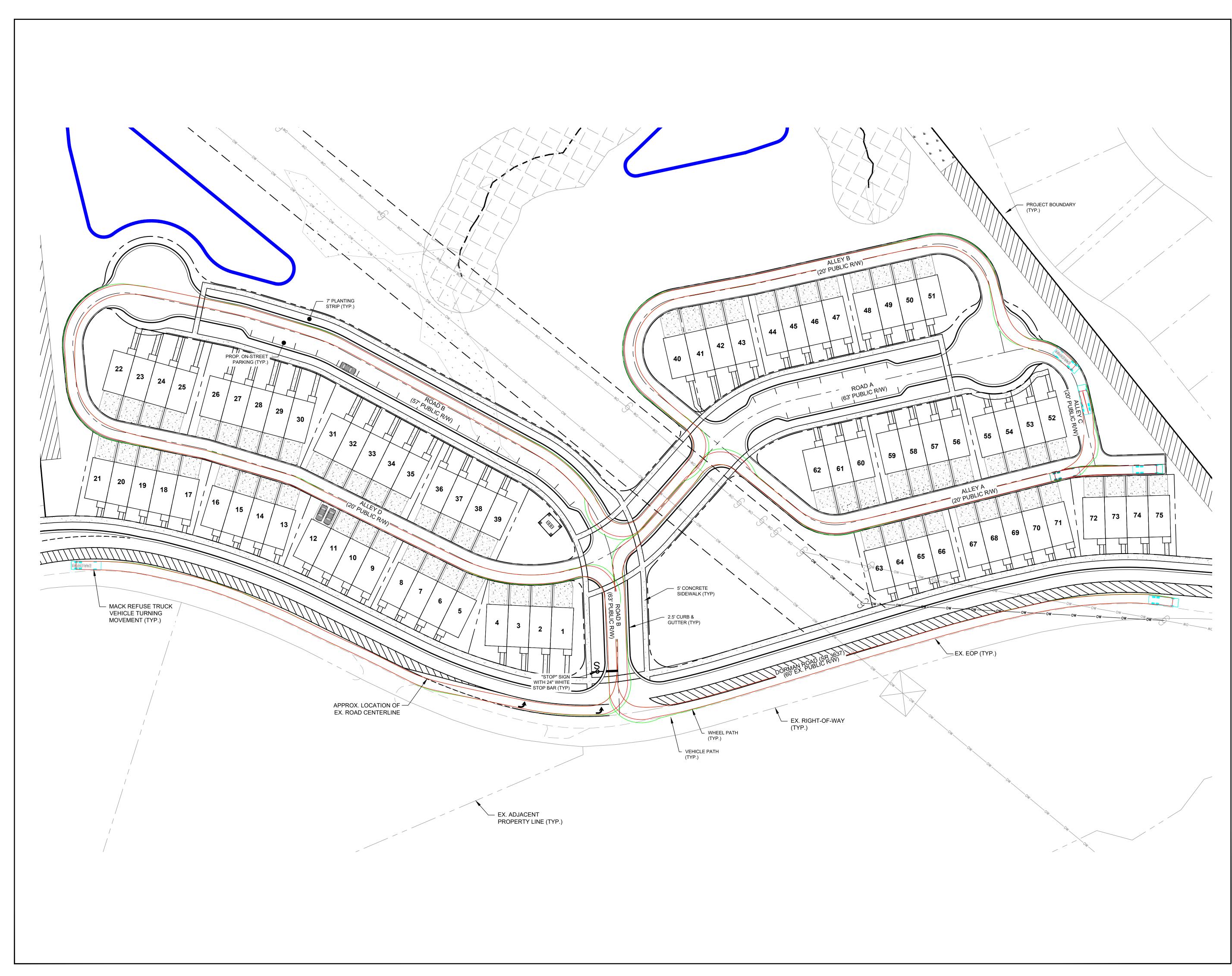
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54

Towne Living at Pineville

Fire Apparatus Turning Movements







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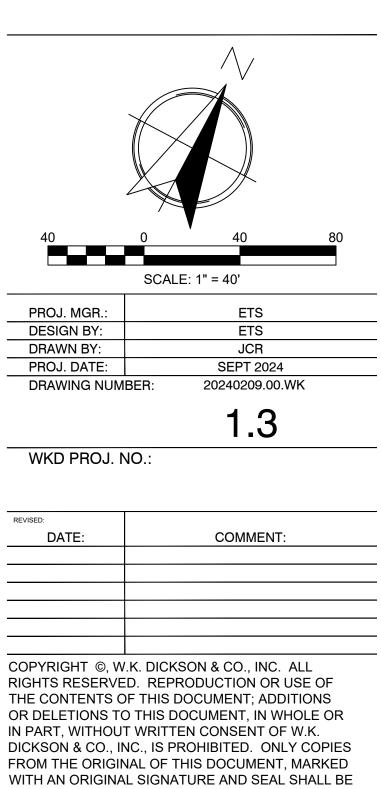


PROJECT:

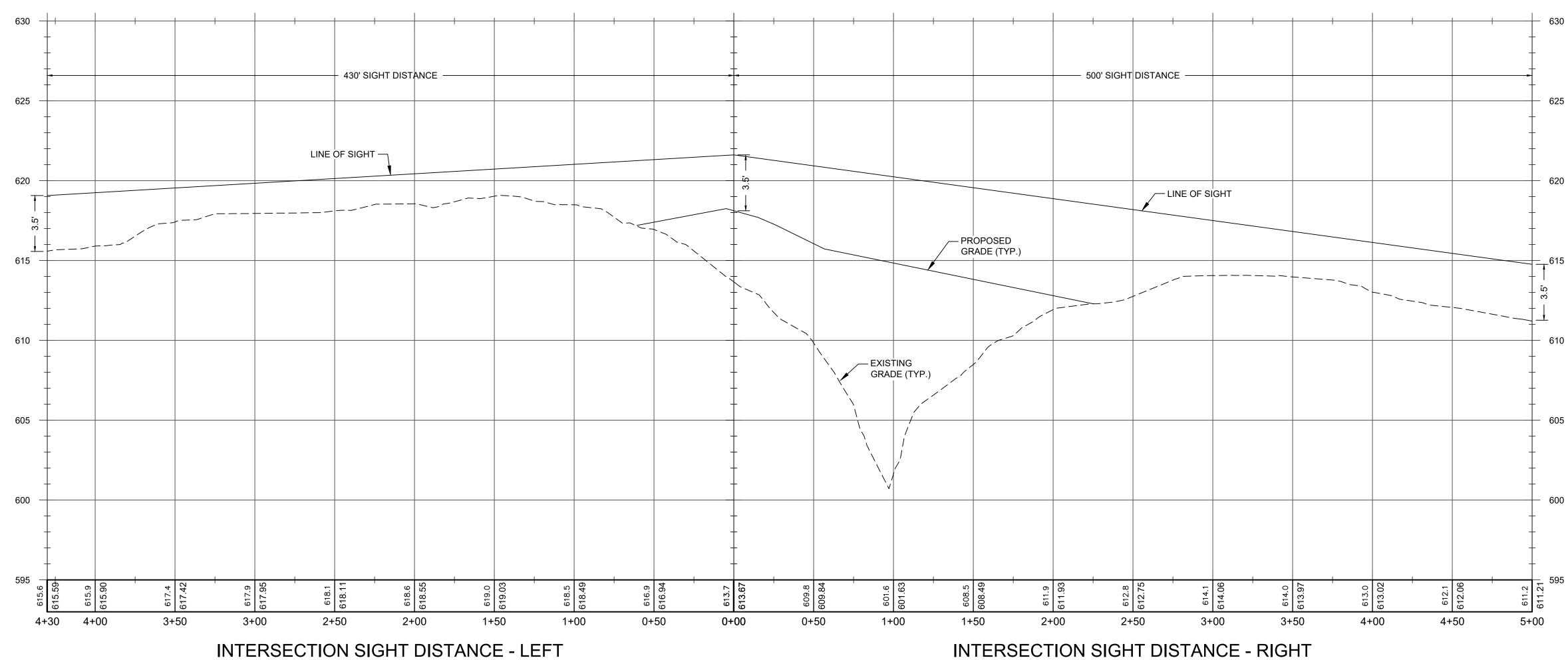
SHEET TITLE:

Towne Living at Pineville

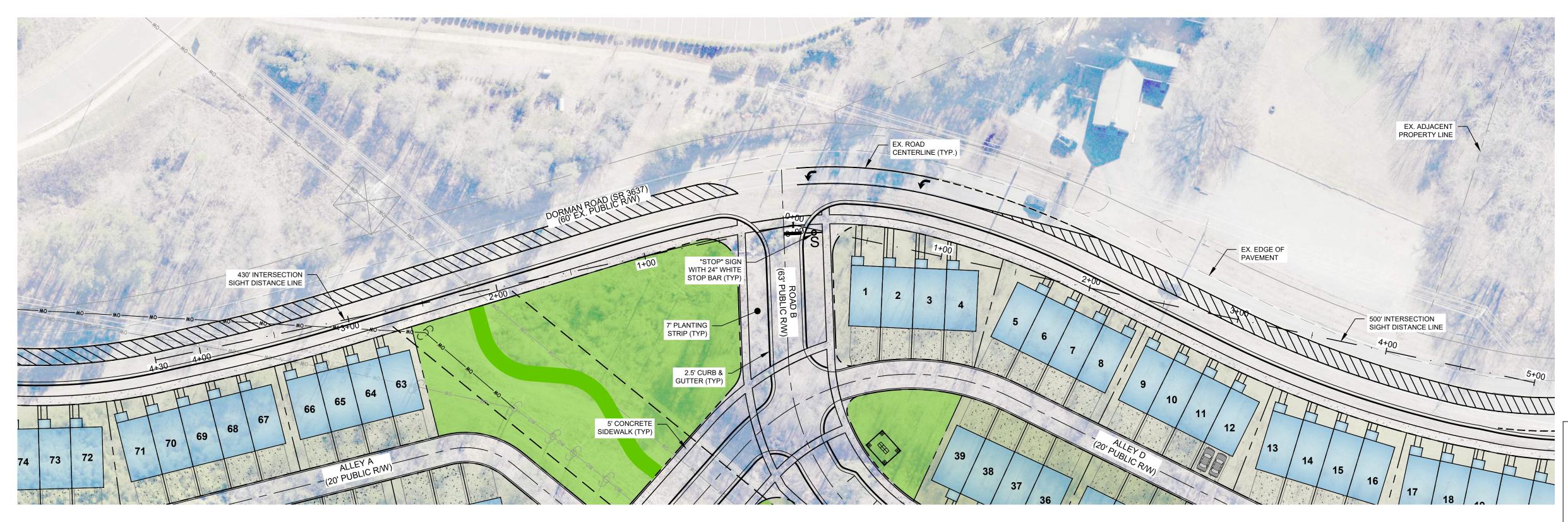
Refuse Truck Turning Movements



CONSIDERED TO BE VALID, TRUE COPIES.



SCALE: HOR 1"=40'; VERT 1"=4'



INTERSECTION SIGHT DISTANCE - RIGHT SCALE: HOR 1"=40'; VERT 1"=4'





1213 W. MOREHEAD STREET, SUITE 300 CHARLOTTE, NC 28208 (t) (704) 334-0078 (f) (704) 334-5348 WWW.WKDICKSON.COM NC LICENSE NO.F-0374

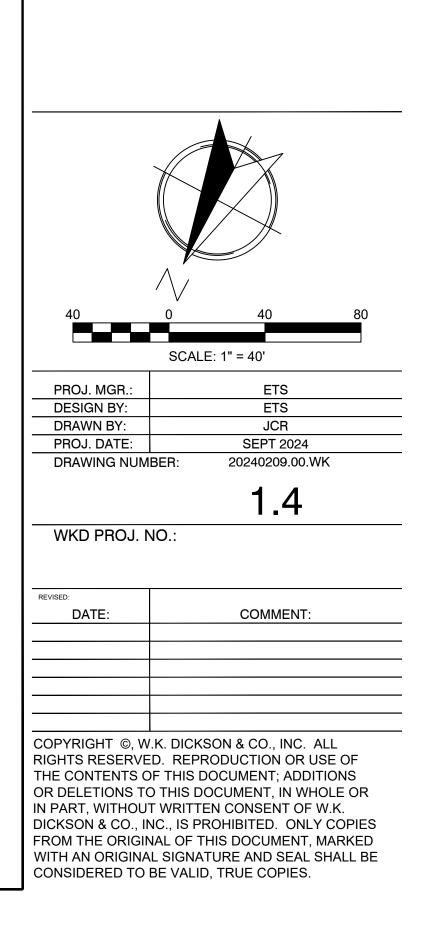


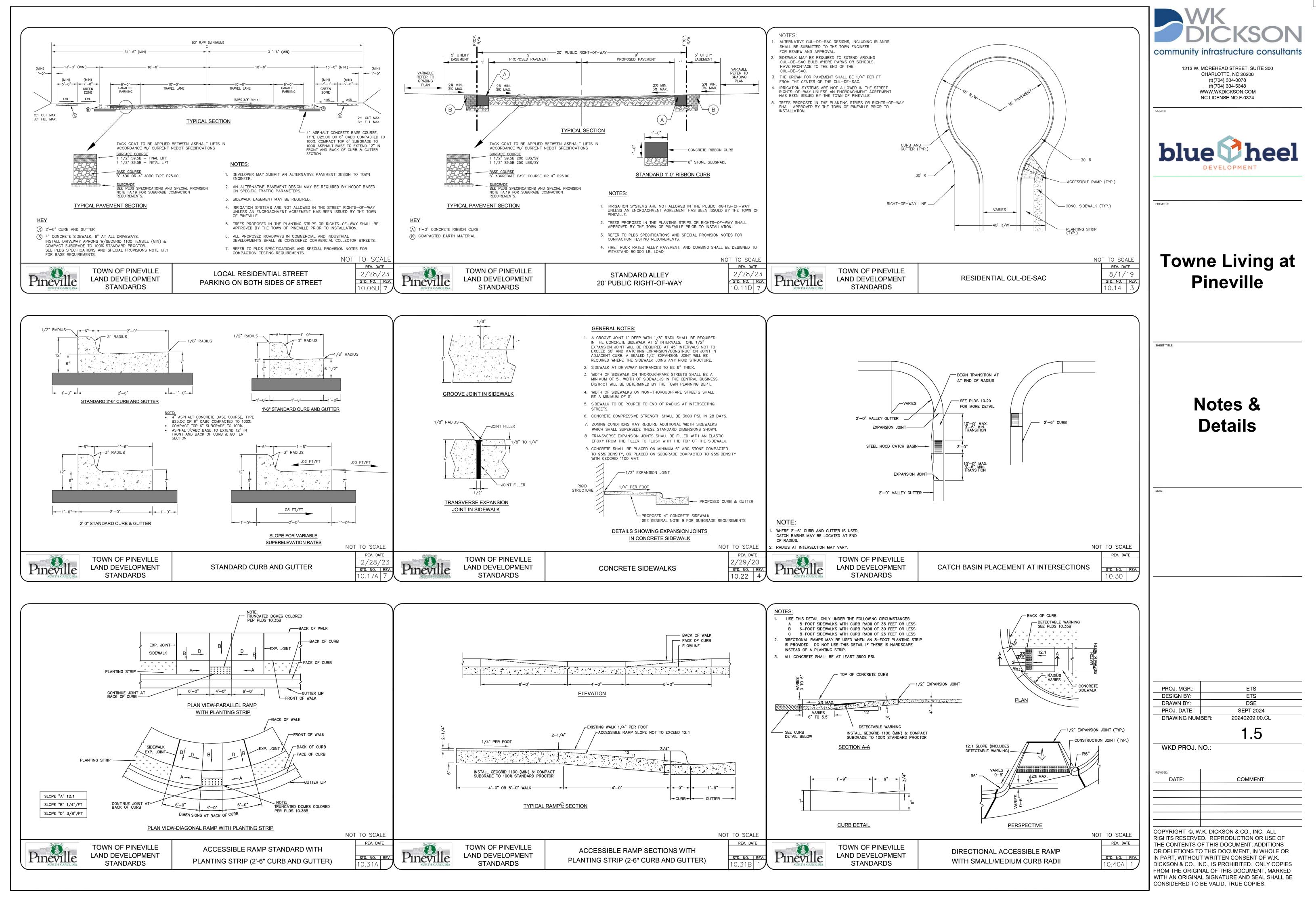
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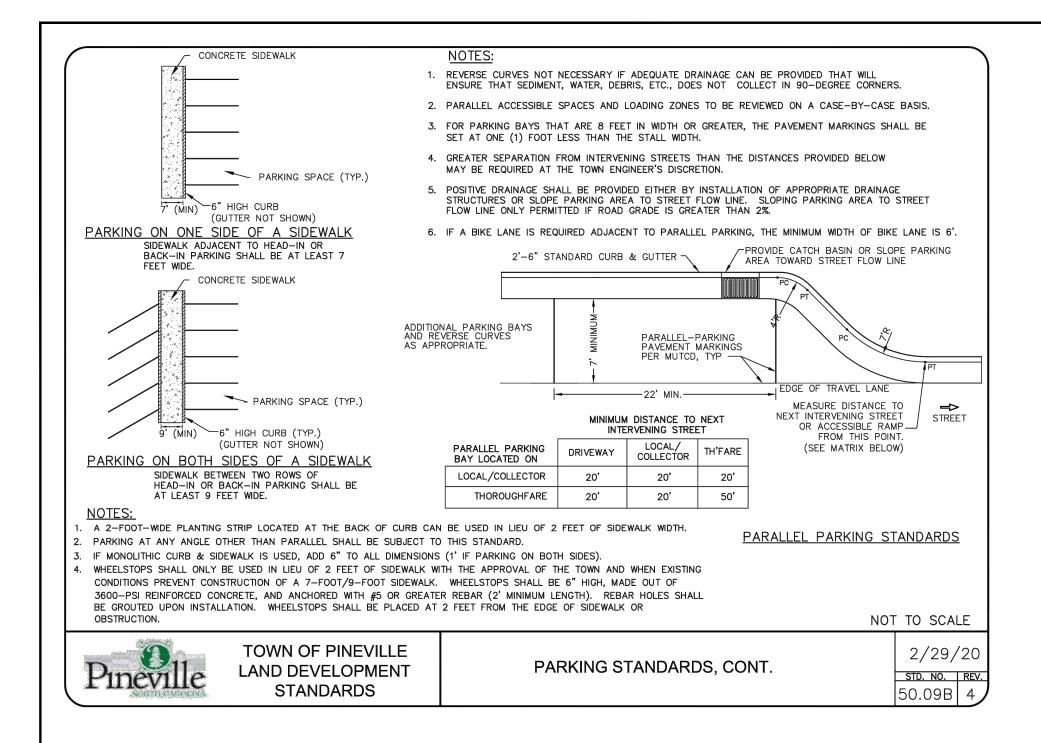
SHEET TITLE:

Towne Living at Pineville

ISD Plan & Profile







PARCEL #SQ. FT.224,023231,926241,927252,499262,497271,923281,923291,923302,644312,373321,804331,788341,773352,195362,172371,720381,704392,055403,118412,040421,841432,092441,988451,596461,596	Parcel Table				
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45 1,596	43	2,092			
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46 1,596	45	1,596			
	46	1,596			

Parcel Table				
PARCEL #	SQ. FT.			
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48	1,976			
49	1,596			
50	1,596			
51	3,465			
52	2,827			
53	1,665			
54	1,608			
55	2,148			
56	2,189			
57	1,762			
58	1,772			
59	2,208			
60	2,224			
61	1,807			
62	3,740			
63	2,015			
64	1,606			
65	1,598			
66	1,977			
67	1,975			
68	1,594			
69	1,603			
70	1,628			
71	2,226			

Parcel	Parcel Table				
PARCEL #	SQ. FT.				
72	2,231				
73	1,624				
74	1,608				
75	1,998				



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CLIENT:

PROJECT:

SHEET TITLE:



Towne Living at Pineville

Notes & Details

PROJ. MGR.:	ETS
DESIGN BY:	ETS
DRAWN BY:	DSE
PROJ. DATE:	SEPT 2024
DRAWING NUMI	BER: 20240209.00.CL

1.6

WKD PROJ. NO.:

REVISED: DATE: COMMENT:

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A varied color palette shall be utilized on homes throughout the subdivision for siding and shall include varied trim, shutter, and accent colors complementing the siding color. Where varied siding and trim, shutter, and accent colors are not provided on an individual dwelling, a minimum of three varied materials shall be provided.

In addition, community commitments will include:

- No Vinyl Siding allowed
- When 100% Hardie Plank (or Hardie equivalent) siding is used then at least two different types of Hardie siding to be employed
 - Decorative trim including vertical siding/board and batten style
 - Decorative shakes
 - o Horizontal siding
- Stone and Brick water table or accents on front of the home
- All elevations of the homes that that front on a right-of-way shall have usable front porches with dimensions that are a minimum of 6' deep and 6' wide. Majority of homes in any given building shall have a 6' deep by 8' wide porches.
 - Porches to include railings
- Garages to include:
 - Carriage style hardware
- Rear elevations to include a private deck or porch

Detailed design shall be provided by using at least three (3) of the following architectural features on front elevations:

- Dormers
- Gables
- Porches
- Cupolas or towers
- Pillars or Posts
- Eaves (minimum 6 inch projection)
- Window Trim (minimum 4 inches wide)
- Bay Windows
- Balconies
- Metal roof accents
- Decorative patterns on exterior finish (e.g. scales/shingles, wainscoting, ornamentation, and similar features)
- Decorative cornices and roof lines (for flat roofs)



Item 7.

To: Town Council

From: Travis Morgan

Date: 2/11/2025

Re: Iconic Equities Warehouse Conditional Zoning Request (Public Hearing/Action Item)

PROPOSAL:

Turner Fortin on behalf of Iconic Equities seeks your consideration and approval for a new warehouse withing the prior 2018 conditional zoning industrial subdivision. Request is for a new 194,382 square foot warehouse on Lot 4 (the last remaining unbuilt parcel) in the subdivision.

BACKGROUND and INFORMATION:

This proposal seeks to update the prior March 2018 conditional zoning approved plan lead by the Lance warehouse and industrial subdivision located along Pineville Distribution Street. Conditional approval is needed for users over 100,000 square feet. Lance warehouse was the only large warehouse in the prior approval.

General site information:

Address:	10203 Pineville Distribution Street
Tax Parcel:	20507120
Property Acres:	15 acres
Square Feet:	194,382 square foot warehouse and distribution (no manufacturing)
Parking Min:	(stated 50 spaces) 1 space per 4000 sqft of warehouse plus 1 per 350 office
Parking Provided:	185

Traffic Study:

Previous subdivision plan had a traffic study and road improvements consisting of additional turn lane stacking from North Polk back Westward to the railroad tracks. Sealed transportation engineer analysis update is included and notes traffic generation from the development to be within the scope of the prior traffic study.

STAFF COMMENTS:

The overall proposal seems consistent the original subdivision approval. The property is a flag shape with a long driveway and property line is approximately 1,400 linear feet to the closest house in Preston Park though mature forest and big Sugar Creek floodplain area. Upon completion of this lot Pineville Distribution Street can be completed/inspected and turned over to the Town and greenway area can be dedicated. Recommended discussion on sidewalk going into the property, rounding up permitted square feet to 200,000sqft., and possible contribution to median or similar to prevent tractor trailers from entering Preston Park. Proposal is recommended and staff finds consistent with adopted plans.

PROCEDURE:

This is a standard legislative process public hearing for public comment on the proposal. You may vote on the proposal after the close of the public hearing, or you may vote to continue the public meeting into the future.

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PLANNING & ZON	11	V G

Submit to Planning Department, 200 Dover St, Pineville, NC 28134 Phone (704) 889-2291 Fax (704) 889-2293

Office Use Only:				Applic	ation #:
Payment Method:	Cash	Check	Credit Card	_ Amount \$	Date Paid
		Zo	ning App		
	DAG SHILL DO				prents listed have been completed
Applicant's Name: ICOI					
					Phone: 404.863.9931
Applicant's Mailing Add		Бау Коао	, Unit 1105, Mia	ami Beach, FL	33139
Property Information					
Property Location: 102	03 Pinevi	lle Distribu	ition St, Pinevill	e, NC 28134	
Property Owner's Mailin	g Address: _	1062 Win	netka Ave, Cha	tsworth, CA 91	1311
Property Owner Name:					Phone: 818.230.7609
Tax Map and Parcel Nu	mber: <u>205(</u>	07120		Existing Zo	oning, G-I
Which are you apply	ing (Check	all that ap			
Rezoning by Right		nditional Zonin		nditional Rezoning	Text Amendment
Fill out section(s) that	t apply:				
Rezoning by Right:	^ر د	ą –			
Proposed Rezoning Desig	gnation				
Conditional Zoning:		s.			
Proposed Conditional Use	<u>Industri</u>	al			
Acreage <u>15.0</u>	Square	Feet <u>194,3</u>	82 Approxim	nate Height 44	# of Rooms N/A
Parking Spaces Required	_50	Parking Spac	es Provided 185	**Pleas	e Attach Site Specific Conditional Plan
Conditional Rezoning:					
Proposed Conditional Rez	oning Desig	nation			
Text Amendment:					
Section					
Proposed Text Change (A	ttach if need	ed)	<u>s</u>		
-					

I do hereby certify that all information which I have provided for this application is, to the best of my knowledge, correct.

 $\overline{\mathcal{O}}$ ° a∕ Signature of Applicant Signature of Property Owner (If not Applicant)

 $\frac{|2/11/2024}{|2|11|2024}$ $\frac{|2|11|2024}{|200}$ Date

Signature of Town Official

Date



December 9th, 2024

Turner Fortin Director of Acquisitions & Development Iconic Equities Mobile 404-863-9931 1508 Bay Road Unit 1105 Miami Beach, FL 33139

RE: Pineville Industrial Lot 4 Trip Generation Memorandum

Dear Mr. Fortin,

This trip generation memorandum is a supplement to the Pineville Industrial Development TIA (completed by Timmons Group sealed 01/12/2018). The purpose of this memorandum is to determine if the current proposed build-out (up to and including Lot 4) exceeds trip generation values assumed in the TIA.

Per the approved TIA, Phase 1 of the subject development included 510,000 square feet (SF) of warehousing. Additionally, Phase 2 of the subject development included 340,000 SF of general light industrial.

Lot 4 will consist of 194,382 SF of general light industrial. Per aerial imagery, 510,000 SF of warehousing and 97,406 SF of general light industrial has already been constructed. Following the construction of Lot 4, the Pineville Industrial Development will consist of 510,000 SF of warehousing and 291,788 SF of general light industrial.

 Table 1 summarizes the Pineville Industrial trip generation as outlined in the TIA.

Table 1. Finevine industrial TA Filases I – If The Generation Summary								
ITE Land Use	Size	ADT	AM Peak Hour			PM Peak Hour		
Code	Size	ADT	In	Out	Total	In	Out	Total
510 – Warehousing	510,000 SF	1,816	121	32	153	41	122	163
110 – General Light Industrial	340,000 SF	2,438	274	37	311	39	289	328
	Total:	4,254	395	69	464	80	411	491

Table 1: Pineville Industrial TIA Phases I – II Trip Generation Summary

SOURCE: Pineville Industrial TIA (completed by Timmons Group sealed 01/12/2018)

Table 2 summarizes the cumulative Pineville Industrial trip generation (including Lot 4). These values were determined by applying the projected percent buildouts to the assumed TIA trip generation shown in **Table 1**.

Table 2. Filleville industrial Lot 4 Trip Generation Summary									
ITE Land Use	Size	0/ Duildout ADT		AM Peak Hour		PM Peak Hour			
Code	Size	% Buildout	ADT	In	Out	Total	In	Out	Total
510 – Warehousing	510,000 SF	100%	1,816	121	32	153	41	122	163
110 – General Light Industrial	291,788 SF	86%	2,097	236	31	267	34	248	282
		Total:	3,913	357	63	420	75	370	445

Table 2: Pineville Industrial Lot 4 T	Frip Generation Summary
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Suite 985 | Columbia, SC 29201

Main Street,

1201

Development | Residential | Infrastructure | Technology

Site

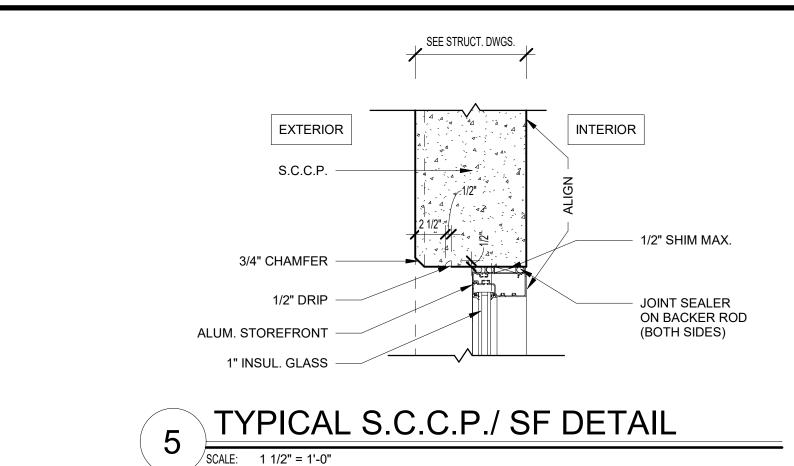


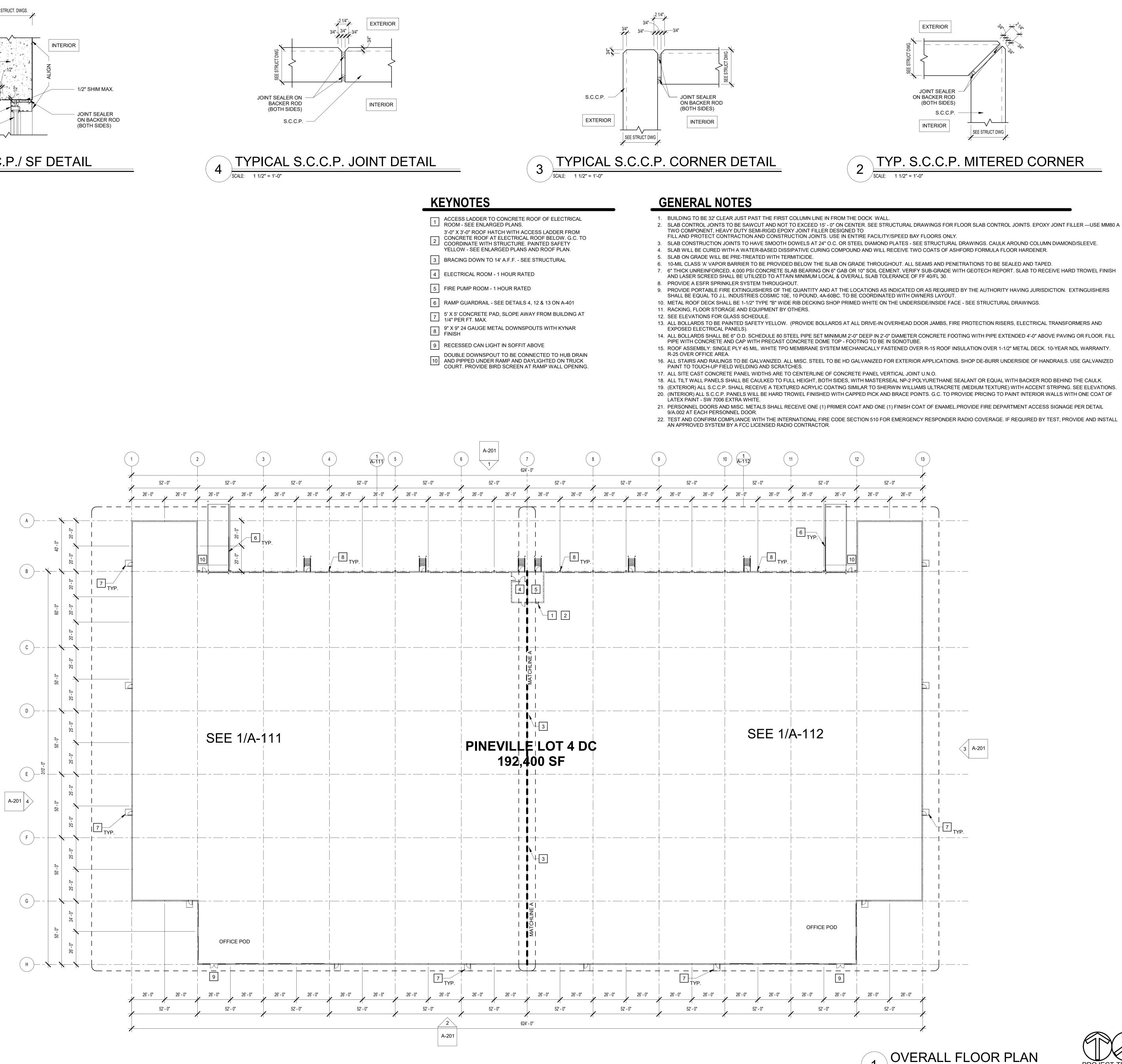
As shown in Tables 1 & 2, with the construction of Lot 4, trips are not projected to exceed trip generation values assumed in the Pineville Industrial Development TIA. Therefore, no TIA update is required due to the development's construction.

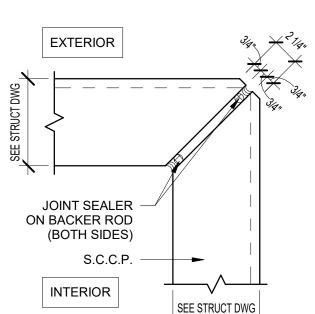
Should you have any questions regarding this memorandum, do not hesitate to contact me.



Senior Project Manager | Transportation







TYP. S.C.C.P. MITERED CORNER

4. SLAB WILL BE CURED WITH A WATER-BASED DISSIPATIVE CURING COMPOUND AND WILL RECEIVE TWO COATS OF ASHFORD FORMULA FLOOR HARDENER.

6. 10-MIL CLASS 'A' VAPOR BARRIER TO BE PROVIDED BELOW THE SLAB ON GRADE THROUGHOUT. ALL SEAMS AND PENETRATIONS TO BE SEALED AND TAPED. 7. 6" THICK UNREINFORCED, 4,000 PSI CONCRETE SLAB BEARING ON 6" GAB OR 10" SOIL CEMENT. VERIFY SUB-GRADE WITH GEOTECH REPORT. SLAB TO RECEIVE HARD TROWEL FINISH

9. PROVIDE PORTABLE FIRE EXTINGUISHERS OF THE QUANTITY AND AT THE LOCATIONS AS INDICATED OR AS REQUIRED BY THE AUTHORITY HAVING JURISDICTION. EXTINGUISHERS 10. METAL ROOF DECK SHALL BE 1-1/2" TYPE "B" WIDE RIB DECKING SHOP PRIMED WHITE ON THE UNDERSIDE/INSIDE FACE - SEE STRUCTURAL DRAWINGS.

13. ALL BOLLARDS TO BE PAINTED SAFETY YELLOW. (PROVIDE BOLLARDS AT ALL DRIVE-IN OVERHEAD DOOR JAMBS, FIRE PROTECTION RISERS, ELECTRICAL TRANSFORMERS AND

14. ALL BOLLARDS SHALL BE 6" O.D. SCHEDULE 80 STEEL PIPE SET MINIMUM 2'-0" DEEP IN 2'-0" DIAMETER CONCRETE FOOTING WITH PIPE EXTENDED 4'-0" ABOVE PAVING OR FLOOR. FILL 15. ROOF ASSEMBLY: SINGLE PLY 45 MIL. WHITE TPO MEMBRANE SYSTEM MECHANICALLY FASTENED OVER R-15 ROOF INSULATION OVER 1-1/2" METAL DECK. 10-YEAR NDL WARRANTY. 16. ALL STAIRS AND RAILINGS TO BE GALVANIZED. ALL MISC. STEEL TO BE HD GALVANIZED FOR EXTERIOR APPLICATIONS. SHOP DE-BURR UNDERSIDE OF HANDRAILS. USE GALVANIZED

18. ALL TILT WALL PANELS SHALL BE CAULKED TO FULL HEIGHT, BOTH SIDES, WITH MASTERSEAL NP-2 POLYURETHANE SEALANT OR EQUAL WITH BACKER ROD BEHIND THE CAULK. 19. (EXTERIOR) ALL S.C.C.P. SHALL RECEIVE A TEXTURED ACRYLIC COATING SIMILAR TO SHERWIN WILLIAMS ULTRACRETE (MEDIUM TEXTURE) WITH ACCENT STRIPING. SEE ELEVATIONS. 20. (INTERIOR) ALL S.C.C.P. PANELS WILL BE HARD TROWEL FINISHED WITH CAPPED PICK AND BRACE POINTS. G.C. TO PROVIDE PRICING TO PAINT INTERIOR WALLS WITH ONE COAT OF 21. PERSONNEL DOORS AND MISC. METALS SHALL RECEIVE ONE (1) PRIMER COAT AND ONE (1) FINISH COAT OF ENAMEL. PROVIDE FIRE DEPARTMENT ACCESS SIGNAGE PER DETAIL

22. TEST AND CONFIRM COMPLIANCE WITH THE INTERNATIONAL FIRE CODE SECTION 510 FOR EMERGENCY RESPONDER RADIO COVERAGE. IF REQUIRED BY TEST, PROVIDE AND INSTALL

/SCALE: 1" = 30'-0"

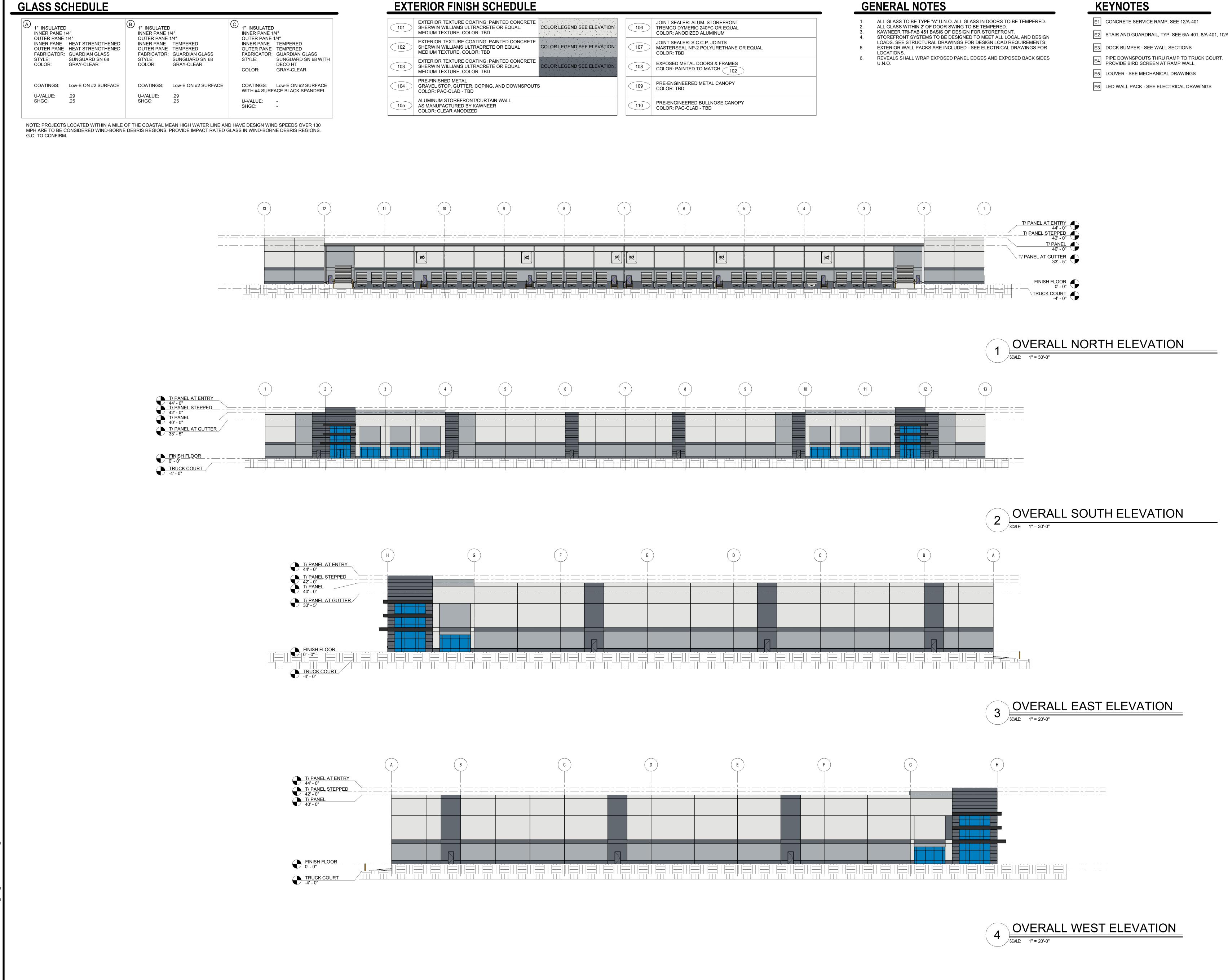


	ltem 7.
ATLAS COLLABORATIVE	
PINEVILLE DC - LOT 4	
PINEVILLE, NC	
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Issue Date Job No. 12/09/2024 pineville-lot4	-
Sheet Title	-
OVERALL FLOOR PLAN	-
Sheet No. A-101	
NOT ISSUED FOR CONSTRUCTION	-





	tem 7.
PINEVILLE DC - LOT 4	
PINEVILLE, NC	
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Revisions	
Issue Date Job No. 12/09/2024 pineville-lot4	,
Sheet Title ENTRY VIEW	
Sheet No. A-200	
NOT ISSUED FOR CONSTRUCTION	,



EXTERIOR FINISH SCHEDULE					
101	EXTERIOR TEXTURE COATING: PAINTED CONCRETE SHERWIN WILLIAMS ULTRACRETE OR EQUAL. MEDIUM TEXTURE. COLOR: TBD	COLOR LEGEND SEE ELEVATION		106	JOINT SEALER: ALUM. STOREFRONT TREMCO DYMERIC 240FC OR EQUAL COLOR: ANODIZED ALUMINUM
102	EXTERIOR TEXTURE COATING: PAINTED CONCRETE SHERWIN WILLIAMS ULTRACRETE OR EQUAL MEDIUM TEXTURE. COLOR: TBD	COLOR LEGEND SEE ELEVATION		107	JOINT SEALER: S.C.C.P. JOINTS MASTERSEAL NP-2 POLYURETHANE OR EQUAL COLOR: TBD
103	EXTERIOR TEXTURE COATING: PAINTED CONCRETE SHERWIN WILLIAMS ULTRACRETE OR EQUAL MEDIUM TEXTURE. COLOR: TBD	COLOR LEGEND SEE ELEVATION		108	EXPOSED METAL DOORS & FRAMES COLOR: PAINTED TO MATCH 102
104	PRE-FINISHED METAL GRAVEL STOP, GUTTER, COPING, AND DOWNSPOUT COLOR: PAC-CLAD - TBD	3		109	PRE-ENGINEERED METAL CANOPY COLOR: TBD
105	ALUMINUM STOREFRONT/CURTAIN WALL AS MANUFACTURED BY KAWNEER COLOR: CLEAR ANODIZED			110	PRE-ENGINEERED BULLNOSE CANOPY COLOR: PAC-CLAD - TBD

- E2 STAIR AND GUARDRAIL, TYP. SEE 6/A-401, 8/A-401, 10/A-401

- E6 LED WALL PACK SEE ELECTRICAL DRAWINGS

	tem 7.
PINEVILLE DC - LOT 4	
PINEVILLE, NC	
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Sheet No. A-201	•
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PINEVILLE DISTRIBUTION LOT 4 CONDITIONAL ZONING PLAN PINEVILLE, NORTH CAROLINA ACCELA

INTERSTATE 485 PINEVILLE DISTRIBUTION STREET

VICINITY MAP SCALE: 1" = 1,000'

DEVELOPER

ICONIC EQUITIES 1508 BAY ROAD, UNIT 1105 MIAMI BEACH, FL 33139 CONTACT: TURNER FORTIN PHONE: 404.863.9931 EMAIL: TURNER@ICONICEQUITIESGROUP.COM

PROPERTY OWNERS

PARCEL ID: 20507120 CONCORD CALIFORNIA ASSOCIATES, LLC 11062 WINNETKA AVE CHATSWORTH, CA 91311

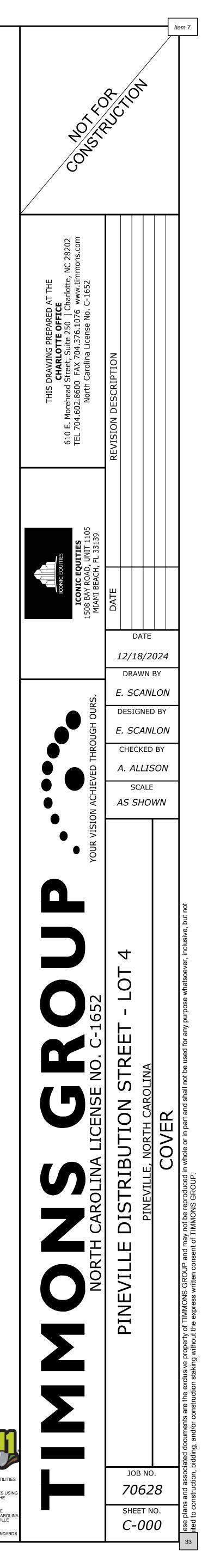
CIVIL/LANDSCAPE

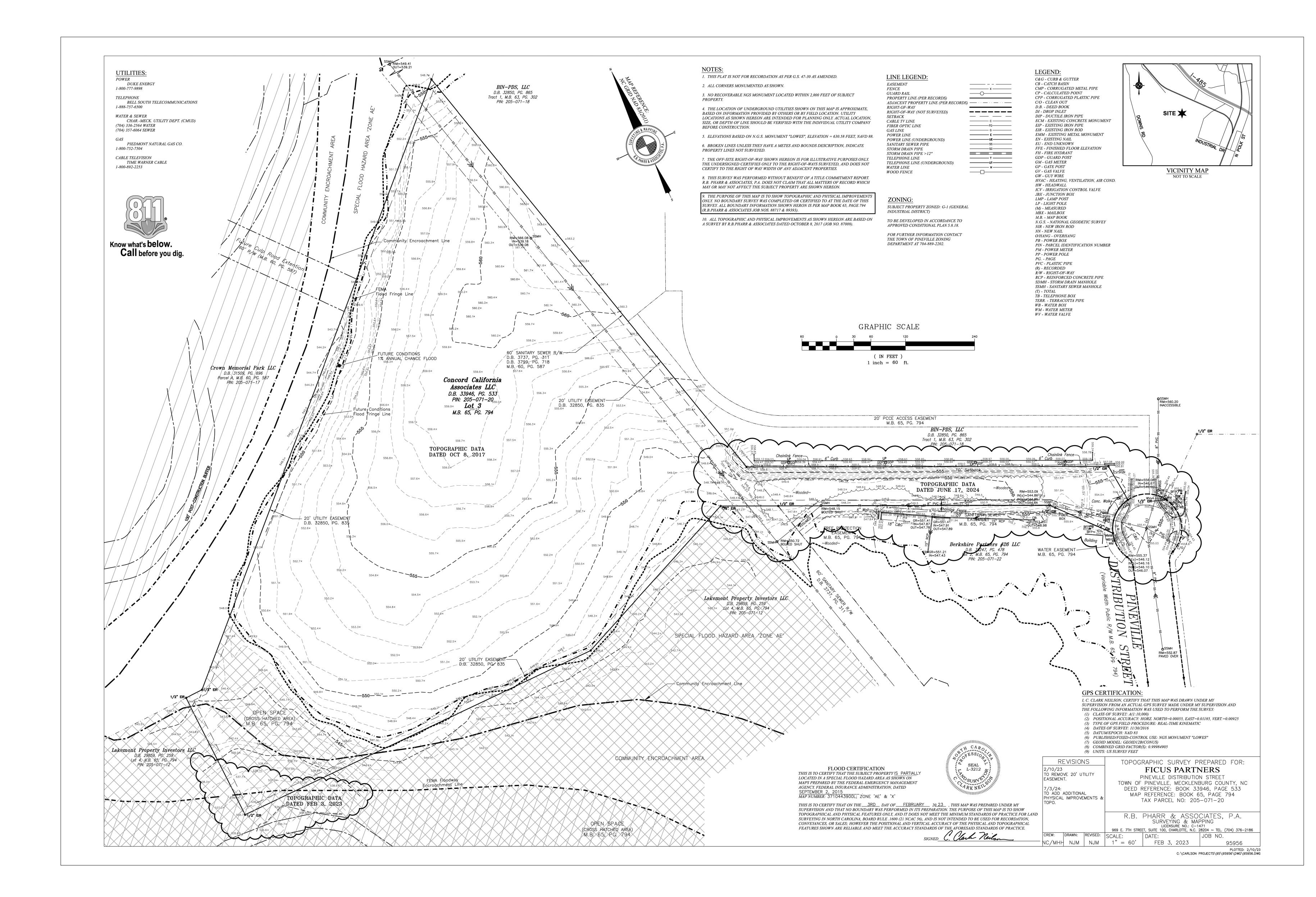
TIMMONS GROUP 610 E. MOREHEAD STREET, SUITE 250 CHARLOTTE, NC 28202 ENGINEER OF RECORD: ANDREW ALLISON, P.E. PHONE: 704.227.1564 EMAIL: ANDREW.ALLISON@TIMMONS.COM

Sheet List Table

Sheet Number	Sheet Title
C-000	COVER
V-100	SURVEY
C-100	CONDITIONAL ZONING SITE PLAN
L-100	LANDSCAPE PLAN
LI-100	LIGHTING PLAN

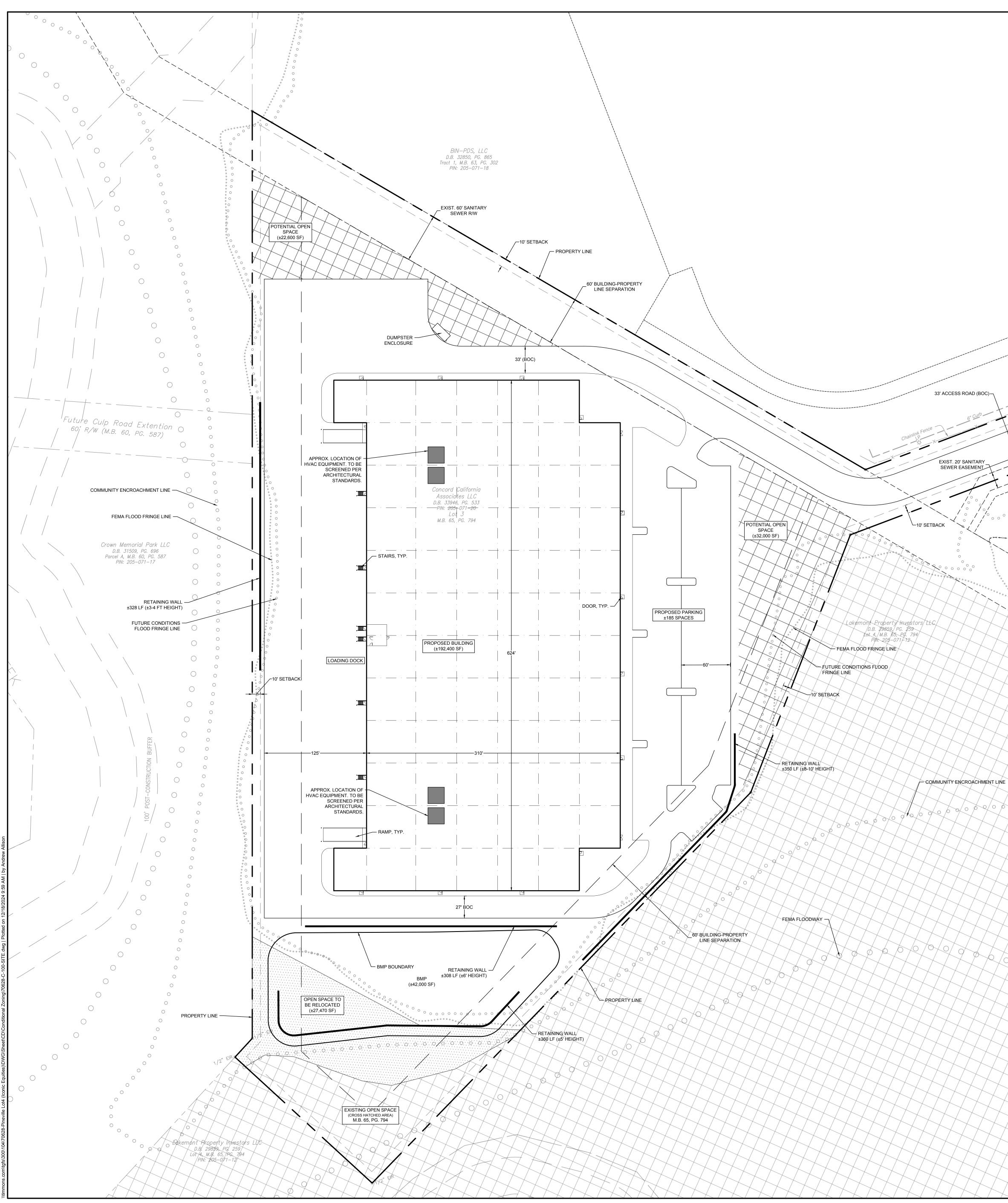


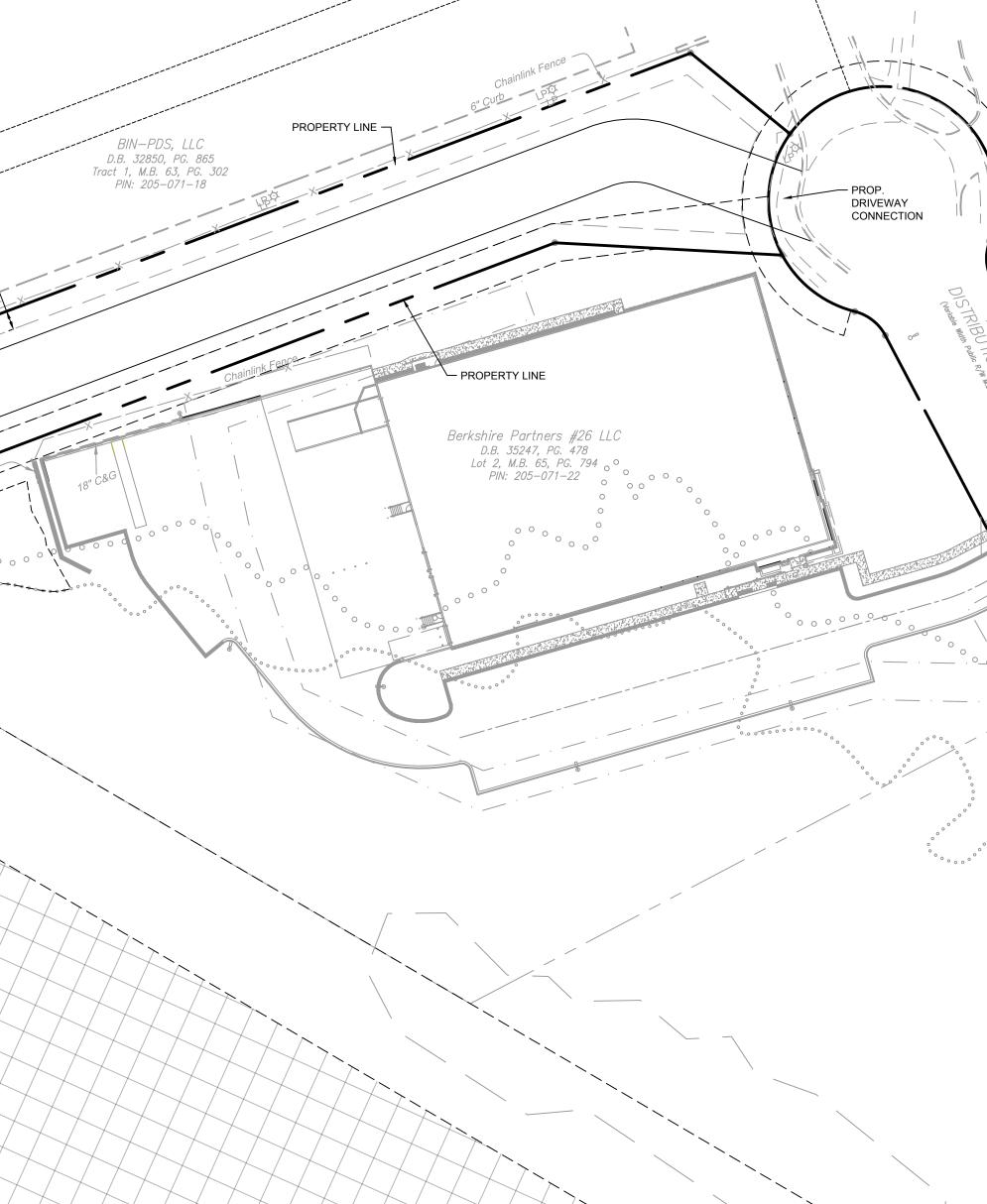




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Item 7.





10' SETBACK –

6" Wa

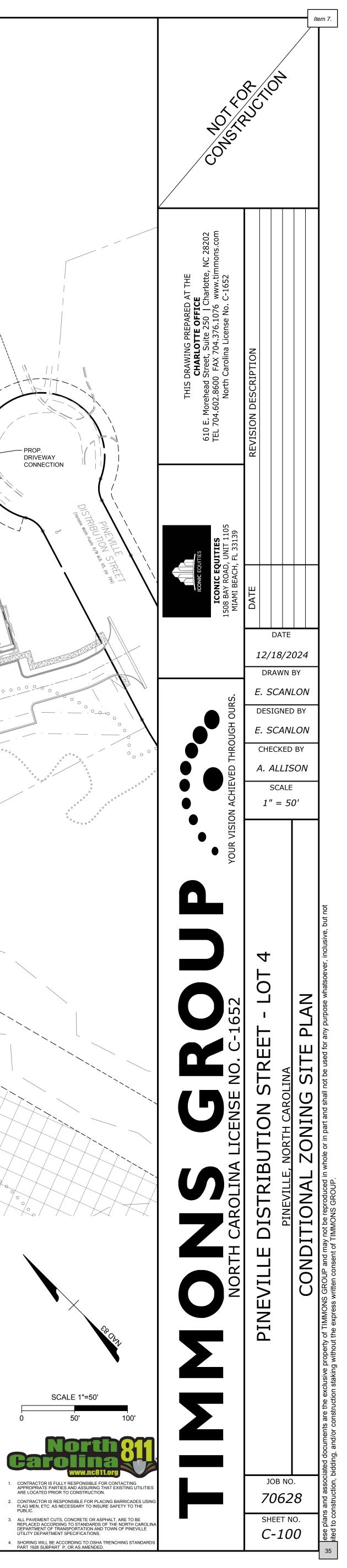
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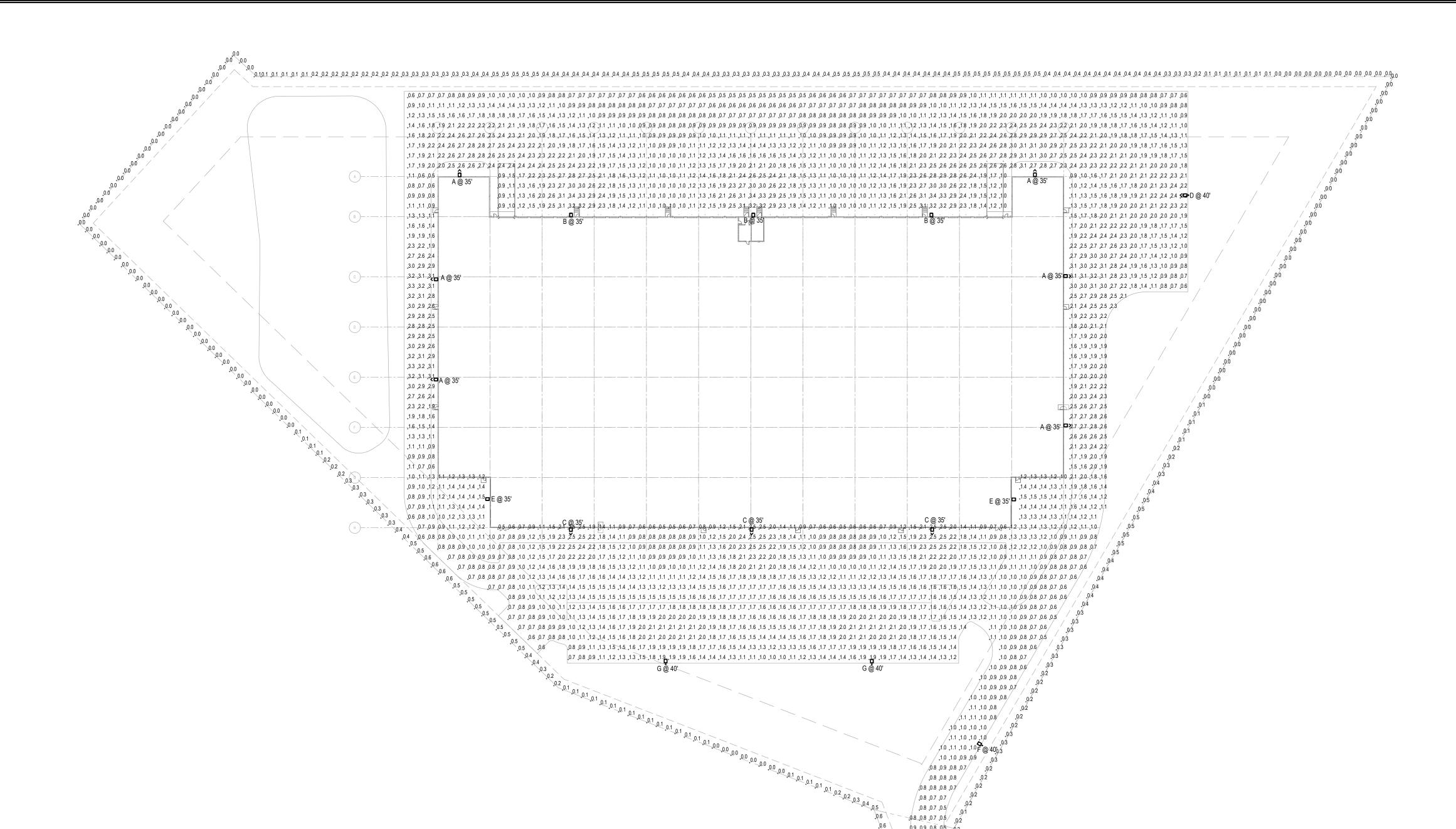
ZONING CODE SUMM	/IARY

OWNER:	CONCORD CALIFORNIA ASSOCIATES, LLC 11062 WINNETKA AVE CHATSWORTH, CA 91311	
TAX PARCEL NO:	205-07-120	
TOTAL SITE AREA:	±15.00 AC.	
ZONING:	G-I	
EXISTING USE:	VACANT	
PROPOSED USE:	INDUSTRIAL (192,400 SF)	
FRONT YARD SETBACK:	15' (MEASURED FROM SIDEWALK)	
REAR YARD SETBACK:	10'	
SIDE YARD SETBACK:	10'	
PARKING CALCULATIONS:	OFFICE: 1 SPACE/350 SF 500 SF/350 SF = 2 SPACES	
	WAREHOUSE: 1 SPACE/4,000 SF 191,900 SF/4,000 SF = 48 SPACES	
TOTAL PARKING REQUIRED:	50 SPACES	1.
TOTAL PARKING PROVIDED:	±185 SPACES (6 ACCESSIBLE, 1 VAN INCLUDED)	2.
WATERSHED:	SUGAR	3.





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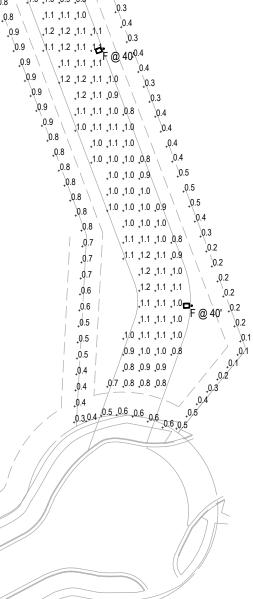
Statistics

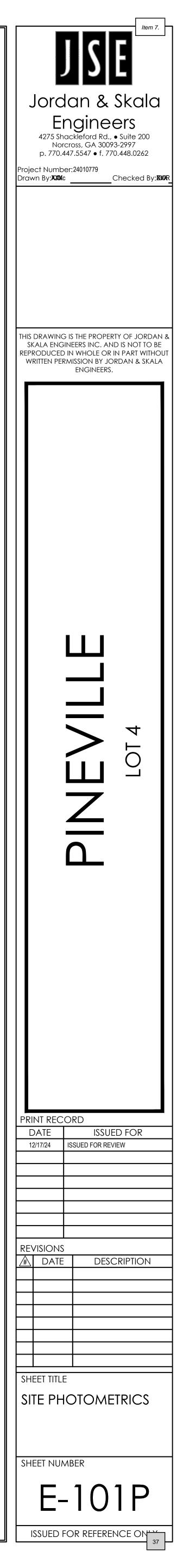
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min		
property line	-	0.3 fc	0.9 fc	0.0 fc	N/A	N/A		
PARKING	+	1.5 fc	3.4 fc	0.5 fc	6.8:1	3.0:1		

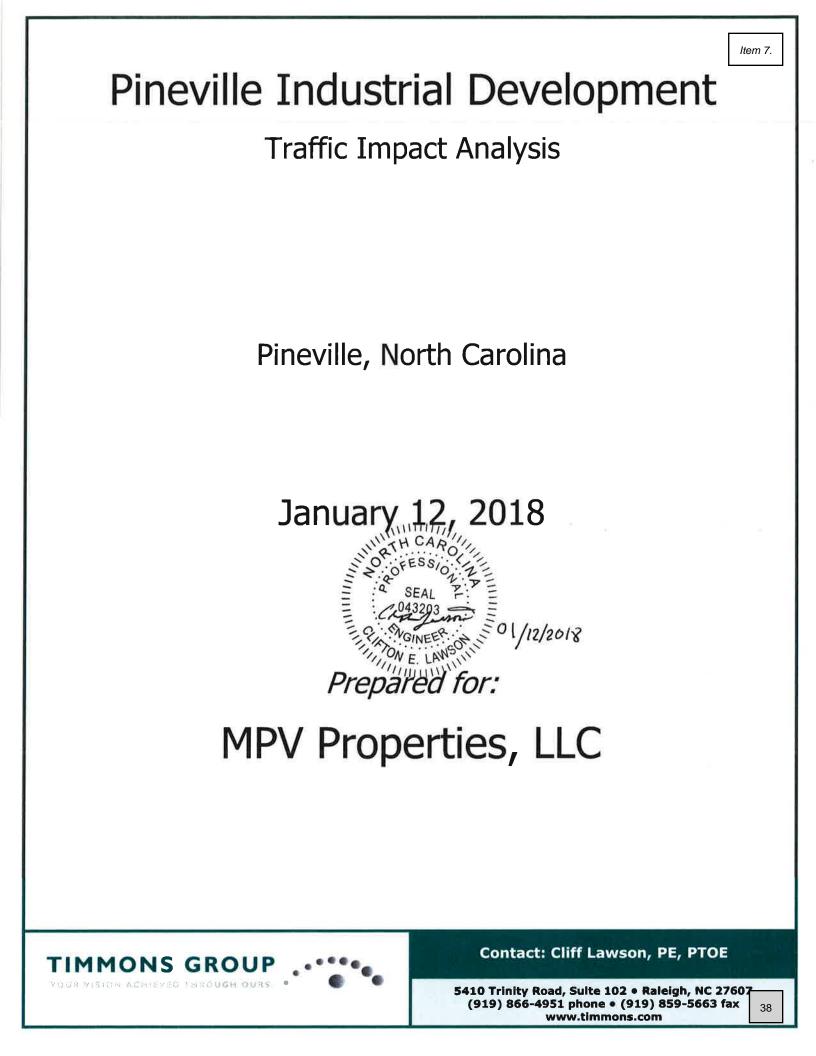
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Symbol	Label	QTY	Manufacturer Catalog Description	facturer Catalog Description	Vanufacturer Catalog Description	Manufacturer Catalog Description		Manufacturer Catalog Description	ifacturer Catalog Description		ufacturer Catalog Description	Description	Number Lamps	Lamp Output	LLF	Input Power
$\stackrel{\wedge}{\square}$	A	6	Lithonia Lighting	DSX2 LED P4 40K 70CRI T3M	D-Series Size 2 Area Luminaire P4 Performance Package 4000K CCT 70 CRI Type 3 Medium	1	35040	0.94	272.65							
	В	3	Lithonia Lighting	DSX2 LED P5 40K 70CRI TFTM	D-Series Size 2 Area Luminaire P5 Performance Package 4000K CCT 70 CRI Forward Throw	1	41069	0.94	326.58							
$\stackrel{\wedge}{\square}$	С	3	Lithonia Lighting	DSX2 LED P3 40K 70CRI TFTM	D-Series Size 2 Area Luminaire P3 Performance Package 4000K CCT 70 CRI Forward Throw	1	30068	0.94	219.4							
	D	1	Lithonia Lighting	DSX2 LED P4 40K 70CRI TFTM	D-Series Size 2 Area Luminaire P4 Performance Package 4000K CCT 70 CRI Forward Throw	1	35808	0.94	272.65							
	E	2	Lithonia Lighting	DSX2 LED P1 40K 70CRI T3M	D-Series Size 2 Area Luminaire P1 Performance Package 4000K CCT 70 CRI Type 3 Medium	1	19480	0.94	134.5							
•	G	2	Lithonia Lighting	DSX2 LED P3 40K 70CRI T2M HS	D-Series Size 2 Area Luminaire P3 Performance Package 4000K CCT 70 CRI Type 2 Medium Houseside Shield	1	25236	0.94	219.4011							
	F	5	Lithonia Lighting	DSX2 LED P1 40K 70CRI T2M HS	D-Series Size 2 Area Luminaire P1 Performance Package 4000K CCT 70 CRI Type 2 Medium Houseside Shield	1	16708	0.94	134.5029							

SITE PHOTOMETRICS







T/	ABLE OF CO	NTENTSI
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- Appendix E Approved Developments

1

This report presents the findings of the traffic impact analysis for the proposed Pineville Industrial Development (Phases I and II). The development will be located off Industrial Drive, in Pineville, NC (see **Figure 1-1**) and will consist of a 510,000 square-foot (SF) warehousing building to be constructed in 2019 as part of Phase I and a 340,000 SF industrial building to be constructed in 2024 as part of Phase II.

Analyses were completed for the 2017 Existing traffic volumes and the 2019 and 2024 (Phases I & II) Background and Build traffic volumes (background + site trips). The purpose of this assessment is as follows:

- 1. Verify that the existing geometry provided within the study area is sufficient to accommodate the projected traffic volumes; and
- 2. Determine what, if any, improvements are necessary at the proposed site driveway connection to Industrial Drive, the intersections of Industrial Drive / Pineville Road / Polk Street and Industrial Drive / Rodney Street, as well as the two railroad crossings of Industrial Drive.

The following steps were taken to determine the potential traffic impacts associated with this project:

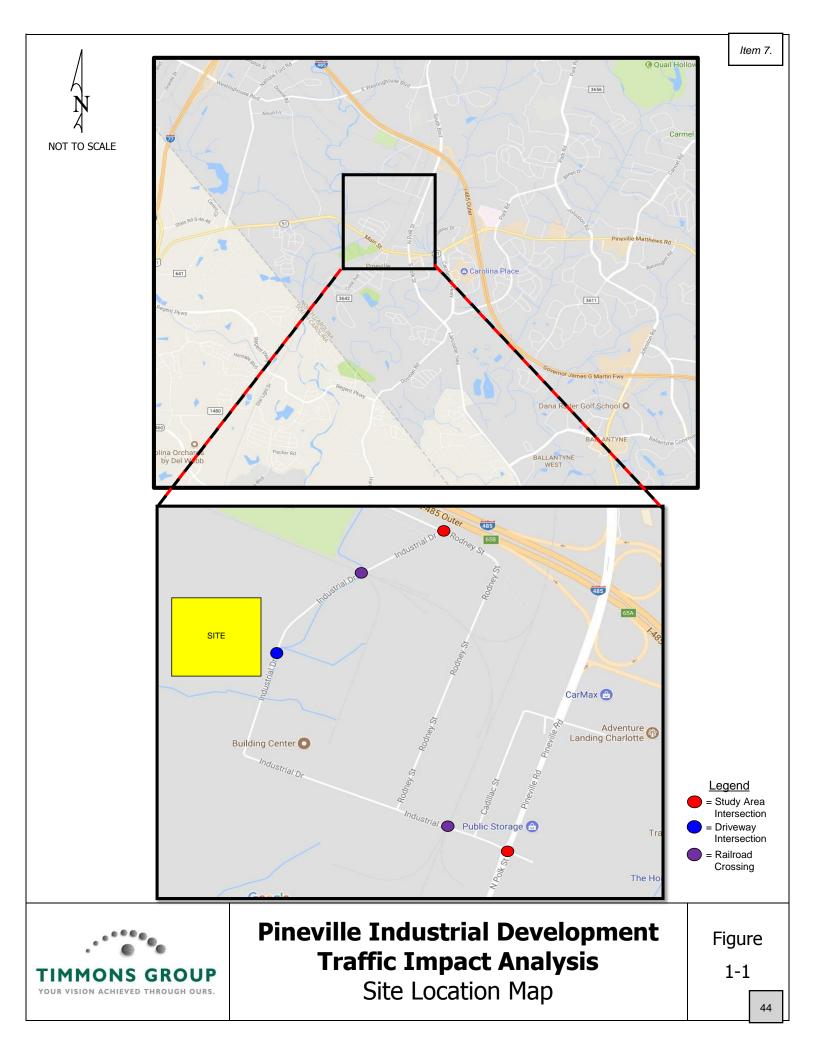
- 1. <u>Data Collection</u> AM (7:00 9:00) and PM (4:00 6:00) peak hour turning movement counts were collected in May and October 2017 at the following four (4) intersections / crossings:
 - Industrial Drive / Pineville Road / Polk Street (signalized);
 - Industrial Drive / Rodney Street (unsignalized);
 - Industrial Drive / Northern Railroad Crossing* (unsignalized); and
 - Industrial Drive / Southern Railroad Crossing*(signalized);

*Railroad Crossings of Industrial Drive.

- <u>Trip Generation/Future Traffic</u> Traffic generated by the proposed development was estimated using the 9th edition of the Institute of Transportation Engineers' <u>Trip Generation Manual</u>. Trip generation was calculated using the total square footage (510,000 SF & 340,000 SF respectively) as the independent variable, as well as the average rate and the equation (per NCDOT guidelines). Projected future traffic volumes were calculated using a 2% ambient growth rate and site trips from the adjacent residential development
- 3. <u>Trip Distribution and Projections</u> The distribution of site-generated trips was based on the distribution of existing area traffic. It was assumed, for purposes of analysis, that projected trips would follow the same patterns as existing traffic.
- 4. <u>Traffic Capacity Analysis</u> Level of service analyses were performed using SYNCHRO Version 9.1 (Build 912, Rev 4) for the following intersections:
 - Industrial Drive / Pineville Road / Polk Street;
 - Industrial Drive / Rodney Street; and
 - Site Driveway #1 / Industrial Drive.

Additionally, queue lengths along industrial drive were observed / recorded to determine if there were any impacts to the two railroad crossings with Industrial Drive.

- 5. <u>Queuing Analysis</u> The 95th percentile queue lengths from the capacity analyses were analyzed at the intersections listed above.
- 6. <u>Review of Proposed Improvements</u> Roadway / railroad crossing improvements proposed to accommodate projected site-generated traffic were evaluated (if applicable).



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2 **EXISTING INFORMATION**

The proposed development will be located off Industrial Drive west of Polk Street / Pineville Road, in Pineville, NC, as shown on Figure 1-1.

2.1 STUDY LIMITS

Access to the proposed site will be provided through one site driveway connection to the outside roadway network made via Industrial Drive (Site Driveway #1). Site Driveway #1 will be located approximately 2,500' (C/L to C/L) south of Rodney Street, approximately 1,650' (C/L to C/L) south of the northern railroad crossing, and approximately 2,715' (C/L to C/L) northwest of the southern railroad crossing. The northern railroad crossing is located approximately 875' (C/L to C/L) south of Rodney Street. Finally, the southern railroad crossing is located approximately 600' (C/L to C/L) west of Pineville Road / Polk Street.

The proposed entrance is shown graphically on Figure 2-1 (all figures are located at the end of their respective chapter). **Figure 2-2** includes the preliminary site layout for the industrial development.

The study limits include the following five (5) intersections / crossings:

- 1. Industrial Drive / Pineville Road / Polk Street
- 2. Industrial Drive / Rodney Street
- 3. Industrial Drive / Southern Railroad Crossing*
- 4. Industrial Drive / Northern Railroad Crossing*
- 5. Site Driveway #1 / Industrial Drive

*Existing railroad crossing of Industrial Drive.

2.2 EXISTING ROADWAYS

SR 4982 (Polk Street / Pineville Road) is a four-lane facility that runs north-south, east of the project study area. The facility has a posted 45-mph speed limit and serves residential and commercial developments as well as commuter traffic. Polk Street / Pineville Road stretches from downtown Charlotte (beginning as Caldwell Street) southward to US-521 (changing names to Lancaster Highway).

Industrial Drive is a two-lane facility that runs approximately north-south in front of the proposed site before turning east-west to intersect Pineville Road / Polk Street. The facility has a posted 35-mph speed limit and primarily services the existing industrial park. Industrial Drive runs from Rodney Street to the northwest to Polk Street / Pineville Road to the east.

Rodney Street is a two-lane facility that runs approximated east-west, north of the project study area. The facility has a posted 35-mph speed limit and primarily services the existing industrial park. Rodney Street runs from Industrial Drive in the south to E Westinghouse Boulevard in the northwest.

2.3 EXISTING INTERSECTIONS / RAILROAD CROSSINGS

Using available aerial imagery and site visits, Timmons Group compiled the existing geometry for each of the study area intersections. The existing intersection geometry is shown on Figure 2-3 and used throughout all analyses.

Polk Street / Pineville Road / Industrial Drive is an eight-phase signalized intersection with protected / permitted left-turn phasing for all four approaches. The north and southbound intersection approaches each include an exclusive left-turn lane, a through lane, and a shared through / right-turn lane. The east

Item 7.

and westbound approaches each include an exclusive left-turn lane and a shared through / right-turn lane.

Industrial Drive / Rodney Street is an unsignalized T-intersection with the northbound Industrial Drive approach encountering the stopped condition. The northbound approach consists of a shared left / right-turn lane. The eastbound approach consists of a shared through / right-turn lane. The westbound approach consists of a shared left-turn / through lane.

Industrial Drive / Northern Railroad Crossing is an unsignalized crossing including cross-buck signage denoting the crossing. At the crossing, Industrial Drive consists of a two-lane roadway section.

Industrial Drive / Southern Railroad Crossing is a signalized crossing including overhead flashers, gates, and cross-buck signage. At the crossing, Industrial Drive consists of a two-lane roadway section.

2.4 TRAFFIC VOLUMES

Timmons Group calculated peak hour volumes for the study area intersections using the AM (7:00 - 9:00) and PM (4:00 - 6:00) peak period turning movement counts undertaken in May and October 2017. Traffic count data is summarized in **Figure 2-4**. The complete traffic count data can be found in **Appendix A**.

2.5 AREA SAFETY REVIEW

Crash data for the past five-year period (2012–2017) was provided by the NCDOT. Per **Table 2-1** below, the intersection of Industrial Drive / Pineville Road / Polk Street had 18 reported accidents. Crash data for the intersection of Industrial Drive / Rodney Street, was provided in December and showed only one accident occurring in 2005. No fatal crashes were reported at the intersection of Polk Street / Pineville Road / Industrial Drive or Industrial / Rodney Street. A crash summary (provided in **Appendix B**) has been included in **Table 2-1** below summarizing the number of crashes, type of crash (injury / property damage), and year of occurrence.

Location	2012	2013	2014	2015	2016	2017	Injury	Property Damage
Polk Street / Pineville Road / Industrial Drive	2	4	7	8	4	3	10	18
Industrial Drive / Rodney Street	0	0	0	1	0	0	0	1

Table 2-1	: Crash	Information
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2.6 CAPACITY ANALYSIS

Using field observations, aerial photography, and traffic count data, traffic operations were analyzed during 2017 (existing) and 2019 / 2024 (without and with the proposed development site trips for Phases I & II).

Capacity analysis allows traffic engineers to determine the impacts of traffic on the surrounding roadway network. The Transportation Research Board's (TRB) *Highway Capacity Manual* (HCM) methodologies govern how the capacity analyses are conducted and how the results are interpreted. There are six letter grades of Levels of Service (LOS) from A to F, with LOS A representing the best operating conditions and LOS F the worst operating conditions. At signalized intersections, an overall intersection LOS E is generally considered unacceptable. At unsignalized intersections, a LOS E is generally considered acceptable only

if the side street encounters delay. Nevertheless, side streets typically function at a LOS F during peak traffic periods, because the traffic volumes often do not warrant a traffic signal to assist side street traffic. **Table 2-2** shows in detail how each of these levels of service are interpreted.

Level of	Roadway Segments or	lators actions	
Service	Controlled Access Highways	Intersections	
А	Free flow, low traffic	No vehicle waits longer than	
	density.	one signal indication.	Nº
В	Delay is not unreasonable,	On a rare occasion motorists	
	stable traffic flow.	wait through more than one signal indication.	
С	Stable condition,	Intermittently drivers wait	
	movements somewhat	through more than one signal	
	restricted due to higher	indication, and occasionally	
	volumes, but not	backups may develop behind	
	objectionable for motorists.	left turning vehicles, traffic	
		flow still stable and	
		acceptable.	
D	Movements more restricted,	Delays at intersections may	
	queues and delays may	become extensive with some,	
	occur during short peaks,	especially left-turning	
	but lower demands occur	vehicles waiting two or more	
	often enough to permit	signal indications, but	
	clearing, thus preventing	enough cycles with lower	
	excessive backups.	demand occur to permit	
		periodic clearance, thus	
		preventing excessive backups.	
E	Actual capacity of the	Very long queues may create	
	roadway invloves delay to	lengthly delays, especially for	
	all motorists due to	left-turning vehicles.	
	congestion.		
F	Forced flow with demand	Backups from locations	
	volumes greater than	downstream restrict or	
	capacity resulting in	prevent movement of vehicles	
	complete congestion.	out of approach creating a	
	Volumes drop to zero in	storage ares during part or	No o C
	extreme cases.	all of an hour.	
SOURCE: "	A Policy on Design of Design of Urb	oan Hiahways and Arterial	

Table 2-2: Level of Service Definitions

SOURCE: "A Policy on Design of Design of Urban Highways and Arterial Streets" - AASHTO, 1973 based upon material published in "Highway Capacity Manual", National Academy of Sciences, 1965.

For signalized and unsignalized intersections, level of service is defined in terms of **delay**, a measure of driver discomfort, frustration, fuel consumption and lost travel time. **Table 2-3** summarizes the delay associated with each LOS category:

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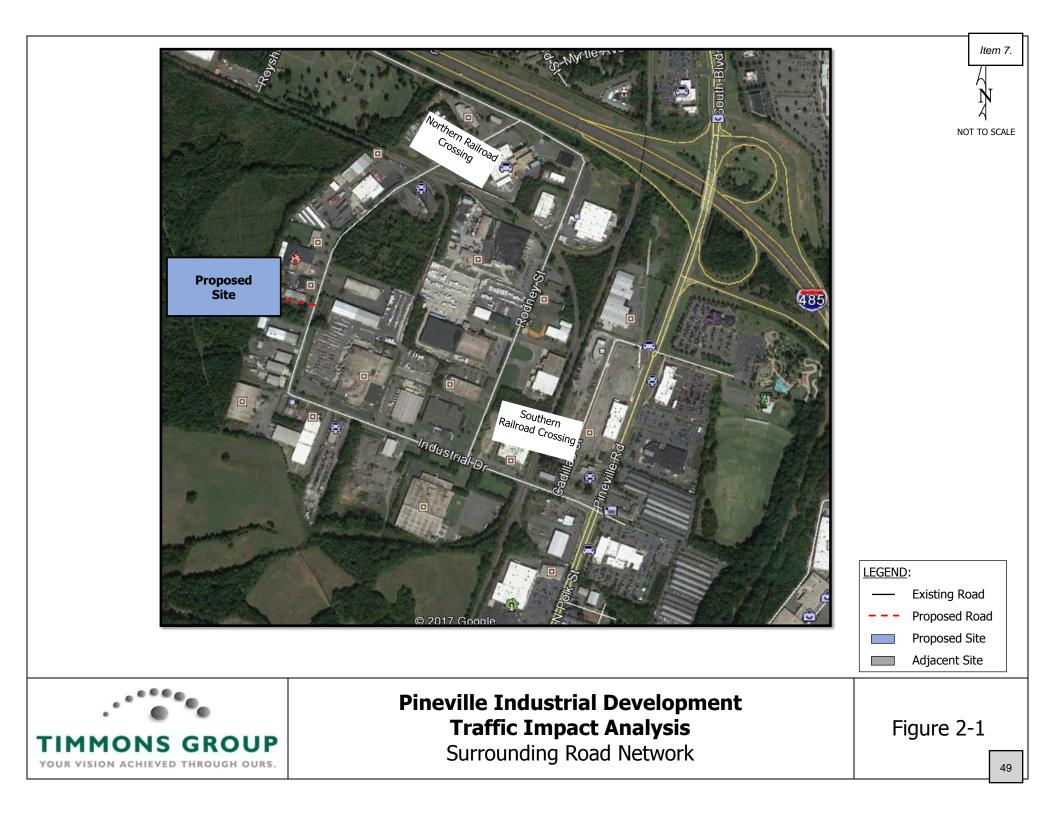
Signalize	ed Intersections	Unsignalized Intersections			
Level of Service	Control Delay per Vehicle (sec/veh)	Level of Service	Average Control Delay (sec/veh)		
А	≤ 10	А	0 to 10		
В	> 10 to ≤ 20	В	> 10 to \le 15		
С	> 20 to ≤ 35	С	> 15 to ≤ 25		
D	> 35 to ≤ 55	D	> 25 to \leq 35		
E	> 55 to ≤ 80	E	> 35 to \leq 50		
F	> 80	F	> 50		

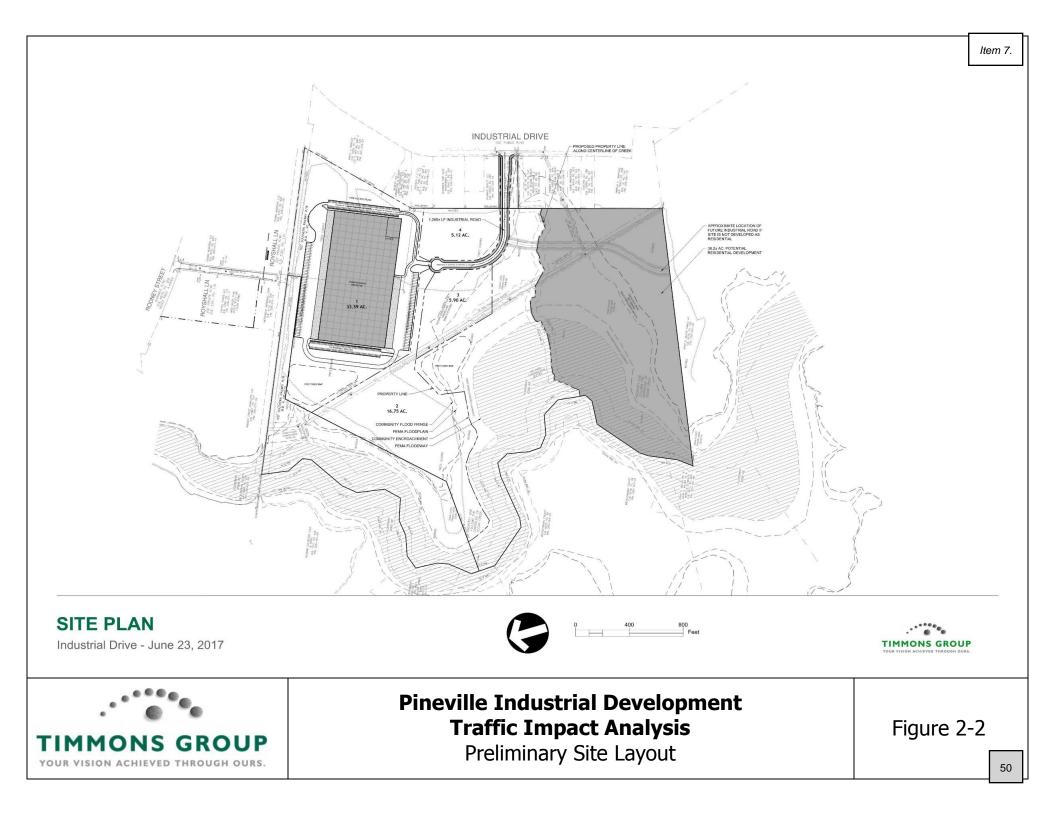
Table 2-3: Signalized and Unsignalized Intersection Level of Service Criteria

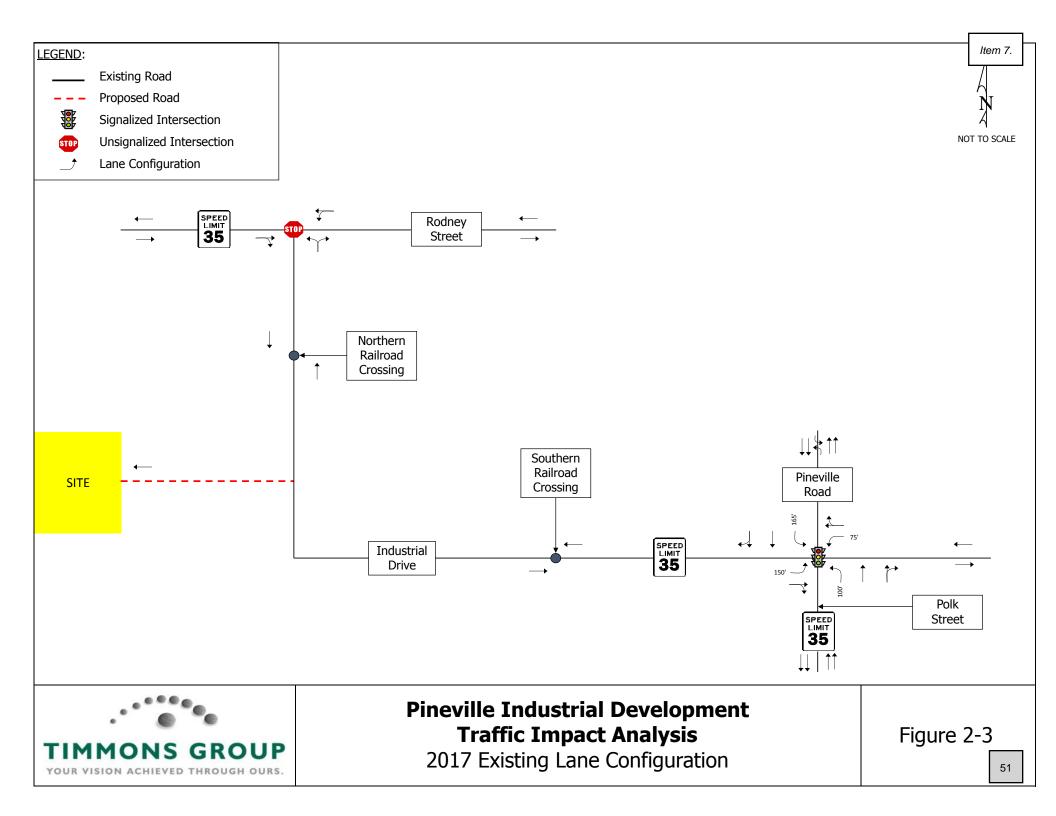
Source: Exhibit 16-2 and Exhibit 17-2 from TRB's "Highway Capacity Manual 2000"

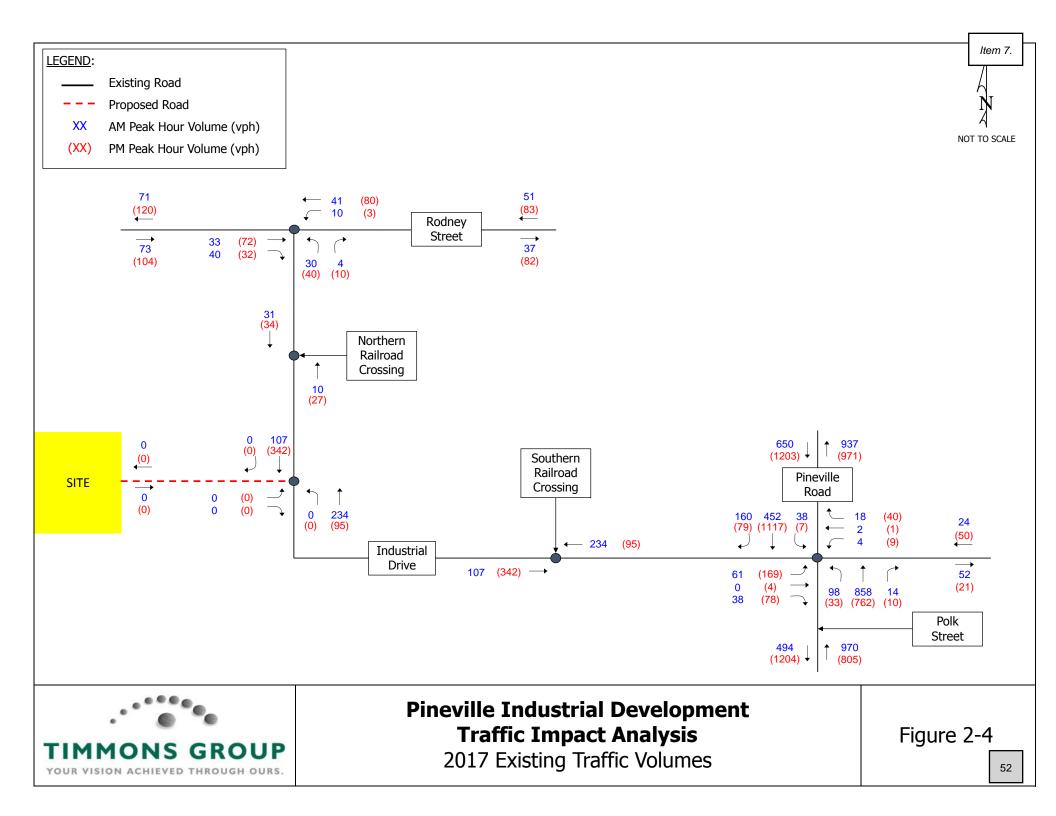
Capacity analyses were performed to assess operational conditions. Study area intersections were analyzed using SYNCHRO Version 9.1 (Build 912, Rev 4) based on Highway Capacity Manual (HCM) methodologies with the following assumptions:

- Existing grades;
- 12-foot lane widths;
- No parking activity, bus stops, or pedestrians;
- Peak hour factor (PHF) of 0.90;
- Heavy vehicle percentages 2%; and
- Existing green splits with timing values found in the provided traffic signal plans (see Appendix C).









3 EXISTING AND BACKGROUND CONDITIONS AND ANALYSIS

3.1 2017 EXISTING ANALYSES

Table 3-1 summarizes the 2017 Existing intersection LOS, delay, and 95th percentile queue lengths based on the geometry shown on **Figure 2-3** and the 2017 Existing traffic volumes shown on **Figure 2-4**. The corresponding SYNCHRO output is included in **Appendix D**.

The signalized intersection of Polk Street / Pineville Road / Industrial Drive is currently operating at a LOS B during both the AM and PM peak hours. During the PM peak hour, Synchro projects that the 95th percentile queue length for the eastbound left-turn lane (170-feet) exceeds available storage (150-feet). Existing turn-lane storage is adequate to handle all remaining 95th percentile queue lengths.

All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are currently operating at a LOS A during the AM and PM peak hours.

Table 3-1: Intersection Level of Service, Delay and 95th Percentile Queue Summary2017 Existing Traffic Volumes

			AM PEAK HOUR			PM PEAK HOUR			
Intersection and Type of Control	Movement and Approach	Turn Lane Storage (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	Delay ¹ (sec/veh)	PEAK H LOS ¹ C C C C C D D D A B B B B B B B B B B A 4 7 C C C C C C C C C C C C C C C C C C	95th Percentile Queue Length (ft)	
1. Polk Street /	EB Left	150	19.4	В	55	33.2	С	170	
Pineville Road (N-S)	EB Thru/Right		24.1	С	48	32.1	С	106	
at Industrial Drive (E-W)	EB Approach		21.2	С		32.9	С	222	
Signalized	WB Left	75	19.2	В	8	27.1	С	18	
92 	WB Thru/Right		29.4	С	32	41.3	D	63	
	WB Approach		27.9	С		38.7	D		
	NB Left	100	6.9	Α	43	7.9	А	20	
	NB Thru/Right		11.7	В	263	11.0	В	242	
	NB Approach		11.3	В		10.9	В		
	SB Left	165	6.8	A	21	7.1	A	7	
	SB Thru/Right		14.2	В	179	19.7	В	444	
	SB Approach		13.7	В		19.6	В		
	Overall		13.0	В		18.4	В		
2. Industrial Drive (N-S)	EB Thru/Right	-	0.0	Α	0	0.0	A	0	
at Rodney Street (E-W)	EB Approach		†	†		†	†	(111	
Unsignalized	WB Left/Thru		1.5	A	1	0.3	А	0	
2	WB Approach		†	1		†	†		
	NB Left/Right		9.3	A	3	9.7	А	5	
	NB Approach		†	†		†	†	122	

† SYNCHRO does not provide level of service or delay for unsignalized movements with no conflicting volumes.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

3.2 2021 BACKGROUND TRAFFIC VOLUMES

Currently there is one approved development in the project study area that will be partially or fully builtout by 2019 and 2024, respectively: Cranford Drive Residential Development (see **Appendix E**). Listed below is the approved development, site trip distribution assumptions, and proposed offsite improvements.

- Cranford Drive Residential Development
 - TIA completed by Timmons Group (sealed 8/25/17)
 - Located off Main Street in Pineville, NC
 - \circ $\;$ Assumed to be fully constructed prior to the Pineville Industrial Development* $\;$
 - 170 detached single-family residential units and 155 townhomes Land Use Codes (LUC) 210 and 230
 - One site driveway connection to Industrial Drive
 - Trip distribution found in existing TIA
 - No assumed offsite improvements

*The build analysis year for the Cranford Drive Residential TIA was 2021; however, to provide a more conservative analysis, it was assumed the development would be fully constructed prior to 2019.

Projected and distributed trips from the approved development (see **Appendix E**) were totaled and are shown in **Figure 3-1**. These trips were added to the 2019 ambient volumes (existing traffic volumes multiplied by a 2% growth factor – found in TIAs for adjacent studies) to determine the 2019 Phase I Background traffic volumes (see **Figure 3-2**). Similarly, approved development trips were added to the 2024 ambient volumes and 2019 Phase I Trip Distribution traffic volumes (see **Figure 4-1**) to determine the 2024 Phase II Background traffic volumes (see **Figure 3-3**).

3.3 2021 BACKGROUND ANALYSIS

Table 3-2a summarizes the 2019 Phase I Background intersection LOS, delay, and 95th percentile queue lengths based on the geometry shown in **Figure 2-3** and the 2019 Phase I Background traffic volumes shown in **Figure 3-2**. The corresponding SYNCHRO output is included in **Appendix D**.

The signalized intersection of Polk Street / Pineville Road / Industrial Drive is projected to operate at a LOS B during the 2019 Phase I Background AM peak hour and LOS C during the PM peak hour. During the PM peak hour, Synchro projects that the 95th percentile queue length for the eastbound left-turn lane (238-feet) will exceed available storage (150-feet). Existing turn-lane storage is adequate to handle all remaining 95th percentile queue lengths.

All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are projected to operate at a LOS A during the 2019 Phase I Background AM and PM peak hours.

Table 3-2a: Intersection Level of Service, Delay and 95th Percentile Queue Summary2019 Phase I Background Traffic Volumes

			AM	PEAK H	IOUR	PM PEAK HOUR			
Intersection and Type of Control	Movement and Approach	Turn Lane Storage (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	
1. Polk Street /	EB Left	150	23.1	С	89	43.3	D	#238	
Pineville Road (N-S)	EB Thru/Right		26.0	C	67	37.2	D	133	
at Industrial Drive (E-W)	EB Approach		24.1	С	-	41.3	D	222	
Signalized	WB Left	75	20.5	С	9	29.9	С	20	
393 	WB Thru/Right		32.0	С	34	45.1	D	71	
	WB Approach	2	30.3	С		42.5	D	-	
	NB Left	100	7.8	Α	48	8.2	А	26	
	NB Thru/Right		15.1	В	282	10.6	В	261	
	NB Approach		14.3	В		10.4	В	-	
	SB Left	165	7.3	Α	22	7.1	А	8	
	SB Thru/Right		15.6	В	196	22.7	С	527	
	SB Approach		15.1	В	-	22.6	С		
	Overall		15.7	В		21.1	С	ł	
2. Industrial Drive (N-S)	EB Thru/Right	-	0.0	A	0	0.0	А	0	
at Rodney Street (E-W)	EB Approach	8	†	†		†	†	14	
Unsignalized	WB Left/Thru		1.4	A	1	0.3	А	0	
	WB Approach		†	<i>t</i>		†	†		
	NB Left/Right	a	9.4	Α	5	9.8	А	6	
	NB Approach		<i>t</i>	†		†	†	122	

† SYNCHRO does not provide level of service or delay for unsignalized movements with no conflicting volumes.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

The signalized intersection of Polk Street / Pineville Road / Industrial Drive is projected to operate at a LOS B during the 2024 Phase II Background AM peak hour and LOS C during the PM peak hour. During the PM peak hour, Synchro projects that the 95th percentile queue length for the eastbound left-turn lane (279-feet) will exceed available storage (150-feet). Existing turn-lane storage is adequate to handle all remaining 95th percentile queue lengths.

All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are projected to operate at a LOS B or better during the 2019 Phase II Background AM and PM peak hours.

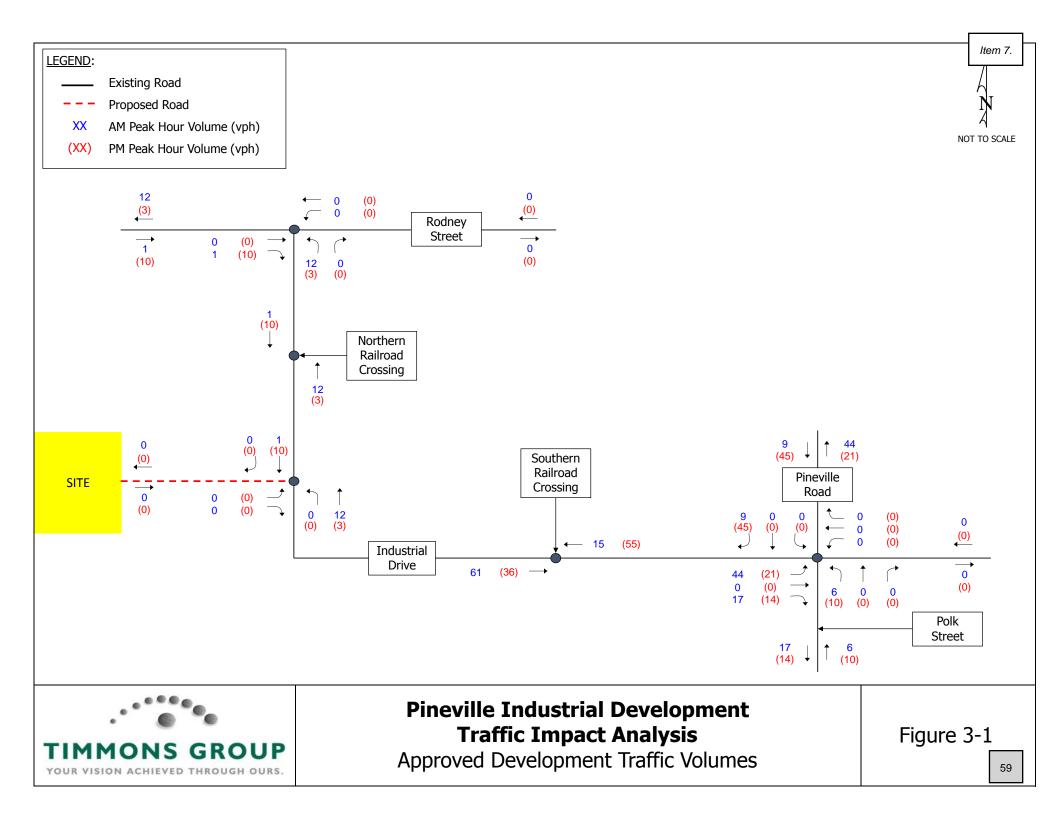
All unsignalized intersection movements at the intersection of Industrial Drive / Site Driveway #1 are projected to operate at a LOS B or better during the 2024 Phase II Background AM and PM peak hours.

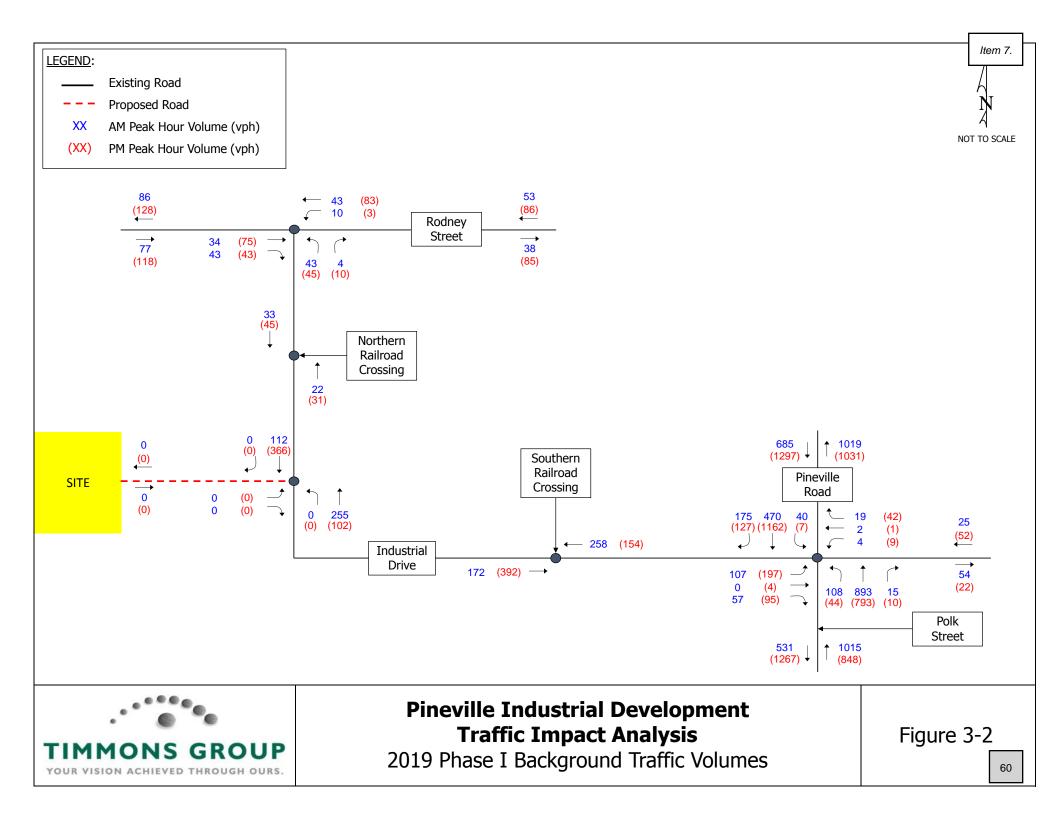
Table 3-2b: Intersection Level of Service, Delay and 95th Percentile Queue Summary2024 Phase II Background Traffic Volumes

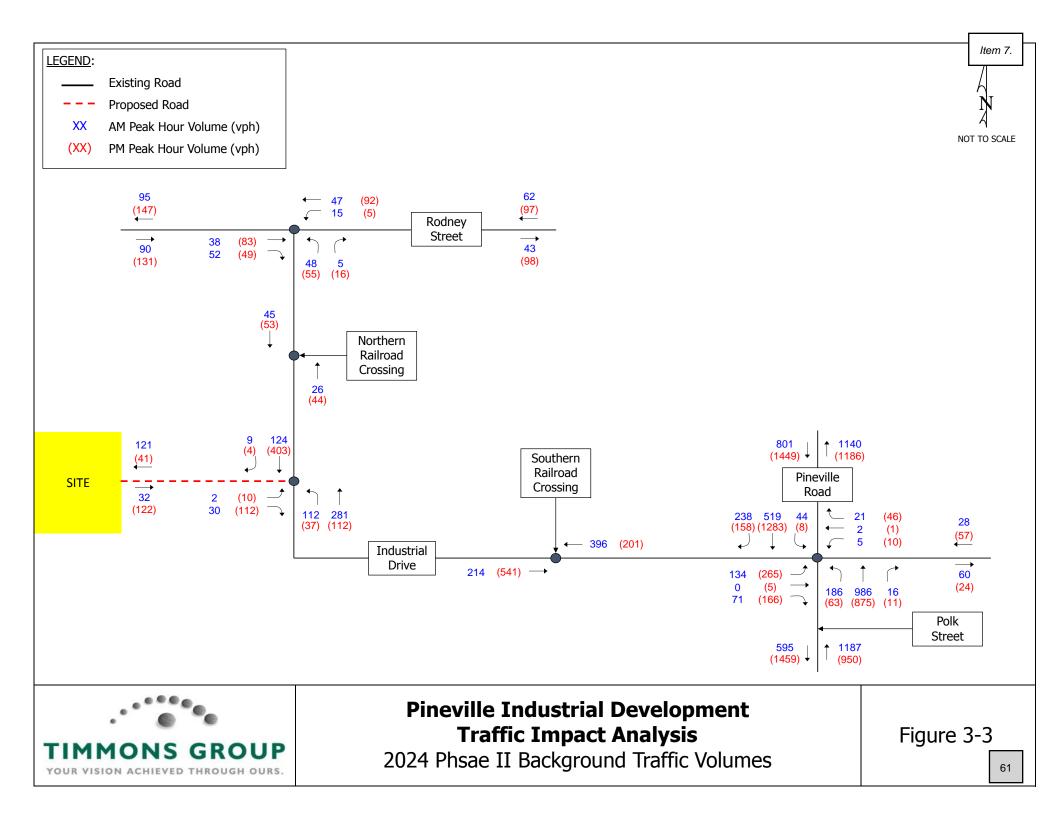
			AM	PEAK H	IOUR	PM	PEAK H	IOUR
Intersection and Type of Control	Movement and Approach	Turn Lane Storage (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	Delay ¹ (sec/veh)	PEAK H LOS ¹ E D E C D D B B B B B B B B C C C C C C C C C	95th Percentile Queue Length (ft)
1. Polk Street /	EB Left	150	28.0	С	117	64.5	E	#279
Pineville Road (N-S)	EB Thru/Right		29.2	С	84	44.3	D	248
at Industrial Drive (E-W)	EB Approach		28.4	С		56.5	E	222
Signalized	WB Left	75	23.0	С	12	32.9	С	21
82	WB Thru/Right		34.6	С	37	50.6	D	76
	WB Approach	2	32.4	С		47.5	D	
	NB Left	100	10.8	В	81	10.3	В	38
	NB Thru/Right		16.3	В	331	11.4	В	300
	NB Approach		15.4	В		11.3	В	
	SB Left	165	7.7	A	24	7.0	A	8
	SB Thru/Right		20.3	С	248	27.5	С	674
	SB Approach		19.6	В		27.4	С	ł
	Overall		18.3	В		26.9	С	-
2. Industrial Drive (N-S)	EB Thru/Right	2 2	0.0	Α	0	0.0	Α	0
at Rodney Street (E-W)	EB Approach	8 8	1	†		†	†	1
Unsignalized	WB Left/Thru		1.9	Α	1	0.4	Α	0
	WB Approach		1	<i>t</i>		†	†	1.77
	NB Left/Right		9.6	Α	6	10.1	В	8
	NB Approach		1	†		†	†	122
3. Industrial Drive (N-S)	EB Thru/Right		9.5	Α	3	13.0	В	22
at Site Driveway #1 (E-W)	EB Approach		1	†		†	†	1.000
Unsignalized	NB Left/Thru		2.8	Α	7	2.3	Α	3
	NB Approach	6	1	+		+	†	322
	SB Thru/Right		0.0	Α	0	0.0	Α	0
	SB Approach		+	+		+	+	8 70

† SYNCHRO does not provide level of service or delay for unsignalized movements with no conflicting volumes.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.







4 SITE TRIP GENERATION AND DISTRIBUTION

Site trips for the Pineville Industrial Development were estimated based on the proposed land use supplied by the developer and subsequently distributed onto the surrounding roadway network for each phase of construction.

4.1 TRIP GENERATION

The traffic generation potential of the proposed development was determined using the *ITE Trip Generation Manual* (Institute of Transportation Engineers, 9th Edition, 2012). **Tables 4-1a** and **4-1b** below list the ITE Land Use Code (LUC) and independent variable used for the development during Phase I and Phase II. Trip generation values were calculated using the total square footage (510,000 SF & 340,000 SF respectively) as the independent variable as well as the average rate and the equation (per NCDOT guidelines).

ITE Land Use Code	Independent	Daily			AM	Peak H	our	PM Peak Hour		
	Variable	In	Out	Total	In	Out	Total	In	Out	Total
510 – Warehousing	510,000 SF	908	908	1,816	121	32	153	41	122	163

Table 4-1a: Phase I Trip Generation Summary

SOURCE: Institute of Transportation Engineers' *Trip Generation Manual* 9th Edition (2012)

Phase I AM peak hour trips generated totaled 121 incoming and 32 outgoing where PM peak hour trips totaled 41 incoming and 122 outgoing. Average daily traffic (ADT) volumes generated by the development totaled 1,816 vehicles per day. No reduction in trips was included due to internal capture and/or pass-by trips.

ITE Land Use Code	Independent	Daily			AM Peak Hour			PM Peak Hour		
	Variable	In	Out	Total	In	Out	Total	In	Out	Total
510 – Warehousing	510,000 SF	908	908	1,816	121	32	153	41	122	163
110 – General Light Industrial	340,000 SF	1219	1219	2,438	274	37	311	39	289	328
	Total:	2,127	2,127	4,254	395	69	464	80	411	491

Table 4-2b: Phase II Trip Generation Summary

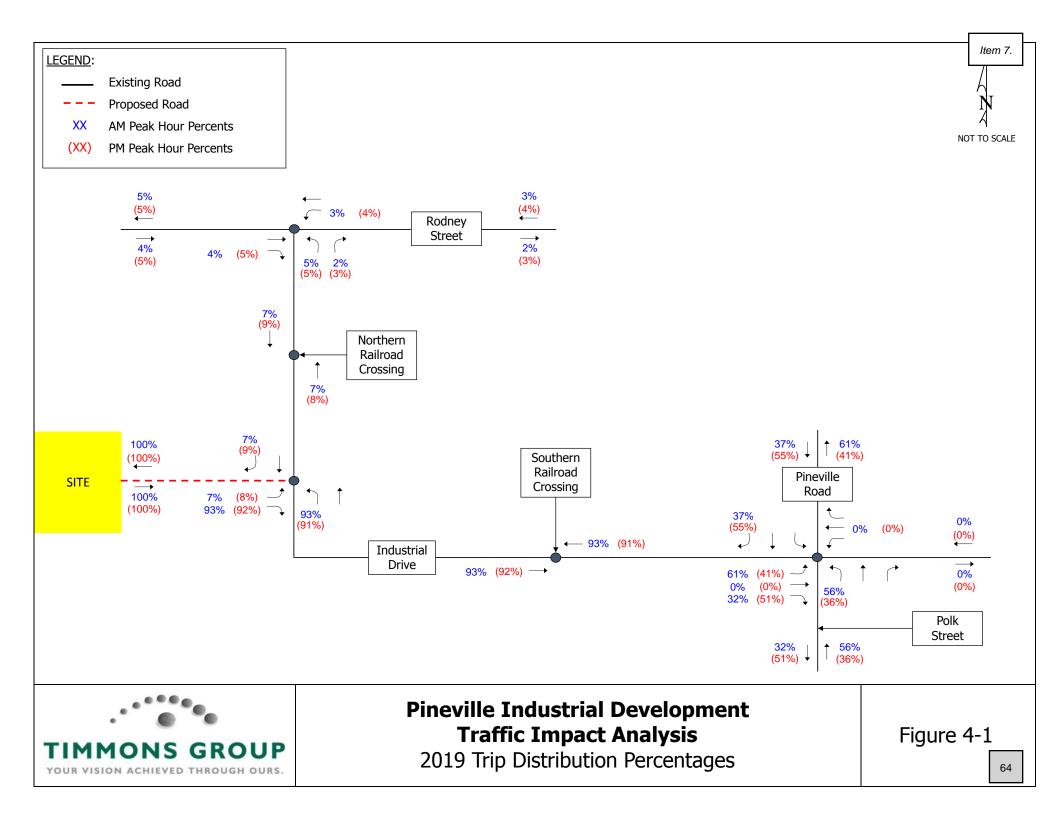
SOURCE: Institute of Transportation Engineers' *Trip Generation Manual* 9th Edition (2012)

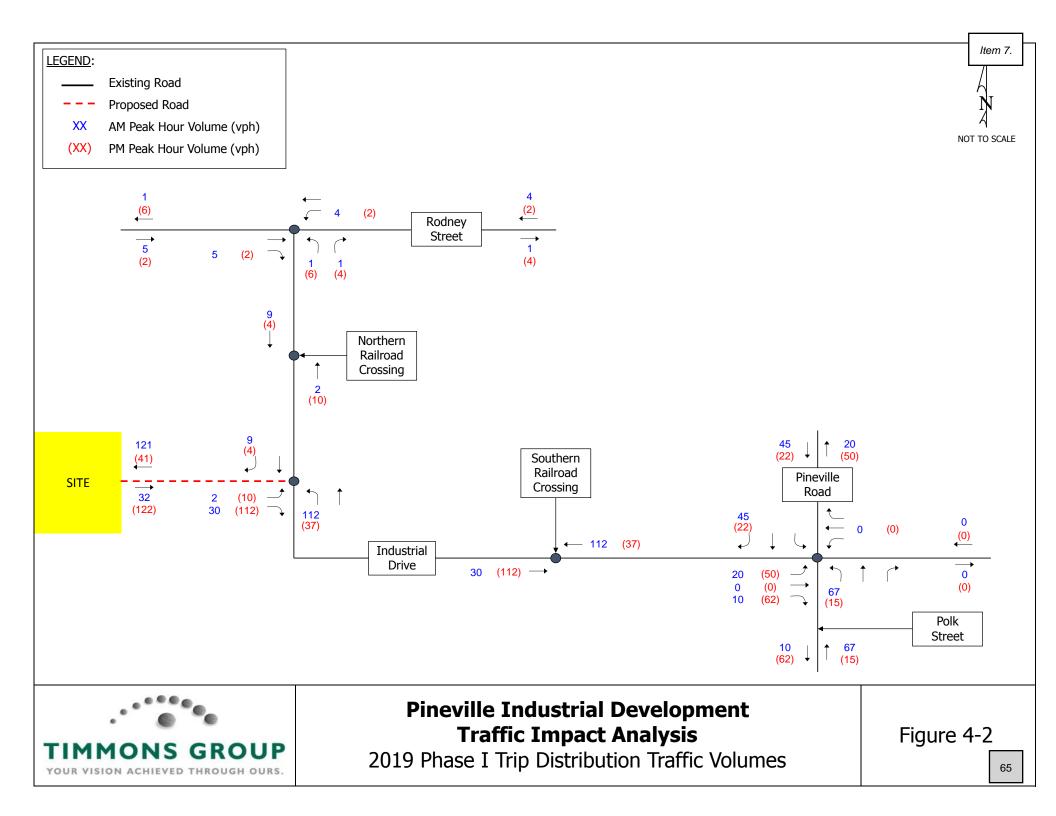
Phase II AM peak hour trips generated totaled 395 incoming and 69 outgoing where PM peak hour trips totaled 80 incoming and 411 outgoing. Average daily traffic (ADT) volumes generated by the development totaled 4,254 vehicles per day. No reduction in trips was included due to internal capture and/or pass-by trips.

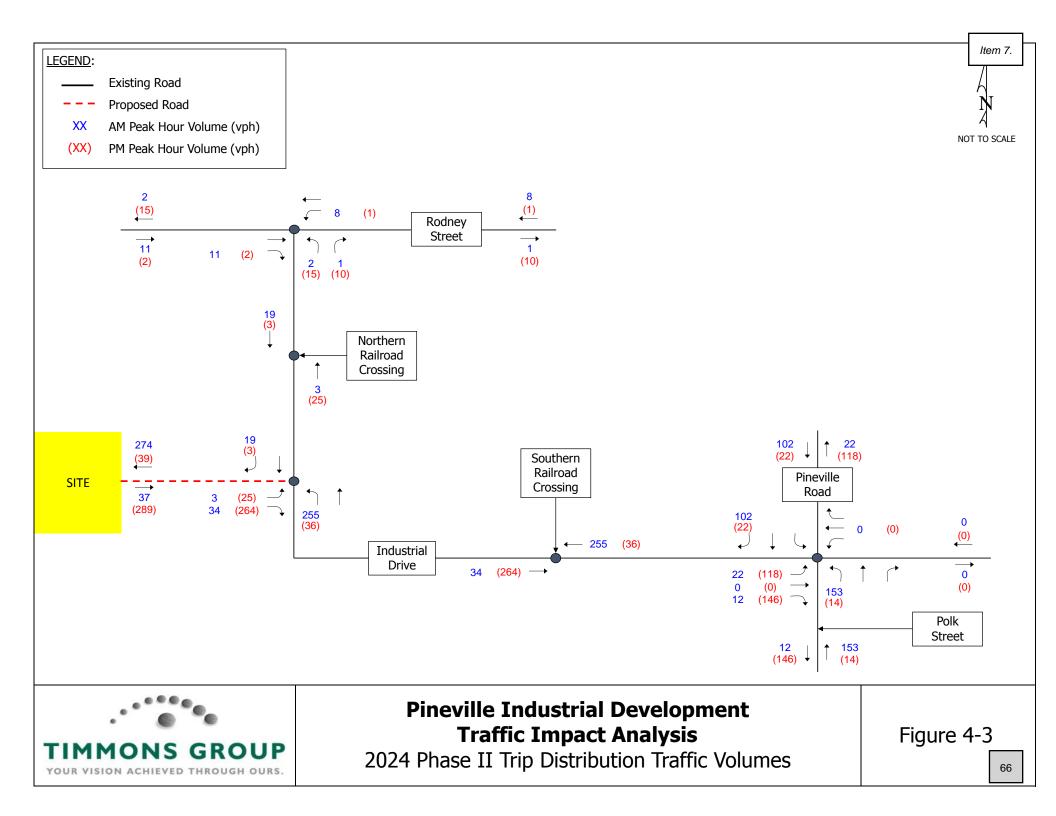
4.2 TRIP DISTRIBUTION

The directional traffic patterns, or trip distribution, of the site-generated traffic was determined using the existing AM and PM peak hour traffic characteristics. It was assumed, for purposes of this study, that all site traffic would enter and exit the study area in the same manner as the existing traffic. Area trip distribution is based on traffic counts performed by Timmons Group. Total trips into and out of the study area using Rodney Street, Industrial Drive, Polk Street, and Pineville Road form the basis for the percentage distribution. Distribution percentages into and out of the study area were calculated using existing traffic volumes entering and exiting the study area. The percentages were routed, via shortest path, to and from the proposed development. The distribution percentages were then applied to the generated trips to predict routes and project traffic volumes for the 2019 Phase I and 2024 Phase II build-

out scenarios. **Figure 4-1** shows the trip distribution percentages and **Figures 4-2** and **4-3** show the 2019 and 2024 Phases I and II site trip distribution volumes, respectively. 2019 Phase I Build traffic volumes were determined by applying the Phase I site trip distribution volumes to the 2019 Phase I Background traffic volumes (see **Figure 3-2**). Similarly, 2024 Phase II Build traffic volumes were determined by applying the Phase II site trip distribution volumes to the 2024 Phase II Background traffic volumes (see **Figure 3-2**). Similarly, 2024 Phase II Build traffic volumes were determined by applying the Phase II site trip distribution volumes to the 2024 Phase II Background traffic volumes (see **Figure 3-3**).







To complete the 2019 Phase I and 2024 Phase II Build analyses (including the proposed development), the estimated site trips were added to the 2019 Phase I and 2024 Phase II Background traffic volumes, respectively. The projected total volumes, along with the existing intersection geometry, were used to complete the capacity and turn lane warrant analyses.

5.1 PHASE I & II BUILD TRAFFIC VOLUMES

The 2019 Phase I Background traffic volumes from **Figure 3-2** were added to the Phase I projected site trips from the Pineville Industrial Development (**Figure 4-2**) to generate the 2019 Phase I Build traffic volumes (background + site) shown on **Figure 5-1**. Similarly, the 2024 Phase II Background traffic volumes from **Figure 3-3** were added to the Phase II projected site trips (**Figure 4-3**) to generate the 2024 Phase II Build traffic volumes shown on **Figure 5-2**.

5.2 PHASE I & II BUILD ANALYSIS

Table 5-1a summarizes the 2019 Phase I Build intersection LOS, delay, and 95th percentile queue lengths based on 2019 Phase I Build traffic volumes (shown on **Figure 5-1**).

The signalized intersection of Polk Street / Pineville Road / Industrial Drive is projected to operate at a LOS B during the 2019 Phase I Build AM peak hour and LOS C during the PM peak hour. During the PM peak hour, Synchro projects that the 95th percentile queue length for the eastbound left-turn lane (267-feet) will exceed available storage (150-feet). Existing turn-lane storage is adequate to handle all remaining 95th percentile queue lengths. Because this intersection is projected to operate at acceptable levels of service during both peak hours, no improvement recommendations are necessary to help mitigate intersection congestion due to the construction of Phase I of the proposed development.

All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are projected to operate at a LOS A during the 2019 Phase I Build AM and PM peak hours. Because all intersection movements are projected to operate at acceptable levels of service during both peak hours, no improvement recommendations are necessary to help mitigate intersection congestion due to the construction of Phase I of the proposed development.

All unsignalized intersection movements at the intersection of Industrial Drive / Site Driveway #1 are projected to operate at a LOS A during the 2019 Phase I Build AM and PM peak hours. No improvements are recommended to help mitigate future capacity concern at the proposed site driveway due to the construction of Phase I of the proposed development. Although Industrial Drive is not an NCDOT owned facility, Timmons Group followed standard NCDOT practices to determine the need for an exclusive turn-lane into the proposed site. Per standard NCDOT Policy on Street and Driveway Access to North Carolina Highways Manual:

"Generally left and right turn lanes and tapers shall be considered when:

• In accordance with G.S. 136-18(29), the average daily traffic meets or exceeds 4,000 vehicles per day on any secondary route (the average daily traffic should include both the existing traffic plus traffic generated by the proposed development)"

With the projected AADT volumes along Industrial Drive not expecting to exceed 4,000 VPD, the construction of turn lanes is not warranted.

Table 5-1a: Intersection Level of Service, Delay and 95th Percentile Queue Summary2019 Phase I Build Traffic Volumes

а. 			AM	PEAK H	IOUR	PM PEAK HOUR			
Intersection and Type of Control	Movement and Approach	Turn Lane Storage (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	
1. Polk Street /	EB Left	150	24.1	С	104	53.3	D	#267	
Pineville Road (N-S)	EB Thru/Right		26.3	C	76	41.5	D	#224	
at Industrial Drive (E-W)	EB Approach		24.8	С		48.6	D	222	
Signalized	WB Left	75	20.8	C	9	30.7	С	20	
20 	WB Thru/Right		32.0	C	34	46.2	D	72	
	WB Approach	3	30.3	С		43.5	D		
	NB Left	100	9.4	Α	76	8.9	Α	33	
	NB Thru/Right		15.2	В	286	10.7	В	265	
	NB Approach		14.3	В		10.5	В		
	SB Left	165	7.5	Α	22	7.1	Α	8	
	SB Thru/Right		19.2	В	220	24.3	С	571	
	SB Approach		18.5	В		24.3	С	ł	
	Overall		17.0	В		23.9	С	-	
2. Industrial Drive (N-S)	EB Thru/Right	-	0.0	A	0	0.0	A	0	
at Rodney Street (E-W)	EB Approach	2	†	†		†	†	(14	
Unsignalized	WB Left/Thru		1.9	Α	1	0.5	Α	0	
	WB Approach		†	1		†	†		
	NB Left/Right		9.5	Α	5	9.9	Α	8	
	NB Approach		1	†		†	†	(12	
3. Industrial Drive (N-S)	EB Thru/Right		9.4	A	3	12.4	В	21	
at Site Driveway #1 (E-W)	EB Approach		†	1		†	†	1.000	
Unsignalized	NB Left/Thru		2.9	Α	7	2.4	A	3	
	NB Approach		+	+		+	†	844	
	SB Thru/Right		0.0	Α	0	0.0	Α	0	
	SB Approach		+	†		†	†	9 75	

† SYNCHRO does not provide level of service or delay for unsignalized movements with no conflicting volumes.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Table 5-1b summarizes the 2024 Phase II Build intersection LOS, delay, and 95th percentile queue lengths based on 2024 Phase II Build traffic volumes (shown on **Figure 5-2**).

The signalized intersection of Polk Street / Pineville Road / Industrial Drive is projected to operate at a LOS C during the 2024 Phase II Build AM peak hour and LOS D during the PM peak hour. During the PM peak hour, Synchro projects that the 95th percentile queue length for the eastbound left-turn lane (508-feet) will exceed available storage (150-feet). Additionally, Synchro projects that the 95th percentile queue length for the northbound left-turn lane (363-feet) will exceed available storage (100-feet) during the AM peak hour. Existing turn-lane storage is adequate to handle all remaining 95th percentile queue lengths. Because this intersection is projected to operate at acceptable levels of service during both peak hours, no improvement recommendations are necessary to help mitigate intersection congestion due to the construction of Phase II of the proposed development.

All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are projected to operate at a LOS B or better during the 2024 Phase II Build AM and PM peak hours. Because all intersection movements are projected to operate at acceptable levels of service during both peak hours, no improvement recommendations are necessary to help mitigate intersection congestion due to the construction of Phase II of the proposed development.

All unsignalized intersection movements at the intersection of Industrial Drive / Site Driveway #1 are projected to operate at a LOS D or better during the 2024 Phase II Build AM and PM peak hours. No improvements are recommended to help mitigate future capacity concern at the proposed site driveway due to the construction of Phase II of the proposed development. Although Industrial Drive is not an NCDOT owned facility, Timmons Group followed standard NCDOT practices to determine the need for an exclusive turn-lane into the proposed site. Per standard NCDOT Policy on Street and Driveway Access to North Carolina Highways Manual:

"Generally left and right turn lanes and tapers shall be considered when:

• In accordance with G.S. 136-18(29), the average daily traffic meets or exceeds 4,000 vehicles per day on any secondary route (the average daily traffic should include both the existing traffic plus traffic generated by the proposed development)"

With the projected AADT volumes along Industrial Drive not expecting to exceed 4,000 VPD, the construction of turn lanes is not warranted.

Table 5-2b: Intersection Level of Service, Delay and 95th Percentile Queue Summary2024 Phase II Build Traffic Volumes

			AM	PEAK H	IOUR	PM PEAK HOUR			
Intersection and Type of Control	Movement and Approach	Turn Lane Storage (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	Delay ¹ (sec/veh)	LOS 1	95th Percentile Queue Length (ft)	
1. Polk Street /	EB Left	150	30.9	С	136	142.8	F	#508	
Pineville Road (N-S)	EB Thru/Right		30.3	C	96	73.9	E	#546	
at Industrial Drive (E-W)	EB Approach	8	30.7	С	-	111.6	F	222	
Signalized	WB Left	75	23.2	C	12	33.6	С	21	
87	WB Thru/Right		34.9	С	37	52.2	D	76	
	WB Approach	2	32.6	С		49.0	D		
	NB Left	100	46.7	D	#363	14.5	В	57	
	NB Thru/Right		16.0	В	338	11.6	В	300	
	NB Approach		23.8	С		11.9	В	-	
	SB Left	165	7.8	A	25	7.0	Α	8	
	SB Thru/Right		22.8	С	301	29.7	С	#697	
	SB Approach		22.1	С		29.6	С	ł	
	Overall		23.9	С		42.6	D	-	
2. Industrial Drive (N-S)	EB Thru/Right	2 2	0.0	Α	0	0.0	Α	0	
at Rodney Street (E-W)	EB Approach	8	1	†		†	†	1	
Unsignalized	WB Left/Thru		2.6	Α	1	0.5	Α	0	
	WB Approach		†	<i>t</i>		†	†	1.000	
	NB Left/Right		9.8	Α	6	10.3	В	12	
	NB Approach		1	1		†	†	122	
3. Industrial Drive (N-S)	EB Thru/Right		11.7	В	11	31.5	D	193	
at Site Driveway #1 (E-W)	EB Approach		1	†		†	†	1.000	
Unsignalized	NB Left/Thru		6.2	Α	30	3.8	A	6	
	NB Approach		1	†		+	†	34	
	SB Thru/Right		0.0	Α	0	0.0	Α	0	
	SB Approach		†	1		+	+	3 .77	

† SYNCHRO does not provide level of service or delay for unsignalized movements with no conflicting volumes.

- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

5.3 RAILROAD CROSSING

Due to the proximity of multiple railroad crossings (along Industrial Drive) to the proposed site, Timmons Group evaluated the need for any crossing improvements due to the construction of the proposed site. Currently, there are two railroad crossings within close proximity of the proposed development.

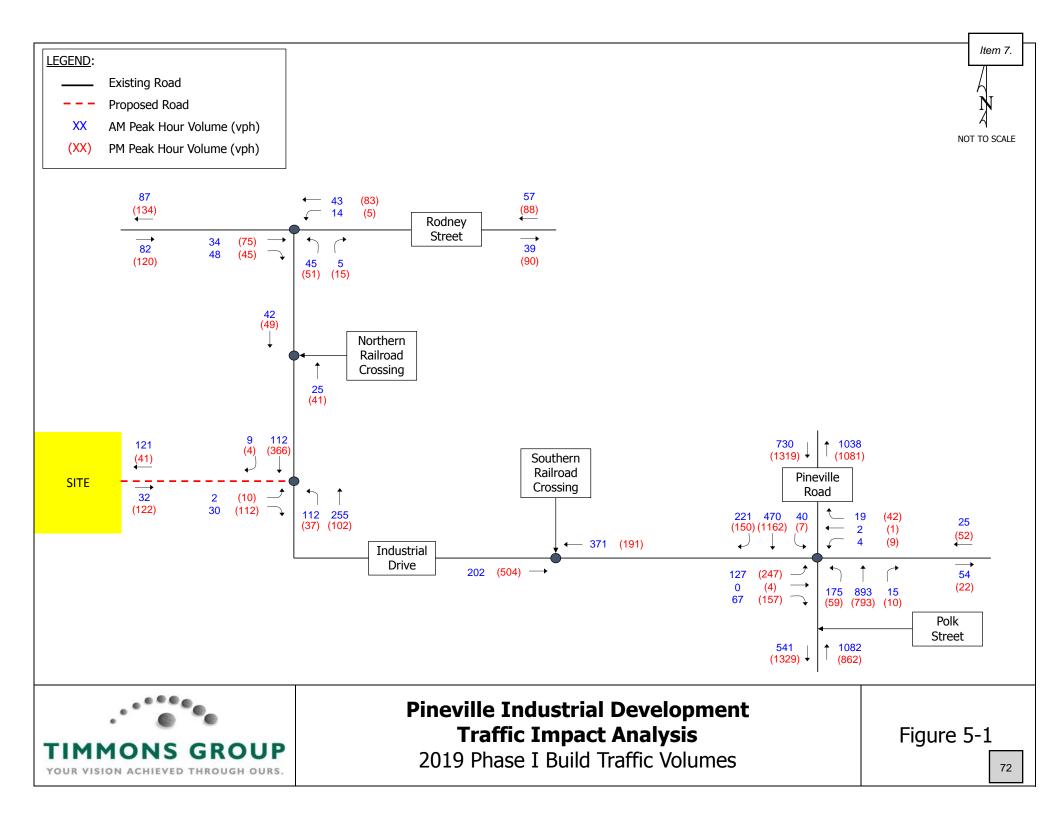
As mentioned earlier in the document, the unsignalized northern railroad crossing includes cross-buck signage for north and southbound drivers to denote the existing crossing. The signalized southern railroad crossing includes overhead flashers, gates, and cross-buck signage for east and westbound drivers to denote the existing crossing. Site Driveway #1 will be located approximately 1,650' (C/L to C/L) south of the northern railroad crossing and approximately 2,715' (C/L to C/L) northwest of the southern railroad crossing. The northern railroad crossing is located approximately 875' (C/L to C/L) south of Rodney Street. Finally, the southern railroad crossing is located approximately 600' (C/L to C/L) west Pineville Road / Polk Street.

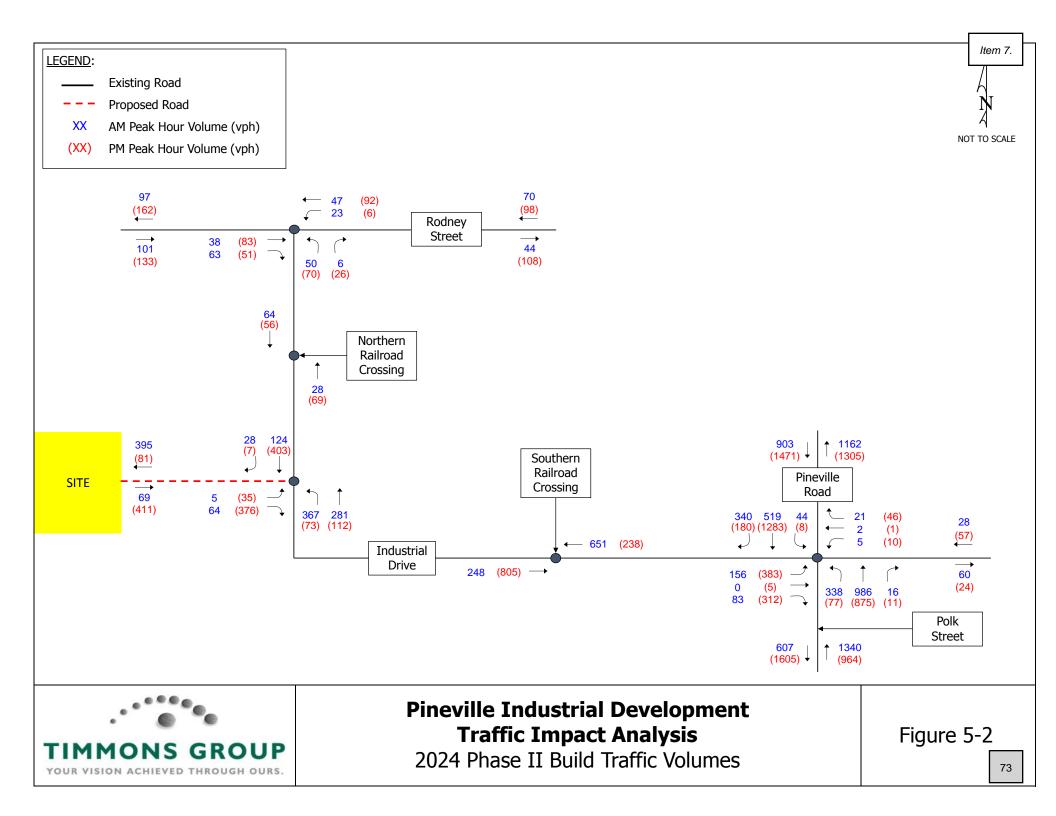
Per **Tables 5-1a** and **5-1b**, Synchro projects that the following:

- Site Driveway #1 / Industrial Drive
 - Shared northbound left-turn / through movement 95th percentile queue length projected not to exceed 6-feet during any peak hour for Phases I and II.
 - Shared southbound through / right-turn movement 95th percentile queue length projected to be 0-feet during both peak hours for Phases I and II.
- Industrial Drive / Rodney
 - Shared northbound left/right-turn movement 95th percentile queue length projected not to exceed 12-feet during any peak hour for Phases I and II.
- Industrial Drive / Pineville Road / Polk Street
 - Exclusive eastbound left-turn movement 95th percentile queue length projected not to exceed 508-feet during any peak hour for Phases I and II.
 - Shared eastbound through / right-turn movement 95th percentile queue length projected not to exceed 546-feet during any peak hour for Phases I and II.

Even though the queuing adjacent to the northern railroad crossing is expected to be minimal (northbound queues at Rodney Street or southbound queues at Site Driveway #1), it is recommended that stop bars be repainted and additional warning signs be placed at the existing crossing to help mitigate any potential safety concerns due to the construction of the proposed development.

Because Synchro projects that eastbound vehicles could (potentially) spillback (from Pineville Road / Polk Street) to the southern railroad crossing, it is recommended that stop bars be repainted and additional warning signs be placed at the existing crossing to help mitigate any potential safety concerns due to the construction of the proposed development. As mentioned earlier, the southern railroad crossing currently has significant enhancements (overhead flashing, crossing gates, etc.). Following the improvements mentioned above, adequate protection should exist for both vehicles and trains to allow for the crossing to operate safely and efficiently.





6 CONCLUSIONS AND RECOMMENDATIONS

Capacity analyses were performed for 2017 Existing, 2019 Phase I Background (existing + ambient growth + approved development trips), 2024 Phase II Background (existing + ambient growth + Phase I site trips + approved development trips), 2019 Phase I Build (Phase I Background + site trips), and 2024 Phase II Build (Phase II Background + site trips) traffic volumes.

Based on the operational analyses the following is offered:

- The signalized intersection of Polk Street / Pineville Road / Industrial Drive is projected to operate at a LOS D or better during the 2019 Phase I and 2024 Phase II Build AM and PM peak hours. No improvements are recommended to help mitigate future capacity concern at the proposed site driveway.
- All unsignalized intersection movements at the intersection of Industrial Drive / Rodney Street are projected to operate at a LOS B or better during the 2019 Phase I and 2024 Phase II Build AM and PM peak hours. No improvements are recommended to help mitigate future capacity concern at the proposed site driveway.
- All unsignalized intersection movements at Industrial Drive / Site Driveway #1 are projected to operate at a LOS D or better during the 2019 Phase I and 2024 Phase II AM and PM peak hours. No improvements are recommended to help mitigate future capacity concern at the proposed site driveway.
- Queuing is not projected to affect operations at the Industrial Drive / Northern Railroad crossing.
- Queueing is projected to affect operations at Industrial Drive / Southern Railroad crossing.

In closing, the following improvements are recommended in conjunction with the construction of the proposed development:

- Industrial Drive / Northern Railroad Crossing:
 - Installation of stop bars (Phase I); and
 - Installation of additional warning signage (Phase I).
- Industrial Drive / Southern Railroad Crossing:
 - Installation of stop bars (Phase I); and
 - Installation of additional warning signage (Phase I).

Item 7.

Appendix A – Traffic Counts

1202 Langdon Terrace Drive Raleigh, NC, 27615 Item 7.

File Name : Pineville(Industrial and Polk) AM Peak

Site Code :

Start Date : 5/25/2017

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									i aye	INU							
						G	roups F	Printed-C	ars + - ⁻	Trucks							
		Pinevill	e Road	ł		Industri	ial Drive	e		Polk	Street			Industri	ial Driv	е	
		South	bound			West	bound			North	bound			East	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
07:00	40	66	3	109	2	0	1	3	5	160	19	184	15	1	26	42	338
07:15	38	129	11	178	10	0	1	11	4	222	36	262	10	0	12	22	473
07:30	24	93	7	124	1	1	0	2	5	219	13	237	9	0	19	28	391
07:45	51	129	16	196	3	0	2	5	4	235	34	273	6	0	14	20	494
Total	153	417	37	607	16	1	4	21	18	836	102	956	40	1	71	112	1696
08:00	47	101	4	152	4	1	1	6	1	182	15	198	13	0	16	29	385
08:15	29	150	3	182	6	1	1	8	2	210	11	223	12	1	26	39	452
08:30	13	109	8	130	2	0	0	2	2	180	15	197	4	1	15	20	349
08:45	26	132	9	167	6	1	0	7	3	138	12	153	9	6	20	35	362
Total	115	492	24	631	18	3	2	23	8	710	53	771	38	8	77	123	1548
Grand Total	268	909	61	1238	34	4	6	44	26	1546	155	1727	78	9	148	235	3244
Apprch %	21.6	73.4	4.9		77.3	9.1	13.6		1.5	89.5	9		33.2	3.8	63		
Total %	8.3	28	1.9	38.2	1	0.1	0.2	1.4	0.8	47.7	4.8	53.2	2.4	0.3	4.6	7.2	
Cars +	248	905	61	1214	33	4	6	43	26	1543	153	1722	77	9	128	214	3193
% Cars +	92.5	99.6	100	98.1	97.1	100	100	97.7	100	99.8	98.7	99.7	98.7	100	86.5	91.1	98.4
Trucks	20	4	0	24	1	0	0	1	0	3	2	5	1	0	20	21	51
% Trucks	7.5	0.4	0	1.9	2.9	0	0	2.3	0	0.2	1.3	0.3	1.3	0	13.5	8.9	1.6

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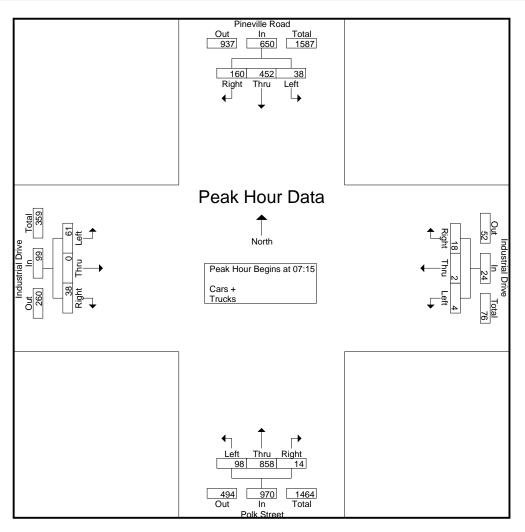
1202 Langdon Terrace Drive Raleigh, NC, 27615

File Name : Pineville(Industrial and Polk) AM Peak Site Code :

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		Pinevil				Industri	al Driv	e		Polk	Street			Industr	ial Drive	Э	
		South	bound			West	bound			North	bound			Eastl	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 to 0	8:45 - P	eak 1 of 1													
Peak Hour for Ent	ire Interse	ection Be	gins at C)7:15													
07:15	38	129	11	178	10	0	1	11	4	222	36	262	10	0	12	22	473
07:30	24	93	7	124	1	1	0	2	5	219	13	237	9	0	19	28	391
07:45	51	129	16	196	3	0	2	5	4	235	34	273	6	0	14	20	494
08:00	47	101	4	152	4	1	1	6	1	182	15	198	13	0	16	29	385
Total Volume	160	452	38	650	18	2	4	24	14	858	98	970	38	0	61	99	1743
% App. Total	24.6	69.5	5.8		75	8.3	16.7		1.4	88.5	10.1		38.4	0	61.6		
PHF	.784	.876	.594	.829	.450	.500	.500	.545	.700	.913	.681	.888	.731	.000	.803	.853	.882



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Item 7.

File Name : Pineville(Industrial and Polk) PM Peak

Site Code :

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									гауе								
						G	roups F	Printed- C	ars +	Trucks							
		Pinevil	le Road	ł		Industr	ial Driv	e		Polk	Street			Industr	ial Driv	е	
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
16:00	21	193	2	216	13	2	1	16	3	158	9	170	21	3	43	67	469
16:15	18	251	2	271	6	0	3	9	3	137	3	143	27	4	35	66	489
16:30	11	241	6	258	11	0	0	11	1	158	11	170	43	1	76	120	559
16:45	7	261	3	271	7	0	1	8	1	155	4	160	28	0	50	78	517
Total	57	946	13	1016	37	2	5	44	8	608	27	643	119	8	204	331	2034
17:00	20	255	2	277	13	1	4	18	2	220	8	230	31	0	59	90	615
17:15	16	277	1	294	12	0	3	15	3	213	10	226	15	3	45	63	598
17:30	24	282	3	309	11	0	0	11	2	152	9	163	19	1	49	69	552
17:45	19	303	1	323	4	0	2	6	3	177	6	186	13	0	16	29	544
Total	79	1117	7	1203	40	1	9	50	10	762	33	805	78	4	169	251	2309
Grand Total	136	2063	20	2219	77	3	14	94	18	1370	60	1448	197	12	373	582	4343
Apprch %	6.1	93	0.9		81.9	3.2	14.9		1.2	94.6	4.1		33.8	2.1	64.1		
Total %	3.1	47.5	0.5	51.1	1.8	0.1	0.3	2.2	0.4	31.5	1.4	33.3	4.5	0.3	8.6	13.4	
Cars +	125	2060	20	2205	76	3	14	93	18	1366	59	1443	196	12	363	571	4312
% Cars +	91.9	99.9	100	99.4	98.7	100	100	98.9	100	99.7	98.3	99.7	99.5	100	97.3	98.1	99.3
Trucks	11	3	0	14	1	0	0	1	0	4	1	5	1	0	10	11	31
% Trucks	8.1	0.1	0	0.6	1.3	0	0	1.1	0	0.3	1.7	0.3	0.5	0	2.7	1.9	0.7

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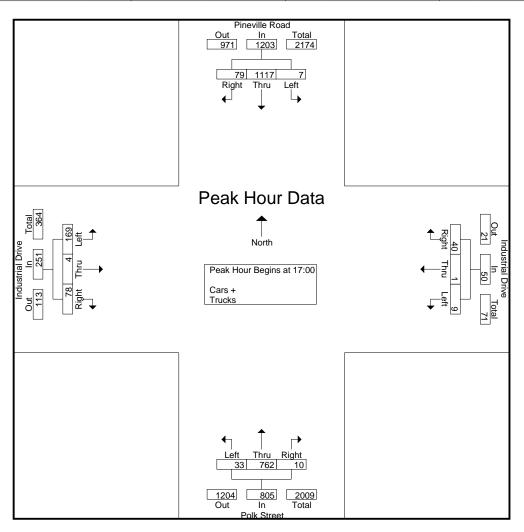
1202 Langdon Terrace Drive Raleigh, NC, 27615

File Name : Pineville(Industrial and Polk) PM Peak Site Code :

Start Date : 5/25/2017

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			le Road			Industri		e			Street			Industr		e]
		South	bound			vvest	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analys	sis From 1	6:00 to 1	7:45 - Pe	eak 1 of 1													
Peak Hour for Ent	ire Inters	ection Be	gins at 1	7:00													
17:00	20	255	2	277	13	1	4	18	2	220	8	230	31	0	59	90	615
17:15	16	277	1	294	12	0	3	15	3	213	10	226	15	3	45	63	598
17:30	24	282	3	309	11	0	0	11	2	152	9	163	19	1	49	69	552
17:45	19	303	1	323	4	0	2	6	3	177	6	186	13	0	16	29	544
Total Volume	79	1117	7	1203	40	1	9	50	10	762	33	805	78	4	169	251	2309
% App. Total	6.6	92.9	0.6		80	2	18		1.2	94.7	4.1		31.1	1.6	67.3		
PHF	.823	.922	.583	.931	.769	.250	.563	.694	.833	.866	.825	.875	.629	.333	.716	.697	.939



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial N and Rodney)AM Peak Site Code :

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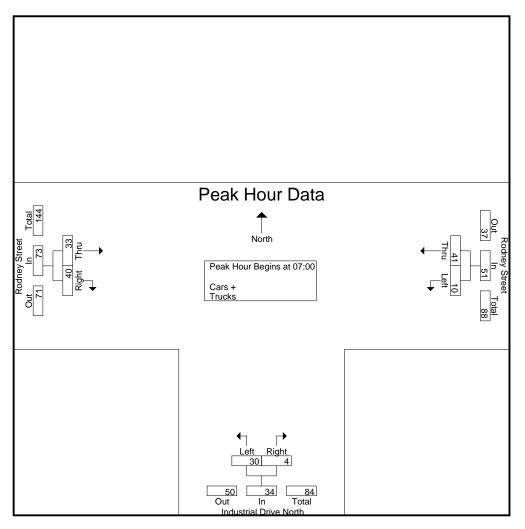
			Gr	oups Printed	- Cars + -	Trucks				
	Ro	odney Stree	et 🛛	Indust	trial Drive I	North	R	odney Stre	et	
	V	<u>Nestbound</u>			lorthbound			Eastbound		
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total
07:00	8	4	12	2	2	4	15	4	19	35
07:15	11	2	13	1	0	1	5	10	15	29
07:30	11	0	11	0	25	25	14	8	22	58
07:45	11	4	15	1	3	4	6	11	17	36
Total	41	10	51	4	30	34	40	33	73	158
08:00	5	6	11	0	2	2	11	3	14	27
08:15	5	3	8	0	5	5	8	10	18	31
08:30	8	2	10	1	0	1	12	5	17	28
08:45	4	4	8	1	5	6	3	9	12	26
Total	22	15	37	2	12	14	34	27	61	112
Grand Total	63	25	88	6	42	48	74	60	134	270
Apprch %	71.6	28.4		12.5	87.5		55.2	44.8		
Total %	23.3	9.3	32.6	2.2	15.6	17.8	27.4	22.2	49.6	
Cars +	61	24	85	5	40	45	71	57	128	258
% Cars +	96.8	96	96.6	83.3	95.2	93.8	95.9	95	95.5	95.6
Trucks	2	1	3	1	2	3	3	3	6	12
% Trucks	3.2	4	3.4	16.7	4.8	6.2	4.1	5	4.5	4.4

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1202Langdon Terace Drive Indian Trail, NC, 28079

> File Name : Pineville(Industrial N and Rodney)AM Peak Site Code : Start Date : 10/24/2017 Page No : 2

		Rodney Stre Westbound		Indu	strial Drive		F	Rodney Stre Eastbound		
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total
Peak Hour Analysis Fro	om 07:00 to	08:45 - Peak	(1 of 1	-			-			
Peak Hour for Entire In	tersection B	egins at 07:0	00							
07:00	8	4	12	2	2	4	15	4	19	35
07:15	11	2	13	1	0	1	5	10	15	29
07:30	11	0	11	0	25	25	14	8	22	58
07:45	11	4	15	1	3	4	6	11	17	36
Total Volume	41	10	51	4	30	34	40	33	73	158
% App. Total	80.4	19.6		11.8	88.2		54.8	45.2		
PHF	.932	.625	.850	.500	.300	.340	.667	.750	.830	.681



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial N and Rodney)PM Peak Site Code :

Item 7.

Start Date : 10/24/2017

Page No : 1

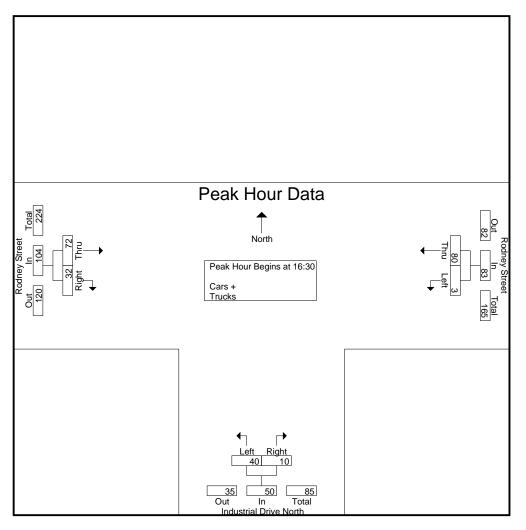
			G	roups Printed	- Cars + -	Trucks				
	R	odney Stree			trial Drive I		R	odney Stre	et	
		Westbound		N	orthbound			Eastbound		
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total
16:00	23	4	27	7	5	12	3	10	13	52
16:15	8	3	11	5	7	12	6	12	18	41
16:30	37	1	38	3	8	11	5	15	20	69
16:45	12	1	13	4	15	19	5	15	20	52
Total	80	9	89	19	35	54	19	52	71	214
17:00	18	0	18	1	6	7	13	23	36	61
		0		1	-	12				
17:15	13	1	14	2	11	13	9	19	28	55
17:30	6	1	/	0	9	9	5	13	18	34
17:45	8	1	9	0	3	3	4	12	16	28
Total	45	3	48	3	29	32	31	67	98	178
Grand Total	125	12	137	22	64	86	50	119	169	392
Apprch %	91.2	8.8		25.6	74.4		29.6	70.4		
Total %	31.9	3.1	34.9	5.6	16.3	21.9	12.8	30.4	43.1	
Cars +	124	11	135	22	61	83	41	115	156	374
% Cars +	99.2	91.7	98.5	100	95.3	96.5	82	96.6	92.3	95.4
Trucks	1	1	2	0	3	3	9	4	13	18
% Trucks	0.8	8.3	1.5	0	4.7	3.5	18	3.4	7.7	4.6

ltem 7.

1202Langdon Terace Drive Indian Trail, NC, 28079

> File Name : Pineville(Industrial N and Rodney)PM Peak Site Code : Start Date : 10/24/2017 Page No : 2

	F	Rodney Stre Westbound		Indu	ustrial Drive Northboun		F	Rodney Stre Eastbound		
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total
Peak Hour Analysis Fro	om 16:00 to 1	7:45 - Peak	(1 of 1	-			-			
Peak Hour for Entire Int	ersection Be	gins at 16:3	30							
16:30	37	⁻ 1	38	3	8	11	5	15	20	69
16:45	12	1	13	4	15	19	5	15	20	52
17:00	18	0	18	1	6	7	13	23	36	61
17:15	13	1	14	2	11	13	9	19	28	55
Total Volume	80	3	83	10	40	50	32	72	104	237
% App. Total	96.4	3.6		20	80		30.8	69.2		
PHF	.541	.750	.546	.625	.667	.658	.615	.783	.722	.859





1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Northern RR Crossing)AM Peak

Site Code : Start Date : 10/24/2017

Page No : 1 Groups Printed- Cars + - Trucks

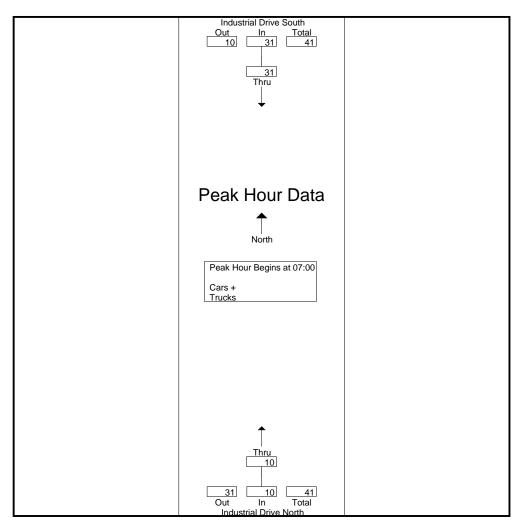
	Industrial Driv Southbo			Drive North hbound	
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
07:00	9	9	5	5	14
07:15	8	8	0	0	8
07:30	11	11	2	2	13
07:45	3	3	3	3	6
Total	31	31	10	10	41
08:00	11	11	1	1	12
08:15	5	5	5	5	10
08:30	8	8	1	1	9
08:45	7	7	3	3	10
Total	31	31	10	10	41
Grand Total	62	62	20	20	82
Apprch %	100		100		
Total %	75.6	75.6	24.4	24.4	
Cars +	60	60	18	18	78
% Cars +	96.8	96.8	90	90	95.1
Trucks	2	2	2	2	4
% Trucks	3.2	3.2	10	10	4.9



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Northern RR Crossing)AM Peak Site Code : Start Date : 10/24/2017 Page No : 2

	Industrial Drive Southbou			Drive North hbound	
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:4	5 - Peak 1 of 1				
Peak Hour for Entire Intersection Begin	s at 07:00				
07:00	9	9	5	5	14
07:15	8	8	0	0	8
07:30	11	11	2	2	13
07:45	3	3	3	3	6
Total Volume	31	31	10	10	41
% App. Total	100		100		
PHF	.705	.705	.500	.500	.732



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Northern RR Crossing)PM Peak Site Code :

Start Date : 10/24/2017

Page No : 1 Groups Printed- Cars + - Trucks

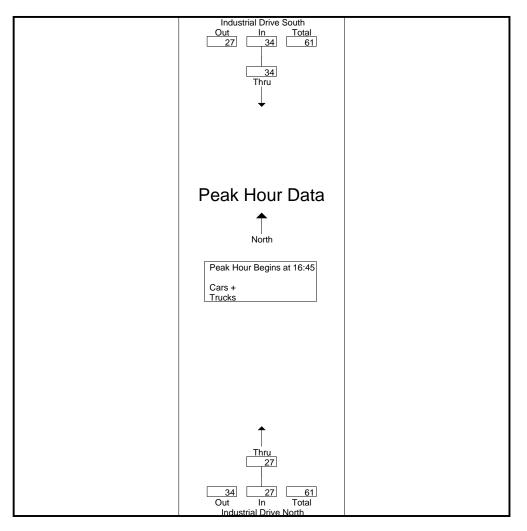
	Industrial Driv Southbo		Industrial	l Drive North hbound	
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
16:00	0	0	0	0	0
16:15	3	3	4	4	7
16:30	2	2	2	2	4
16:45	6	6	10	10	16
Total	11	11	16	16	27
17:00	14	14	2	2	16
17:15	9	9	5	5	14
17:30	5	5	10	10	15
17:45	2	2	2	2	4_
Total	30	30	19	19	49
Grand Total	41	41	35	35	76
Apprch %	100		100		
Total %	53.9	53.9	46.1	46.1	
Cars +	34	34	35	35	69
% Cars +	82.9	82.9	100	100	90.8
Trucks	7	7	0	0	7
% Trucks	17.1	17.1	0	0	9.2



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Northern RR Crossing)PM Peak Site Code : Start Date : 10/24/2017 Page No : 2

	Industrial Drive Southbou		Industrial Drive Northbou		
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:4	5 - Peak 1 of 1	••			
Peak Hour for Entire Intersection Begin	s at 16:45				
16:45	6	6	10	10	16
17:00	14	14	2	2	16
17:15	9	9	5	5	14
17:30	5	5	10	10	15
Total Volume	34	34	27	27	61
% App. Total	100		100		
PHF	.607	.607	.675	.675	.953





1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Southern RR Crossing)AM Peak Site Code :

Start Date : 10/24/2017

Page No : 1 Groups Printed- Cars + - Trucks

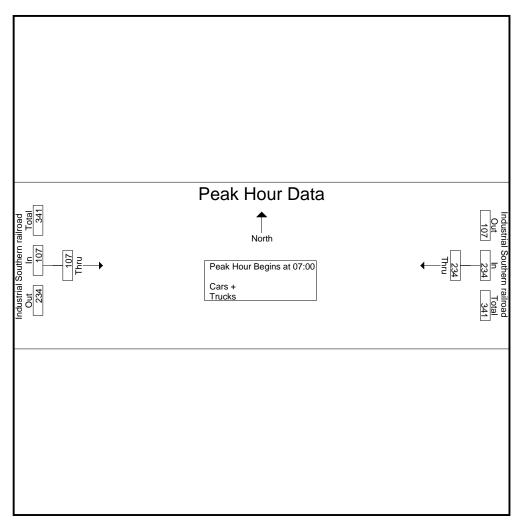
7			Gloups Filited- Cars +		
	thern railroad		outhern railroad		
	ound	Eas	stbound	Wes	
I Int. Total	App. Total	Thru	App. Total	Thru	Start Time
3 101	38	38	63	63	07:00
8 82	28	28	54	54	07:15
7 65	17	17	48	48	07:30
4 93	24	24	69	69	07:45
341	107	107	234	234	Total
85	19	19	66	66	08:00
74	20	20	54	54	08:15
66	30	30	36	36	08:30
5 59	25	25	34	34	08:45
284	94	94	190	190	Total
625	201	201	424	424	Grand Total
		100		100	Apprch %
2	32.2	32.2	67.8	67.8	Total %
586	184	184	402	402	Cars +
5 93.8	91.5	91.5	94.8	94.8	% Cars +
	17	17	22	22	Trucks
6.2	8.5	8.5	5.2	5.2	% Trucks



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Southern RR Crossing)AM Peak Site Code : Start Date : 10/24/2017 Page No : 2

	Industrial Souther Westbour		Industrial Southe Eastbour		
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
		App. Total	mu	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:4					
Peak Hour for Entire Intersection Begin	s at 07:00				
07:00	63	63	38	38	101
07:15	54	54	28	28	82
07:30	48	48	17	17	65
07:45	69	69	24	24	93
Total Volume	234	234	107	107	341
% App. Total	100		100		
PHF	.848	.848	.704	.704	.844





1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Southern RR Crossing)PM Peak Site Code :

Start Date : 10/24/2017

Page No : 1 Groups Printed- Cars + - Trucks

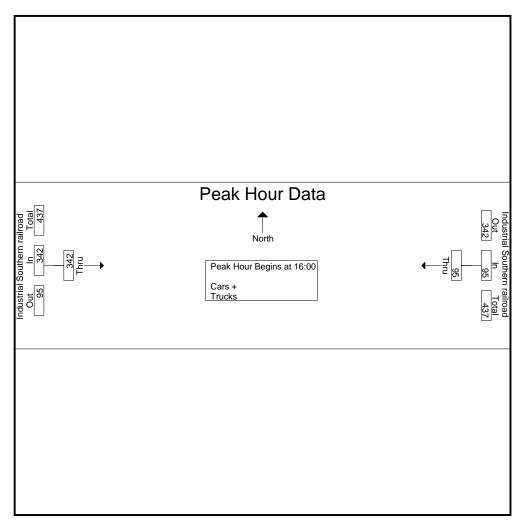
[]		ps Finiteu- Cars + -			
	Industrial Souther	n railroad	Industrial Souther	rn railroad	
	Westbour	nd	Eastbour	nd	
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
16:00	28	28	96	96	124
16:15	24	24	66	66	90
16:30	23	23	114	114	137
16:45	20	20	66	66	86
Total	95	95	342	342	437
17:00	23	23	96	96	119
17:15	22	22	66	66	88
17:30	19	19	38	38	57
17:45	21	21	44	44	65
Total	85	85	244	244	329
Grand Total	180	180	586	586	766
Apprch %	100		100		
Total %	23.5	23.5	76.5	76.5	
Cars +	158	158	564	564	722
% Cars +	87.8	87.8	96.2	96.2	94.3
Trucks	22	22	22	22	44
% Trucks	12.2	12.2	3.8	3.8	5.7



1202Langdon Terace Drive Indian Trail, NC, 28079

File Name : Pineville(Industrial Southern RR Crossing)PM Peak Site Code : Start Date : 10/24/2017 Page No : 2

	Industrial Souther Westbour		Industrial Southe Eastbou		
Start Time	Thru	App. Total	Thru	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:4	5 - Peak 1 of 1				
Peak Hour for Entire Intersection Begin	s at 16:00				
16:00	28	28	96	96	124
16:15	24	24	66	66	90
16:30	23	23	114	114	137
16:45	20	20	66	66	86
Total Volume	95	95	342	342	437
% App. Total	100		100		
PHF	.848	.848	.750	.750	.797



Appendix B – Accident Data

North Carolina Department of Transportation Traffic Engineering Accident Analysis System Fiche, Intersection, and Strip Reports Code Index

	•	-	
T - Type of Accident Codes	E	- Road Feat	ure Codes
0 = UNKNOWN	0	= NO SPECI	AL FEATURE
1 = RAN OFF ROAD - RIGHT	1	= BRIDGE	
2 = RAN OFF ROAD - LEFT	2	= BRIDGE A	PPROACH
3 = RAN OFF ROAD - STRAIGHT	3	= UNDERPA	SS
4 = JACKKNIFE	4	= DRIVEWA	Y, PUBLIC
5 = OVERTURN/ROLLOVER		= DRIVEWA	
13 = OTHER NON-COLLISION			FERSECTION
14 = PEDESTRIAN			YINTERSECTION
15 = PEDALCYCLIST		= T-INTERS	
16 = RR TRAIN, ENGINE	-	= Y-INTERS	
17 = ANIMAL			CIRCLE/ROUNDABOUT
18 = MOVABLE OBJECT			NT, OR MORE
19 = FIXED OBJECT			D TO INTERSECTION
20 = PARKED MOTOR VEHICLE			ERSECTION MEDIAN CROSSING
21 = REAR END, SLOW OR STOP			BEGINNING - DIVIDED HIGHWAY
,		4 = END OR 5 = OFF RAM	
22 = REAR END, TURN		6 = OFF RAM	
23 = LEFT TURN, SAME ROADWAY			-
24 = LEFT TURN, DIFFERENT ROAD		-	IP TERMINAL ON CROSSROAD
25 = RIGHT TURN, SAME ROADWAY			LANE BETWEEN ON AND OFF RAMP
26 = RIGHT TURN, DIFFERENT ROAD 27 = HEAD ON		9 = ON RAM	
		20 = ON RAM	-
28 = SIDESWIPE, SAME DIRECTION			P TERMINAL ON CROSSROAD
29 = SIDESWIPE, OPPOSITE DIRECT			AD CROSSING
30 = ANGLE		3 = TUNNEL	
31 = BACKING UP			-USE PATHS OR TRAILS
32 = OTHER COLLISION WITH VEHIC	ile 2	25 = OTHER	
R - Road Condition Codes	L - Light Condition Codes	_	W - Weather Condition Codes
1 = DRY	1 = DAYLIGHT		1 = CLEAR
2 = WET	2 = DUSK		2 = CLOUDY
3 = WATER (STANDING, MOVING)	3 = DAWN		3 = RAIN
4 = ICE	4 = DARK - LIGHTED ROAD	OWAY	4 = SNOW
5 = SNOW	5 = DARK - ROADWAY NOT	T LIGHTED	5 = FOG, SMOG, SMOKE
6 = SLUSH	6 = DARK - UNKNOWN LIG	HTING	6 = SLEET, HAIL, FREEZING RAIN/DRIZZLE
7 = SAND, MUD, DIRT, GRAVEL	7 = OTHER		7 = SEVERE CROSSWINDS
8 = FUEL, OIL	8 = UNKNOWN		8 = BLOWING SAND, DIRT, SNOW
9 = OTHER			9 = OTHER
10 = UNKNOWN			
	Ch - Road Character		Op - Traffic Control Operating
S - Accident Severity Codes	1 = STRAIGHT, LEVEL		1 = YES
K = FATAL	2 = STRAIGHT, HILLCREST	Г	2 = NO
A = A-LEVEL INJURY	3 = STRAIGHT, GRADE		3 = UNKNOWN
B = B-LEVEL INJURY	4 = STRAIGHT, BOTTOM (S	SAG)	
C = C-LEVEL INJURY	5 = CURVE, LEVEL	,	
O = PROPERTY DAMAGE ONLY	6 = CURVE, HILLCREST		
	7 = CURVE, GRADE		

8 = CURVE, BOTTOM (SAG)

9 = OTHER

North Carolina Department of Transportation Traffic Engineering Accident Analysis System Fiche, Intersection, and Strip Reports Code Index

Veh Mnvr - Vehicle Maneuver Codes	Dv - Traffic Control Device
1 = STOPPED IN TRAVEL LANE	0 = NO CONTROL PRESENT
2 = PARKED OUT OF TRAVEL LANES	1 = STOP SIGN
3 = PARKED IN TRAVEL LANES	2 = YIELD SIGN
4 = GOING STRAIGHT AHEAD	3 = STOP AND GO SIGNAL
5 = CHANGING LANES OR MERGING	4 = FLASHING SIGNAL WITH STOP SIGN
6 = PASSING	5 = FLASHING SIGNAL WITHOUT STOP SIGN
7 = MAKING RIGHT TURN	6 = RR GATE AND FLASHER
8 = MAKING LEFT TURN	7 = RR FLASHER
9 = MAKING U-TURN	8 = RR CROSSBUCKS ONLY
10 = BACKING	9 = HUMAN CONTROL
11 = SLOWING OR STOPPING	10 = WARNING SIGN
12 = STARTING IN ROADWAY	11 = SCHOOL ZONE SIGNS
13 = PARKING	12 = FLASHING STOP AND GO SIGNAL
14 = LEAVING PARKED POSITION	13 = DOUBLE YELLOW LINE, NO PASSING ZONE
15 = AVOIDING OBJECT IN ROAD	14 = OTHER

Alchl/Drgs - Driver Alcohol/Drugs Suspected Status Codes

0 = NO

- 1 = YES ALCOHOL, IMPAIRMENT SUSPECTED
- 2 = YES ALCOHOL, NO IMPAIRMENT DETECTED
- 3 = YES OTHER DRUGS, IMPAIRMENT SUSPECTED
- 4 = YES OTHER DRUGS, NO IMPAIRMENT DETECTED
- 5 = YES ALCOHOL AND OTHER DRUGS, IMPAIRMENT SUSPECTED
- 6 = YES ALCOHOL AND OTHER DRUGS, NO IMPAIRMENT DETECTED

7 = UNKNOWN

Ped Actn - Pedestrian Action Codes

- 1 = ENTERING OR CROSSING SPECIFIED LOCATION
- 2 = WALKING, RIDING, RUNNING/JOGGING WITH TRAFFIC
- 3 = WALKING, RIDING, RUNNING/JOGGING AGAINST TRAFFIC
- 4 = WORKING
- 5 = PUSHING VEHICLE
- 6 = APPROACHING OR LEAVING VEHICLE
- 7 = PLAYING
- 8 = STANDING
- 9 = OTHER

Ci - Roadway Contributing Circumstances

- 0 = NONE (NO UNUSUAL CONDITIONS) 1 = ROAD SURFACE CONDITION 2 = DEBRIS3 = RUT, HOLES, BUMPS 4 = WORK ZONE (CONSTRUCTION, MAINTENANCE, UTILITY) 5 = WORN TRAVEL-POLISHED SURFACE 6 = OBSTRUCTION IN ROADWAY 7 = TRAFFIC CONTROL DEVICE INOPERATIVE, NOT VISIBLE OR MISSING 8 = SHOULDERS LOW, SOFT OR HIGH
- 9 = NO SHOULDERS
- 10 = NON-HIGHWAY WORK
- 11 = OTHER
- 12 = UNKNOWN

North Carolina Department of Transportation Traffic Engineering Accident Analysis System Fiche, Intersection, and Strip Reports Code Index

Obj Strk - Object Struck Codes	Unit # - Vehicle Style Codes
14 = PEDESTRIAN	1 = PASSENGER CAR
15 = PEDALCYCLIST	2 = PICKUP
17 = ANIMAL	3 = LIGHT TRUCK (MINI-VAN, PANEL)
18 = MOVABLE OBJECT	4 = SPORT UTILITY
20 = PARKED MOTOR VEHICLE	5 = VAN
33 = TREE	6 = COMMERCIAL BUS
34 = UTILITY POLE	7 = SCHOOL BUS
35 = LUMINAIRE POLE NON-BREAKAWAY	8 = ACTIVITY BUS
36 = LUMINAIRE POLE BREAKAWAY	9 = OTHER BUS
37 = OFFICIAL HIGHWAY SIGN NON-BREAKAWAY	10 = SINGLE UNIT TRUCK (2-AXLE, 6-TIRE)
38 = OFFICIAL HIGHWAY SIGN BREAKAWAY	11 = SINGLE UNIT TRUCK (3 OR MORE AXLES)
39 = OVERHEAD SIGN SUPPORT	12 = TRUCK/TRAILER
40 = COMMERCIAL SIGN	13 = TRUCK/TRACTOR
41 = GUARDRAIL END ON SHOULDER	14 = TRACTOR/SEMI-TRAILER
42 = GUARDRAIL FACE ON SHOULDER	15 = TRACTOR/DOULBES
43 = GUARDRAIL END IN MEDIAN	16 = UNKNOWN HEAVY TRUCK
44 = GUARDRAIL FACE IN MEDIAN	17 = TAXICAB
45 = SHOULDER BARRIER END	18 = FARM EQUIPMENT
46 = SHOULDER BARRIER FACE	19 = FARM TRACTOR
47 = MEDIAN BARRIER END	20 = MOTORCYCLE
48 = MEDIAN BARRIER FACE	21 = MOPED
49 = BRIDGE RAIL END	22 = MOTOR SCOOTER OR MOTOR BIKE
50 = BRIDGE RAIL FACE	23 = PEDALCYCLE
51 = OVERHEAD PART UNDERPASS	24 = PEDESTRIAN
52 = PIER ON SHOULDER OF UNDERPASS	25 = MOTOR HOME/RECREATIONAL VEHICLE
53 = PIER IN MEDIAN OF UNDERPASS	26 = OTHER
54 = ABUTMENT OF UNDERPASS	27 = ALL TERRAIN VEHICLE (ATV)
55 = TRAFFIC ISLAND CURB OR MEDIAN	28 = FIRETRUCK
56 = CATCH BASIN OR CULVERT ON SHOULDER	29 = EMS VEHICLE, AMBULANCE, RESCUE SQUAD
57 = CATCH BASIN OR CULVERT ON MEDIAN	30 = MILITARY
58 = DITCH	31 = POLICE
59 = EMBANKMENT	32 = UNKNOWN
60 = MAILBOX	
61 = FENCE OR FENCE POST	

62 = CONTRUCTION BARRIER

63 = CRASH CUSHION 64 = OTHER FIXED OBJECT

North Carolina Department of Transportation

Traffic Engineering Accident Analysis System
Internetien Analysis Devent

				In	tersectio														
				<u>S</u>	tudy Cri	teria	Su	mmary											
County Date:	MECKL	ENBURG /2012 to	04/3	0/2017	City: Study:		and	Rural 7242											
Locatio	08 32	l (Polk St- sis should			t SR 3542	(Ind	ustri	al Dr).	**	Cras	h ra	ites	cont	ain	ed i	n th	is		
					Repo	ort D	etai	ls											
Acc								Total		Inju	ries		Co	ndit	ion	Ro	ad	Trf	сC
No	Crash ID	Date		Acc	ident Type		C	amage	F	Α	в	С	R	L	W	Ch	Ci	D٧	0
1	103473281	05/23/201 15:13	2	REAR END,	SLOW OR S	STOP	\$	2000	0	0	0	0	1	1	1	1	0	3	1
Unit	1:1	Alchl/Drgs:	0	Speed:	10 MPH	Dir:	s	Veh N	Invr	/ Ped	Actr	1:	11		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	0 MPH	Dir:	s	Veh N	Invr	Ped	Actr	1:	11		Obj	Strk:			
2	103600255	10/27/201 10:30	2	REAR END,	SLOW OR S	TOP	\$	11200	0	0	0	2	1	1	2	1	0	3	1
			0	Speed:	35 MPH	Dir:	s	Veh N	Invr	/ Ped	Actr	1:	4		Obj	Strk:			
Unit	1:1	Alchl/Drgs:																	
Unit Unit	1:1 2:1	Alchl/Drgs: Alchl/Drgs:	0	Speed:	25 MPH	Dir:	s	Veh N	Invr	/ Ped	Actr	1:	11		Obj	Strk:		20	

onit	3.1	Alcin Digs.	'	Speed.	0	WIFTI	DII.	**	ven	WIIIVI	Feu	Acti		2		Obj	Suk.		20	
3	103720686	02/15/2013 22:34		ANGLE				s	3400	0	0	0	0	1	5	1	1	0	3	1
Unit	1:4	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	s	Veh	Mnvr	/ Ped	Actn		4		Obj	Strk:	:		
Unit	2 : 10	Alchl/Drgs:	0	Speed:	15	MPH	Dir:	Е	Veh	Mnvr	/ Ped	Actn		7		Obj	Strk:	:		
4	103751319	04/29/2013 13:01		LEFT TURN,	SAN	IE RO	ADWA	Y \$	7500	0	0	0	0	1	1	2	3	0	3	1
Unit	1:1	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	w	Veh	Mnvr	/ Ped	Actn	1:	8		Obj	Strk:	:		
Unit	2:1	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	SW	Veh	Mnvr	Ped	Actr		4		Obj	Strk:	:		
5	103918119	10/17/2013 17:53		REAR END,	SLO	N OR S	STOP	\$	7000	0	0	0	2	2	1	3	1	0		
Unit	1:1	Alchl/Drgs:	0	Speed:	25	MPH	Dir:	Ν	Veh	Mnvr	/ Ped	Actn	1:	4		Obj	Strk:	:		
Unit	2 : 5	Alchl/Drgs:	0	Speed:	0	MPH	Dir:	Ν	Veh	Mnvr	/ Ped	Actn		1		Obj	Strk:	:		
6	103983348	12/27/2013 17:37		REAR END,	SLO	N OR S	STOP	\$	1200	0	0	0	0	1	4	1	1	0	0	
Unit	1:4	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	Ν	Veh	Mnvr	/ Ped	Actr	1:	12		Obj	Strk:	:		
Unit	2:4	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	Ν	Veh	Mnvr	Ped	Actr		1		Obj	Strk:	:		
7	104009922	02/08/2014 16:24		REAR END,	SLO	N OR S	STOP	\$	2500	0	0	0	0	1	1	1	1	0		
Unit	1:1	Alchl/Drgs:	0	Speed:	25	мрн	Dir:	s	Veh	Mnvr	/ Ped	Actn	:	4		Obj	Strk:	;		
Unit	2:1	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	s	Veh	Mnvr	/ Ped	Actr	:	11		Obj	Strk:	:		
8	104028058	03/08/2014 10:22		REAR END,	SLO	N OR S	STOP	\$	3250	0	0	0	0	1	1	1	1	0	3	1
		Alchl/Drgs:		Speed:					Veh					4		Obj	. .			

06/27/2017

North Carolina Department of Transportation
Traffic Engineering Accident Analysis System

Acc								T	Tota		Inju	ries		Co	ndif	ion	Ro	ad	Trfo	c
No	Crash ID	Date		Acc	iden	t Type		1	Damage	F	Á	в	С	R	L	W	Ch	Ci	Dv	0
18	104448464	07/30/2015 12:05		RAN OFF R	OAD -	RIGH	Т	\$	1805	0	0	0	1	1	1	1	1	0	5	1
Unit	1:2	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	NE	Veh l	Mnvr	/ Ped	Actn	:	8		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	SW	Veh	Mnvr	/ Ped	Actn	:	4	_	Obj	Strk:	_	18	
19	104508930	10/05/2015 11:48		REAR END,	SLO	N OR S	STOP	\$	9500	0	0	0	1	1	1	2	1	0	3	
Unit	1:5	Alchl/Drgs:	0	Speed:	30	MPH	Dir:	s	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:2	Alchl/Drgs:	7	Speed:	0	MPH	Dir:	s	Veh	Mnvr	/ Ped	Actn	:	1		Obj	Strk:			
20	104554191	11/12/2015 20:10		LEFT TURN ROADWAYS		EREN	т	s	2000	0	0	0	0	1	4	1	1	0	3	
Unit	1:4	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	Ν	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	15	MPH	Dir:	E	Veh	Mnvr	/ Ped	Actn	:	8	_	Obj	Strk:			
21	104606113	12/04/2015 23:27		ANGLE				\$	4000	0	0	0	2	1	4	1	1	0	3	
Unit	1:4	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	s	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:5	Alchl/Drgs:	0	Speed:	4	MPH	Dir:	Е	Veh	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
22	104630745	02/01/2016 14:20		ANGLE				\$	9000	0	0	0	1	1	1	1	3	0	3	
Unit	1:1	Alchl/Drgs:	0	Speed:	35	MPH	Dir:	Ν	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:2	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	w	Veh	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
23	104889056	10/25/2016 15:06		LEFT TURN	, SAN	IE RO/	ADWAY	\$	9000	0	0	0	0	1	1	1	1	0	3	-
Unit	1:1	Alchl/Drgs:	0	Speed:	8	MPH	Dir:	Ν	Veh I	Mnvr	/ Ped	Actn	:	8		Obj	Strk:			
Unit	2:2	Alchl/Drgs:	0	Speed:	30	MPH	Dir:	s	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
24	104912808	11/10/2016 08:31		REAR END,	SLO	N OR S	STOP	\$	3000	0	0	0	0	1	1	1	1	0	0	
Unit	1:1	Alchl/Drgs:	0	Speed:	15	MPH	Dir:	Е	Veh I	Mnvr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:3	Alchl/Drgs:	7	Speed:	0	MPH	Dir:	E	Veh I	Mnvr	/ Ped	Actn	: 	1		Obj	Strk:			
25	104932924	11/29/2016 11:53		BACKING U	Р			\$	1500	0	0	0	0	2	1	2	1	0	3	
Unit	1:2	Alchl/Drgs:	7	Speed:	2	MPH	Dir:	Ν	Veh I	Mnvr	/ Ped	Actn	:	10		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	1	MPH	Dir:	s	Veh	Mnvr	/ Ped	Actn	:	1		Obj	Strk:			
26	104964447	01/03/2017 13:19		ANGLE				\$	1500	0	0	0	0	1	1	2	1	0	3	
Unit	1:14	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	Е	Veh I	Mnvr	/ Ped	Actn	:	8		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	7	Speed:	0	MPH	Dir:	w	Veh I	Mnvr	/ Ped	Actn		1		Obi	Strk:			

				In	ters	ectio	n An	alysi	is Repor	t							lter	n	7.	
Acc		1							Total		Inju			Co	ndit					
No	Crash ID	Date		Acc	iden	t Type	Ð		Damage	F	A	В	С	R	L	W	Ch	Ci	Dv	0
Unit	2:1	Alchl/Drgs:	0	Speed:	40	MPH	Dir:	s	Veh M	Anvr.	/ Ped	Actn	:	11		Obj	Strk:			
9	104144964	07/18/2014 08:37		RIGHT TUR	N, SA	ME RO	DADW	AY \$	7200	0	0	0	0	1	1	1	1	0	3	1
Unit	1:14	Alchl/Drgs:	0	Speed:	8	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	7		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	5	MPH	Dir:	N	Veh M	Anvr	Ped	Actn	:	7		Obj	Strk:			
10	104156148	08/12/2014 17:27		REAR END,	SLO	N OR S	STOP	\$	3200	0	0	0	1	1	1	1	1	0	3	1
Unit	1:1	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	11		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	11		Obj	Strk:			
Unit	3:2	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	Ν	Veh M	Invr	Ped	Actn	:	1		Obj	Strk:			
11	104244597	11/08/2014 22:36		REAR END,	SLO	N OR S	STOP	\$	1000	0	0	0	0	1	4	1	1	0		
Unit	1:4	Alchl/Drgs:	1	Speed:	35	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	11		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	0	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	1		Obj	Strk:			
12	104246853	12/17/2014 17:50	_	ANGLE				\$	600	0	0	0	0	1	4	1	1	0	3	1
Unit	1:1	Alchl/Drgs:	0	Speed:	0	MPH	Dir:	s	Veh M	/Invr	Ped	Actn	:	8		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	10	MPH	Dir:	NW	Veh M	/Invr	Ped	Actn	:	4		Obj	Strk:			
13	104281833	12/19/2014 16:03		REAR END,	SLO	N OR S	STOP	\$	1050	0	0	0	0	1	1	1	1	0		
Unit	1:4	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	11		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	15	MPH	Dir:	N	Veh M	Anvr	Ped	Actn	:	1		Obj	Strk:			
14	104271217	01/17/2015 19:17		RAN OFF R	DAD -	LEFT		\$	15100	0	0	0	1	1	4	1	1	0	0	
Unit	1:2	Alchl/Drgs:	0	Speed:	47	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	5		Obj	Strk:		55	
15	104298852	02/19/2015 18:42		REAR END,	SLO	N OR S	STOP	\$	1500	0	0	0	0	1	4	1	1	0	0	
Unit	1:1	Alchl/Drgs:	0	Speed:	15	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	11		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	0	Speed:	10	MPH	Dir:	Ν	Veh M	/Invr	Ped	Actn	:	1		Obj	Strk:			
16	104438671	07/18/2015 13:19		REAR END,	SLO	N OR S	STOP	\$	2000	0	0	0	0	1	1	1	1	0	0	
Unit	1:4	Alchl/Drgs:	7	Speed:	25	MPH	Dir:	Ν	Veh M					4		Obj	Strk:			
Unit	2:1	Alchl/Drgs:	7	Speed:	0	MPH	Dir:	Ν	Veh M	Anvr	Ped	Actn	:	1		Obj	Strk:			
17	104441323	07/24/2015 14:37		REAR END,	SLO	N OR S	STOP	\$	2200	0	0	0	1	1	1	1	1	0	3	1
Unit	1:5	Alchl/Drgs:	0	Speed:	20	MPH	Dir:	NW	Veh M	/Invr	/ Ped	Actn	:	4		Obj	Strk:			
Unit	2:4	Alchl/Drgs:	0	Speed:	0	MPH	Dir:	NW	Veh M	/nvr	/ Ped	Actn	:	1		Obj	Strk:			

North Carolina Department of Transportation Traffic Engineering Accident Analysis System

_	rk:	-	Trf Dv 3	1
	rk:	0		1
bj St 	rk:			
1		0	3	1
bj St	rk:			
bj St	rk:	3	20	
bj St	rk:	;	20	
bj St	rk:	4	20	
	bj St bj St	bj Strk: bj Strk: bj Strk: bj Strk:	bj Strk: bj Strk:	bjStrk: 20 bjStrk: 20

Summary Statistics

High Level Crash Summary

Crash Type	Number of Crashes	Percent of Tota
Total Crashes	28	100.00
Fatal Crashes	0	0.00
Non-Fatal Injury Crashes	10	35.71
Total Injury Crashes	10	35.71
Property Damage Only Crashes	18	64.29
Night Crashes	8	28.57
Wet Crashes	2	7.14
Alcohol/Drugs Involvement Crashes		2 57

Crash Severity Summary

Crash Type	Number of Crashes	Percent of Total
Total Crashes	28	100.00
Fatal Crashes	0	0.00
Class A Crashes	0	0.00
Class B Crashes	1	3.57
Class C Crashes	9	32.14
Property Damage Only Crashes	18	64.29

Vehicle Exposure Statistics Annual ADT = 999999 Total Vehicle Exposure = 1826 (MEV)

	Crashes Per 100 Million
Crash Rate	Vehicles Entered
Total Crash Rate	1.53
Fatal Crash Rate	0.00
Non Fatal Crash Rate	0.55
Night Crash Rate	0.44
Wet Crash Rate	0.11
EPDO Rate	5.59

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3.64

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134205.00

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Miscellaneous Statistics Severity Index = EPD0 Crash Index = Estimated Property Damage Total = \$

Accident Type	Number of Crashes	Percent of Total
ANGLE	6	21.43
BACKING UP	1	3.57
LEFT TURN, DIFFERENT ROADWAYS	1	3.57
LEFT TURN, SAME ROADWAY	2	7.14
RAN OFF ROAD - LEFT	1	3.57
RAN OFF ROAD - RIGHT	1	3.57
REAR END, SLOW OR STOP	15	53.57
RIGHT TURN, SAME ROADWAY	1	3.57

Injury Summary Number of Percent Injury Type of Total Injuries Fatal Injuries 0.00 Class A Injuries 0.00 Class B Injuries Class C Injuries 7.69 92.31 Total Non-Fatal Injuries 13 100.00 Total Injuries 13

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			Traffic Engineering Accident Analysis S Intersection Analysis Report
	Monthly Sumn	nary	
Month	Number of Crashes	Percent of Total	-
Jan	2	7.14	-
Feb	4	14.29	
Mar	2	7.14	
Apr	2	7.14	
May	1	3.57	
Jun	0	0.00	
Jul	4	14.29	
Aug	1	3.57	
Sep	0	0.00	
Oct	4	14.29	
Nov	4	14.29	
Dec	4	14.29	
			-
	Daily Summar	¥	
Day	Number of Crashes	Percent of Total	

North Carolina Department of Transportation

Hour Crashes of Total 0100-0159 0.00 0200-0259 0.00 0300-0359 0.00 0400-0459 0500-0559 0.00 0600-0659 0.00 0700-0759 0.00 0800-0859 10.71 0900-0959 1000-1059 1100-1159 7.14 1200-1259 3.57 1300-1359 10.71 1400-1459 7.14 1500-1559 10.71 1600-1659 7.14 1700-1759 14.29 1800-1859 3.57 1900-1959 3.57 2000-2059 2100-2159 0.00 2200-2259 7.14 2300-2359 3.57

Hourly Summary

Number of

Percent

Mon

Tue

Wed

Thu

Fri

Sat

Sun

17.86

17.86

21.43

25.00

0.00

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Angle

Side Swipe Other

Run Off Road &

Fixed Object

Light and Road Conditions Summary Condition Dry Wet Other Total Day 18 2 0 0 Dark 8 8 Other 0 0 0 0 Total 26 0 28

Object Struck Summary

Object Type	Times Struck	Percent of Total
MOVABLE OBJECT	1	14.29
PARKED MOTOR VEHICLE	5	71.43
TRAFFIC ISLAND CURB OR MEDIAN	1	14.29

Vehicle Type Summ	Vehicle Type Summary			
Vehicle Type	Number Involved	Percent of Total		
LIGHT TRUCK (MINI-VAN, PANEL)	2	3.39		
PASSENGER CAR	31	52.54		
PICKUP	9	15.25		
SINGLE UNIT TRUCK (2-AXLE, 6-TIRE)	1	1.69		
SPORT UTILITY	10	16.95		
TRACTOR/SEMI-TRAILER	2	3.39		
VAN	4	6.78		

Yearly Totals Summary

Accident Totals

		Accident	lotals	
Year	Total Accidents	Fatal Accidents	Injury Accidents	Property Damage Only Accidents
2012	2	0	1	1
2013	4	0	1	3
2014	7	0	1	6
2015	θ	0	5	3
2016	4	0	1	з
2017	3	0	1	2
Total	28	0	10	18

Injury Totals

Year	Fatal Injuries	Class A, B, or C Injuries
2012	0	2
2013	0	2
2014	0	1
2015	0	6
2016	0	1
2017	0	1
Total	0	13

1	Miscellaneous To	tals
Pr	operty Damage	EPDO Index
Ş	13200	9.40
Ş	19100	11.40
ş	18800	14.40
Ş	38105	45.00
Ş	22500	11.40
ş	22500	10.40
Ş	134205	102.00
	Pr 5 5 5 5 5 5 5	\$ 19100 \$ 18800 \$ 38105 \$ 22500 \$ 22500

			Type of Acc	ident Totals			
				Run Off Road &			
Year	Left Turn	Right Turn	Rear End	Fixed Object	Angle	Side Swipe	Other
2012	0	0	2	0	0	0	0
2013	1	0	2	0	1	0	0
2014	0	1	5	0	1	0	0

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Item 7.

North Carolina Department of Transportation Traffic Engineering Accident Analysis System Intersection Analysis Report Study Criteria

			Log No.	PHI	No. TIPI	No. K	A Cf.	B/C Cf.	ADT	ADT Route
11000047242			41000047242				6.8	8.4	999999	
Request Date	Courier Serv	ice	Phone No.	Ex	t. Fax	No.				
Count	ty		Mun	icipalit	iy					
Name	Code	Div.	Name		Code	Y-Line Ft	. Be	gin Date	End Date	e Years
MECKLENBURG	60	10	All and Rural			150	05/	01/2012	04/30/20	L7 5.00
ocation Text					Requestor					
	ates contain		SR 3542 (Indust this analysis	rial	Cliff Lawson Timmons Grou					

105064867

i.	n	a	i	5	5	1	a	e.			

Excluded Accidents
103719919
103983408
104009927
104055797
104154613
104185058
104215370
104216414
104242062
104271374
104299565
104372680

104372680
104421037

\$

Name	Code
US 521	20000521
POLK	50024505
PINEVILLE	50024239
SOUTH	50028612
SR 3542	40003542

06/27/2017

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Year

Left Turn

Right Turn

Rear End

15

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		Study Criteria Summary
County:	MECKLENBURG	City: All and Rural
Date:	11/1/2012 to 10/31/2017	Study: 41000050292
Location:	Industrial Dr at Rodney St	

Report Details

Acc							Total		Inju	ries		Co	ndit	ion	Ro	ad	Trfc	Ct
No	Crash I	D Da	ite	Acc	ident Type		Damage	F	Α	в	С	R	L	w	Ch	Ci	Dv	Op
1	1042838	21 02/02/ 08		RAN OFF RO	DAD - RIGHT	\$	\$ 100	0	0	0	0	2	1	2	3	0	0	
Unit	1:14	Alchl/Drg	s: 0	Speed:	1 MPH Dir	: NE	Veh N	Anvr /	Ped	Actr	:	7		Obj	Strk:		40	
		Acc No - Acci			is B. C - Class C													

Summary Statistics

High Level Crash Summary

No. 1				
Crash Type	Number of Crashes	Percent of Total		
Total Crashes	1	100.00		
Fatal Crashes	0	0.00		
Non-Fatal Injury Crashes	0	0.00		
Total Injury Crashes	0	0.00		
Property Damage Only Crashes	1	100.00		
Night Crashes	0	0.00		
Wet Crashes	1	100.00		
Alcohol/Drugs Involvement Crashes	0	0.00		

Crash Severity Summary Number of Percent Crashes of Total Crash Type 100.00 Fatal Crashes 0 Fatal Crashes Class A Crashes Class B Crashes Class C Crashes Property Damage Only Crashes 0.00 0 0 0.00 100.00

Vehicle Exposure Statistics Annual ADT = 3300 Total Vehicle Exposure = 6.03 (MEV)

Crash Rate	Crashes Per 100 Millio	
Crash Rate	Vehicles Entered	
Total Crash Rate	16.60	
Fatal Crash Rate	0.00	
Non Fatal Crash Rate	0.00	
Night Crash Rate	0.00	
Wet Crash Rate	16.60	
EPDO Rate	16.60	

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	Traffic E	nginee	ering Accid	nt of Transporta ent Analysis Sys lysis Report	
Miscellan	eous Statistics				
Severity Index =			1.00	D	
EPDO Crash Index =			1.00	D	
Estimated Property Damage 7	Total = \$		100.00	D	
Accident	Type Summary				
Accident Type		ber of shes	Percent of Total	-	
RAN OFF ROAD - RIGHT		1	100.00	-	
Injury Sum	Number of	Perc			
Fatal Injuries	Injuries	of To			
-	-				
Class A Injuries	0	0.0	-		
Class B Injuries	0	0.0	-		
Class C Injuries	0	0.0			
Total Non-Fatal Injuries	0	0.0	0		
Total Injuries					

North Carolina Department of Transp Traffic Engineering Accident Analysis Intersection Analysis Report	

Monthly Summary					
Month	Number of Crashes	Percent of Total			
Jan	0	0.00			
Feb	1	100.00			
Mar	0	0.00			
Apr	0	0.00			
May	0	0.00			
Jun	0	0.00			
Jul	0	0.00			
Aug	0	0.00			
Sep	0	0.00			
Oct	0	0.00			
Nov	0	0.00			
Dec	0	0.00			

Daily Summary				
Day	Number of Crashes	Percent of Total		
Mon	1	100.00		
Tue	0	0.00		
Wed	0	0.00		
Thu	0	0.00		
Fri	0	0.00		
Sat	0	0.00		
Sun	0	0.00		

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Hour	Number of	Percent
	Crashes	of Total
0000-0059	0	0.00
0100-0159	0	0.00
0200-0259	0	0.00
0300-0359	0	0.00
0400-0459	0	0.00
0500-0559	0	0.00
0600-0659	0	0.00
0700-0759	0	0.00
0800-0859	1	100.00
0900-0959	0	0.00
1000-1059	0	0.00
1100-1159	0	0.00
1200-1259	0	0.00
1300-1359	0	0.00
1400-1459	0	0.00
1500-1559	0	0.00
1600-1659	0	0.00
1700-1759	0	0.00
1800-1859	0	0.00
1900-1959	0	0.00
2000-2059	0	0.00
2100-2159	0	0.00
2200-2259	0	0.00
2300-2359	0	0.00

North Carolina Department of Transportation
Traffic Engineering Accident Analysis System
Intersection Analysis Report

Light and Road Conditions Summary				
Condition	Dry	Wet	Other	Total
Day	0	1	0	1
Dark	0	0	0	0
Other	0	0	0	0
Total	0	1	0	1

Object Struck Summary				
	Times	Percent		
Object Type	Struck	of Total		
COMMERCIAL SIGN	1	100.00		

Vehicle Type Summary				
	Number	Percent		
Vehicle Type	Involved	of Total		
TRACTOR/SEMI-TRAILER	1	100.00		

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North Carolina Department of Transportation Traffic Engineering Accident Analysis System Intersection Analysis Report

Yearly Totals Summary

Accident Totals					
Year	Total Accidents	Fatal Accidents	Injury Accidents	Property Damage Only Accidents	
2012	0	0	0	0	
2013	0	0	0	0	
2014	0	0	0	0	
2015	1	0	0	1	
2016	0	0	0	0	
2017	0	0	0	0	
Total	1	0	0	1	

	Injury Totals									
Year	Fatal Injuries	Class A, B, or C Injuries								
2012	0	0								
2013	0	0								
2014	0	0								
2015	0	0								
2016	0	0								
2017	0	0								
Total	0	0								

	J	Miscellaneous To	tals
Year	Pr	operty Damage	EPDO Index
2012	\$	0	0.00
2013	\$	0	0.00
2014	\$	0	0.00
2015	\$	100	1.00
2016	\$	0	0.00
2017	\$	0	0.00
Total	\$	100	1.00

Run Off Road &							
Year	Left Turn	Right Turn	Rear End	Fixed Object	Angle	Side Swipe	Other
2012	0	0	0	0	0	0	0
2013	0	0	0	0	0	0	0
2014	0	0	0	0	0	0	0

			Traffic Eng	olina Departme ineering Accid tersection Anal	ent Anal	ysis System	
Year	Left Turn	Right Turn	Rear End	Run Off Road & Fixed Object	Angle	Side Swipe	Other
2015	0	0	0	1	0	0	0
2016	0	0	0	0	0	0	0
2017	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	0

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Study Criteria

Study Name			Log No.	PH No.	TIP I	No. K/.	A Cf.	B/C Cf.	ADT	ADT Route
41000050292			41000050292			7	5.8	8.4	3300	
Request Date	Courier Serv	rice	Phone No.	Ext.	Fax	No.				
Cou	nty		Mun	icipality						
Cou Name	nty Code	Div.	Mun	icipality	Code	Y-Line Ft.	Beç	jin Date	End Date	e Years
		Div.		icipality	Code	Y-Line Ft.		jin Date	End Date 10/31/203	

trial Dr at Rodney St

Excluded Acciden
105035691
104501387
104185059
104155626
104009919
103926165

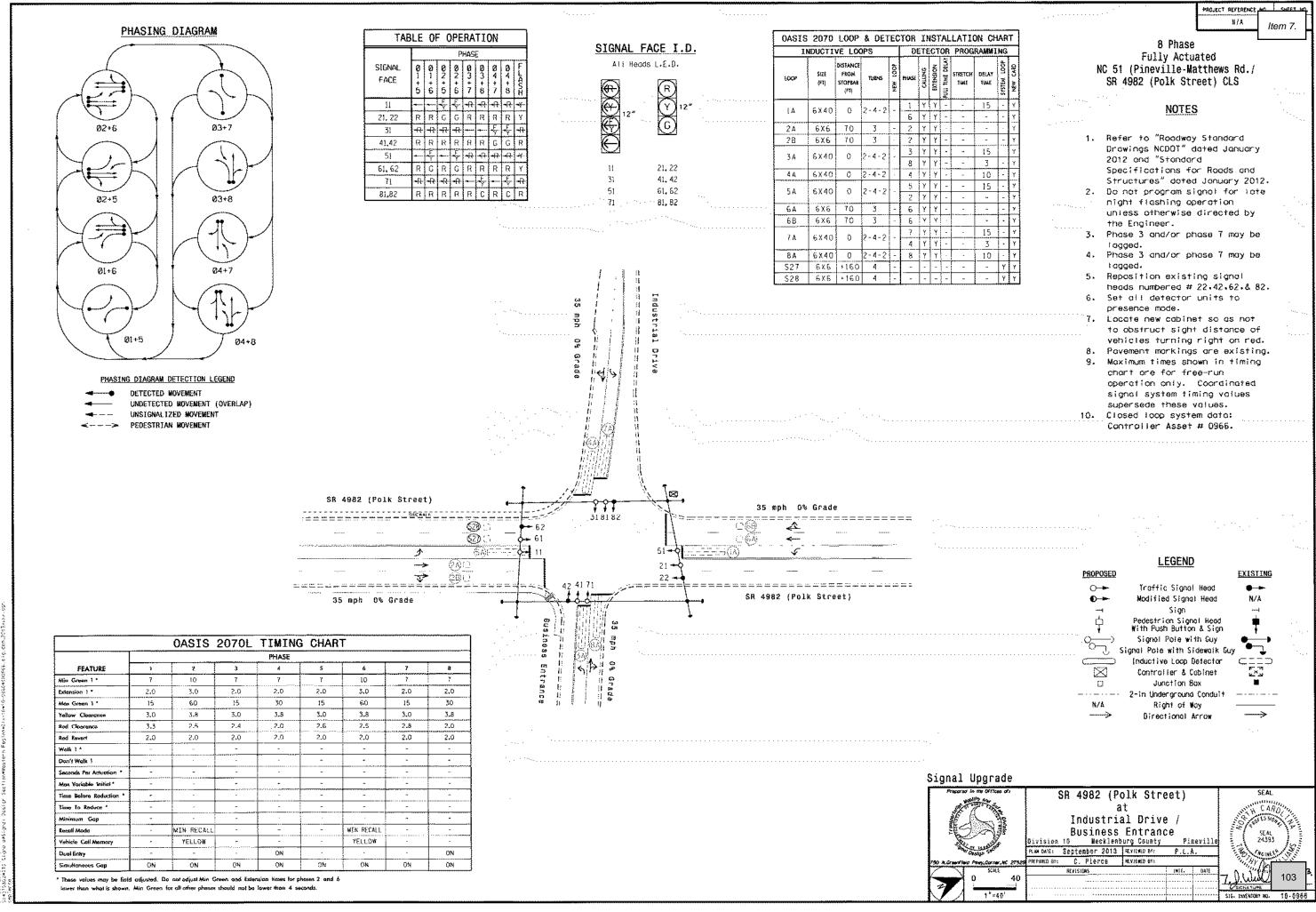
Fiche Roads		
Name	Code	
RODNEY	50026333	
INDUSTRIAL	50014936	
SR 5436	40005436	

Intersection Road Combinations								
Name	Code	Code	Name					
RODNEY	50026333	50014936	INDUSTRIAL					

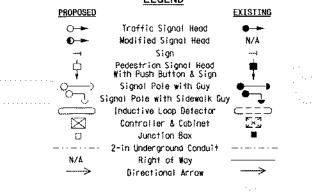
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Appendix C – Traffic Signal Plans



R PRO	ION C		_		8 Phase Fully Actuated	
STRETI THAL		EM LOOP	W CARD	I	SR 4982 (Polk Street) CLS	
		SYSTEM	NEW			te terrer
-	15	-	Y		NOTES	•
-	-	-	Y			1999 A.
	-	1.	Y			
	-	-	Y	1.	Refer to "Roadway Standard	
	15		Y		Drawings NCDOT" dated January	
·	3	-	Y		2012 and "Standard	
	10	1.	Y		Specifications for Roads and	
· -	15	-	Y	*	Structures" dated January 2012.	
- 1		-	γ	2.	Do not program signal for late	
- 1	- 1	-	Y	a transmission of	night flashing operation unless otherwise directed by	· · · · ·
	~	1.	Y		the Engineer.	· .
1 .	15	1.	Y		Phase 3 and/or phase 7 may be	
	3	17	Y	5.	lagged.	
+ .	10	1.	Y	4,	Phase 3 and/or phase 7 may be	
- 1		Υ	Y		lagged.	
	<u> </u>	ŤŸ	Y	5.	Reposition existing signal	
					heads numbered # 22.42.62.& 82.	
				6.	Set all detector units to	
					presence mode.	···· ···
				7.	Locate new cabinet so as not	
					to obstruct sight distance of	
				•	vehicles turning right on red.	
				8. 9.	Pavement morkings are existing. Maximum times shown in timing	
				5.	chart are for free-run	
					operation only. Coordinated	
					signal system timing values	
					supersede these values.	
				10.	Closed loop system data:	
			• •		Controller Asset # 0966.	



Appendix D – Synchro Analysis Outputs

2017 Existing Traffic Volumes

966: N Polk Street/I	Pinevill	e Road	l & Inc	lustrial	Drive/	Drivev	vay				11/0)3/2017
	٦	-	\mathbf{r}	4	+	×	1	Ť	۲	1	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲.	¢î		۲	4î		۲	≜ ⊅		<u>۲</u>	≜ ⊅	
Traffic Volume (vph)	61	0	38	4	2	18	98	858	14	38	452	160
Future Volume (vph)	61	0	38	4	2	18	98	858	14	38	452	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.850			0.864			0.998			0.961	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1583	0	1770	1609	0	1770	3532	0	1770	3401	0
Flt Permitted	0.597						0.299			0.257		
Satd. Flow (perm)	1112	1583	0	1863	1609	0	557	3532	0	479	3401	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	68	0	42	4	2	20	109	953	16	42	502	178
Shared Lane Traffic (%)												
Lane Group Flow (vph)	68	42	0	4	22	0	109	969	0	42	680	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	_
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	_
Two way Left Turn Lane	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	Yes	4.00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	N 1 A	9	15	N L A	9	15	NIA	9	15		9
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	7 4	4		3	8		5	2		1	6	
Permitted Phases	т	4		0	0		2	0		6	0	
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	_
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	_
Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	_
All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	12.9	11.6		10.9	9.2		34.1	35.1		31.3	26.0	

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11/02/2017

966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	→	*	4	Ļ	•	•	t	1	1	Ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Actuated g/C Ratio	0.25	0.22		0.21	0.18		0.66	0.68		0.61	0.50	
v/c Ratio	0.17	0.12		0.01	0.08		0.19	0.40		0.08	0.40	
Control Delay	19.4	24.1		19.2	29.4		6.9	11.7		6.8	14.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	19.4	24.1		19.2	29.4		6.9	11.7		6.8	14.2	
LOS	В	С		В	С		А	В		А	В	
Approach Delay		21.2			27.9			11.3			13.7	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)	15	9		1	5		10	57		4	76	
Queue Length 95th (ft)	55	48		8	32		43	263		21	179	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	506	696		440	621		677	3256		634	3135	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.13	0.06		0.01	0.04		0.16	0.30		0.07	0.22	
Intersection Summary												
Area Type:	Other											
Cycle Length: 120												
Actuated Cycle Length: 51.7	,											
Natural Cycle: 75												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.40												
Intersection Signal Delay: 13				Int	tersection	LOS: B						
Intersection Capacity Utilization	tion 52.5%			IC	U Level o	of Service	A					
Analysis Period (min) 15												

opilis and Fliases.	900. IN FOR Street/Filleville Road & Industrial Drive/Driveway	
•ø1	√ ¶ Ø2	√ Ø3

Ø1	₫ ø2	√ Ø3	ø₄
16 s	66 s	14 s	24 s
▲ ø5	▶ Ø6	∕ ∕ ø7	₩ Ø8
16 s	66 s	16 s	22 s

11/03/2017

	→	\mathbf{r}	∢	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			र्स	Y		
Traffic Volume (veh/h)	33	40	10	41	30	4	
Future Volume (Veh/h)	33	40	10	41	30	4	
Sign Control	Free			Free	Stop		
Grade	2%			-1%	5%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	37	44	11	46	33	4	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			81		127	59	
vC1, stage 1 conf vol			•				
vC2, stage 2 conf vol							
vCu, unblocked vol			81		127	59	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)						ş. _	
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		96	100	
cM capacity (veh/h)			1517		861	1007	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	81	57	37				
Volume Left	0	11	33				
Volume Right	44	0	4				
cSH	1700	1517	875				
Volume to Capacity	0.05	0.01	0.04				
Queue Length 95th (ft)	0.00	1	3				
Control Delay (s)	0.0	1.5	9.3				
Lane LOS	0.0	1.5 A	9.3 A				
Approach Delay (s)	0.0	1.5	9.3				
Approach LOS	0.0	1.5	9.3 A				
••			А				
Intersection Summary							
Average Delay			2.4				
Intersection Capacity Utilization	ation		19.4%	IC	U Level c	f Service	1
Analysis Period (min)			15				

Lane Group EBL EBL EBR WBL WBT WBR NBL NBT NBT NBT SBL SBR SBR Lane Configurations P<	966: N Polk Street/Pineville Road & Industrial Drive/Driveway													
Lane Configurations $\begin{tabular}{lllllllllllllllllllllllllllllllllll$		٦	+	*	4	Ļ	×	•	1	*	1	Ļ	- √	
Traffic Volume (vph) 169 4 78 9 1 40 33 762 10 7 1117 79 Future Volume (vph) 160 1900	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Traffic Volume (vph) 169 4 78 9 1 40 33 762 10 7 1117 79 Future Volume (vph) 1900	Lane Configurations	۲	4î		۲	4î		٦	∱ ⊅		۲	≜ 1≱		
Future Volume (vph) 169 4 78 9 1 40 33 762 10 7 1117 79 Ideal Flow (vph) 1900 <td< td=""><td></td><td>169</td><td>4</td><td>78</td><td>9</td><td>1</td><td>40</td><td>33</td><td>762</td><td>10</td><td>7</td><td>1117</td><td>79</td></td<>		169	4	78	9	1	40	33	762	10	7	1117	79	
Ideal Flow (php) 1900 100<			4	78		1				10	7			
Storage Length (th) 150 0 1 1 1 0 1 1 1 0 1 1 0 1			1900	1900	1900	1900	1900	1900		1900	1900		1900	
Storage Lanes 1 0 1 1 1 1 0 1 0 Taper Length (ft) 25 25 25 25 25 25 Lane Uhl, Factor 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.95 0.955 Fit 0.050 0.050 0.950 0.950 0.950 0.950 Stat, Flow (port) 1770 1596 0 1770 1589 0 1770 3532 0 1770 3504 0 Right Tum on Red No No No No No No No Stat. Flow (proth) 35 35 35 35 35 35 165 103 11744 111 8 124 88 114 11 8 11 8 124 88 11 14 37 847 11 8 132 0 114 37 847 11 8											165			
Tape: Length (ft) 25 25 25 25 Lane Util, Factor 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.950 0.950 Fit Protected 0.950 0.950 0.950 0.950 0.950 0.950 0.950 Stdt. Flow (pert) 1770 1596 0 1707 3532 0 1770 3532 0 504 0 Stdt. Flow (pert) 777 1596 0 1300 1589 0 183 3532 0 503 350 0 Stdt. Flow (pert) 771 1596 0 1300 158 0.098 0.273 0 Stdt. Flow (pht) 355 35 35 35 35 104 1 144 37 847 11 8 14 1 144 37 847 11 8 1241 88 116 166 166 166 166 16 16 16				0	1					0	1			
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.950 0.990 Fit Protected 0.950 0.50 0.50 0.55 355 355 355 355 1632 1043 155 0.95 0.90	J. J	25			25			25			25			
Fit Protected 0.950 0.950 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1596 0 1770 1589 0 1770 3504 0 Fit Permitted 0.414 0.688 0.088 0.273 0 509 3504 0 Right Turn on Red No		1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95	
Satd. Flow (prot) 1770 1596 0 1770 1596 0 1770 1596 0 1770 3532 0 1770 3504 0 FIt Permitted 0.414 0.698 0.098 0.027 0.0273 0.0273 0.0273 0.0273 0.0273 0.0273 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.0213 0.020	Frt		0.857			0.853			0.998			0.990		
Fit Permitted 0.414 0.698 0.098 0.273 Satd. Flow (perm) 771 1596 0 1300 1589 0 183 3532 0 509 3504 0 Right Turn on Red No No No No No No No No Link Spearce (ft) 1961 266 1652 1043 35 35 35 35 35 35 361 362 1030 144 37 847 11 8 1241 88 Shared Lane Traffic (%) 188 4 87 10 1 44 37 847 11 8 1241 88 Shared Lane Traffic (%) 10 10 45 0 37 858 0 8 1329 0 10 100 100 No <no< td=""> No<no< td=""> No No 11 8 1241 88 1241 18 1241 18 1241 18 1241 <td< td=""><td>Flt Protected</td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td></td<></no<></no<></no<></no<></no<></no<></no<></no<></no<></no<></no<>	Flt Protected	0.950			0.950			0.950			0.950			
Fit Permitted 0.414 0.698 0.098 0.273 Satd. Flow (perm) 771 1596 0 1300 1589 0 183 3532 0 509 3504 0 Right Turn on Red No No No No No No No No Link Spisance (ft) 1961 266 1652 1043 3 <td>Satd. Flow (prot)</td> <td>1770</td> <td>1596</td> <td>0</td> <td>1770</td> <td>1589</td> <td>0</td> <td>1770</td> <td>3532</td> <td>0</td> <td>1770</td> <td>3504</td> <td>0</td>	Satd. Flow (prot)	1770	1596	0	1770	1589	0	1770	3532	0	1770	3504	0	
Satd. Flow (perm) 771 1596 0 1300 1589 0 183 3532 0 509 3504 0 Right Turn on Red No No No No No No No Link Speed (mph) 35 35 35 35 35 1043 Travel Time (s) 382 52 32.2 20.3 Peak Hour Factor 0.90	(i)	0.414			0.698			0.098			0.273			
Right Turn on Red No No No No No No No Stadt.Flow (RTOR)	Satd. Flow (perm)		1596	0	1300	1589	0	183	3532	0	509	3504	0	
Satcl. Flow (RTOR) Link Speed (mph) 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90				No			No			No			No	
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 382 52 322 20.3 Peak Hour Factor 0.90 1.90 1.00														
Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 D.90 1.1 44 37 847 11 8 124 88 DETA DetA DA No	· /		35			35			35			35		
Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 0.	,		1961			266			1652			1043		
Peak Hour Factor 0.90	()		38.2			5.2			32.2			20.3		
Adj. Flow (vph) 188 4 87 10 1 44 37 847 11 8 1241 88 Shared Lane Traffic (%) 188 91 0 10 45 0 37 858 0 8 1329 0 Lane Group Flow (vph) 188 91 0 10 45 0 37 858 0 8 1329 0 Enter Blocked Intersection No	()	0.90		0.90	0.90	0.90	0.90	0.90		0.90	0.90		0.90	
Shared Lane Traffic (%) Lane Group Flow (vph) 188 91 0 10 45 0 37 858 0 8 1329 0 Enter Blocked Intersection No N	Adj. Flow (vph)													
Lane Group Flow (vph) 188 91 0 10 45 0 37 858 0 8 1329 0 Enter Blocked Intersection No	2 (1)				-				-		-			
Enter Blocked Intersection No No <th< td=""><td></td><td>188</td><td>91</td><td>0</td><td>10</td><td>45</td><td>0</td><td>37</td><td>858</td><td>0</td><td>8</td><td>1329</td><td>0</td></th<>		188	91	0	10	45	0	37	858	0	8	1329	0	
Lane Alignment Left Left Right				No						No				
Median Width(ft) 12 12 12 12 12 12 12 Link Offset(ft) 0	Lane Alignment			Right	Left			Left			Left			
Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Tum Lane 1.00	Ţ		12	J			J		12	Ū			Ū	
Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane			0			0			0			0		
Two way Left Turn Lane Yes Headway Factor 1.00 <			16			16			16			16		
Headway Factor1.00<	()											Yes		
Turning Speed (mph)1591591591599Turn Typepm+ptNApm+ptNApm+ptNApm+ptNAProtected Phases74385216Detector Phase74385216Detector Phase74385216Switch Phase77.07.07.07.010.07.010.0Minimum Initial (s)7.07.07.07.010.014.021.014.021.0Minimum Split (s)14.020.014.022.016.066.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%59.7Yellow Time (s)3.03.83.03.83.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Turn Type pm+pt NA pm+pt NA pm+pt NA pm+pt NA Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (s) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8		15		9	15		9	15		9	15		9	
Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5		pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA		
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 -1.3 -1.3 <td></td> <td></td> <td>4</td> <td></td> <td></td> <td>8</td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td> <td></td>			4			8			2			6		
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 -1.3 -1.3 <td>Permitted Phases</td> <td>4</td> <td></td> <td></td> <td>8</td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td> <td></td> <td></td>	Permitted Phases	4			8			2			6			
Minimum Initial (s)7.07.07.07.07.010.07.010.0Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin		7	4		3	8		5	2		1	6		
Minimum Split (s)14.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.03.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Switch Phase													
Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0		
Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0		
Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 <t< td=""><td>Total Split (s)</td><td>16.0</td><td>24.0</td><td></td><td>14.0</td><td>22.0</td><td></td><td>16.0</td><td>66.0</td><td></td><td>16.0</td><td>66.0</td><td></td></t<>	Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0		
Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%		
All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min	Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7		
Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0	Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8		
Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5		
Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3		
Lead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0		
Lead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin						Lag		Lead			Lead			
Vehicle Extension (s) 2.0 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min		Yes			Yes			Yes			Yes			
Recall Mode None None None None Min None Min		2.0			2.0	2.0		2.0	3.0		2.0	3.0		
		None	None		None	None		None	Min		None	Min		
	Act Effct Green (s)	18.5	16.9		13.2	10.0		42.9	41.8		41.5	37.5		

2017 Existing PM Peak Hour Timmons Group Synchro 9 Report Page 1

Pineville Industrial TIA 966: N Polk Street/Pineville Road & Industrial Drive/Driveway

966: N Polk Street/I	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	-	\mathbf{r}	4	-	•	1	Ť	1	1	Ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Actuated g/C Ratio	0.25	0.23		0.18	0.14		0.58	0.57		0.56	0.51	
v/c Ratio	0.54	0.25		0.03	0.21		0.12	0.43		0.02	0.75	
Control Delay	33.2	32.1		27.1	41.3		7.9	11.0		7.1	19.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	33.2	32.1		27.1	41.3		7.9	11.0		7.1	19.7	
LOS	С	С		С	D		А	В		А	В	
Approach Delay		32.9			38.7			10.9			19.6	
Approach LOS		С			D			В			В	
Queue Length 50th (ft)	80	37		4	23		7	115		2	316	
Queue Length 95th (ft)	170	106		18	63		20	242		7	444	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	376	517		331	425		382	2795		513	2773	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.50	0.18		0.03	0.11		0.10	0.31		0.02	0.48	
Intersection Summary												
21	Other											
Cycle Length: 120												
Actuated Cycle Length: 73.9												
Natural Cycle: 80												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.75												
Intersection Signal Delay: 18					tersection							
Intersection Capacity Utilizat Analysis Period (min) 15	ion 57.8%			IC	U Level o	of Service	В					

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16 s	66 s	14 s	24 s
* ø5		▶ _{Ø7}	₩ Ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

Item 7.

	→	\mathbf{F}	4	-	1	۲
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	¢.			4	Y	
Traffic Volume (veh/h)	72	32	3	80	40	10
Future Volume (Veh/h)	72	32	3	80	40	10
Sign Control	Free			Free	Stop	
Grade	2%			-1%	5%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	80	36	3	89	44	11
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			116		193	98
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			116		193	98
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		94	99
cM capacity (veh/h)			1473		794	958
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	116	92	55			
Volume Left	0	3	44			
Volume Right	36	0	11			
cSH	1700	1473	822			
Volume to Capacity	0.07	0.00	0.07			
Queue Length 95th (ft)	0	0	5			
Control Delay (s)	0.0	0.3	9.7			
Lane LOS		А	А			
Approach Delay (s)	0.0	0.3	9.7			
Approach LOS			А			
Intersection Summary						
Average Delay			2.1			
Intersection Capacity Utiliza	ation		16.6%	IC	U Level o	f Service
Analysis Period (min)			10.070	10		
			15			

2019 Phase I Background Traffic Volumes

Lane Group EBL EBT EBR WBL WBT WBR NBT NBT NBT SBL SBT SBR Lane Configurations 1 0 1 0 0 0	966: N Polk Street/Pineville Road & Industrial Drive/Driveway												
Lane Configurations 1 0 1 1 0 1 1 1 1 0 1 1 1 0 1 1 1 0 1 0 1 0 1 1 1 0 0 0		٦	-	\mathbf{r}	4	+	×	1	1	1	1	ţ	~
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume ("ph) 107 0 57 4 2 19 108 833 15 40 470 175 ideal Flow (wphp) 1900 100 <	Lane Configurations	<u>۲</u>	4î		<u>۲</u>	4î		٦	≜ ⊅		۳.	≜ ⊅	
Ideal Flow (vphp) 1900 <td>Traffic Volume (vph)</td> <td>107</td> <td>0</td> <td>57</td> <td>4</td> <td>2</td> <td>19</td> <td>108</td> <td>893</td> <td>15</td> <td>40</td> <td>470</td> <td>175</td>	Traffic Volume (vph)	107	0	57	4	2	19	108	893	15	40	470	175
Storage Length (ft) 150 0 0 1 1 1 0 1 1 0 1 1 0 1 0 1 1 1 0 1 1 0 1 1 1 1 1 1 1	Future Volume (vph)	107	0	57	4	2	19	108	893	15	40	470	175
Storage Lange 1 0 1 1 1 1 0 1 0 Taper Length (ft) 25	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Taper Length (ft) 25 25 25 25 Lane Util, Factor 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.95 0.95 Fit 0.850 0.950 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1583 0 1608 0 548 3529 0 376 3394 0 Satd. Flow (prot) 1770 1583 0 1863 1008 0 548 3529 0 376 3334 0 Satd. Flow (prot) 877 1583 0 1863 1008 0 548 352 355 355 1100 353 355 1103 1033 1003 1043 376 355 355 1103 1033 1009 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90	Storage Length (ft)	150		0	0		75	100		0	165		0
Lane Uli, Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 1.00 0.95 Frt 0.860 0.950 0.920 0.920 0.920 0.920 0.920 0.920 0.920 0.920 0.930 0.90	Storage Lanes	1		0	1		1	1		0	1		0
Fri 0.850 0.863 0.997 0.950 0.950 Fil Protected 0.950 0.950 0.950 0.950 0.950 0.950 Fil Premitted 0.471 1770 1583 0 1770 3529 0 1770 3524 0 0.202 Satd. Flow (perm) 877 1583 0 1863 1608 0 548 3529 0 376 3394 0 Satd. Flow (perm) 877 1583 0 1863 1608 0 548 3529 0 376 3394 0 Link Speed (mph) 35 35 35 35 35 35 35 1043 Travel Time (S) 38.2 5.2 32.2 20.3 1043 1009 0.44 522 194 Lene Alonent Faftic (%) 119 63 0 4 23 0 120 1009 0 44 76 0 Link Offset(th)	Taper Length (ft)	25			25			25			25		
Fit Protected 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1583 0 1770 1608 0 1770 3529 0 1770 3394 0 Fit Permitted 0.471	Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Satd. Flow (prot) 1770 1583 0 1770 1608 0 1770 3529 0 1770 3394 0 FIt Permitted 0.471 0.284 0.294 0.202 0.203 0.90	Frt		0.850			0.863			0.997			0.959	
Fit Permitted 0.471 0.294 0.202 Satd. Flow (perm) 877 1583 0 1863 1608 0 548 3529 0 376 3394 0 Right Turn on Red No No No No No No No Link Speed (mph) 35 35 35 35 35 35 Peak Hour Factor 0.90 Casa Ditto fito f	Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (perm) 877 1583 0 1863 1608 0 548 3529 0 376 3394 0 Right Turn on Red No No No No No No No No Link Speed (mph) 35 35 35 35 104 Jone Jone <td>Satd. Flow (prot)</td> <td>1770</td> <td>1583</td> <td>0</td> <td>1770</td> <td>1608</td> <td>0</td> <td>1770</td> <td>3529</td> <td>0</td> <td>1770</td> <td>3394</td> <td>0</td>	Satd. Flow (prot)	1770	1583	0	1770	1608	0	1770	3529	0	1770	3394	0
Right Turn on Red No No No No No No No Stadt. Flow (RTOR)	Flt Permitted	0.471						0.294			0.202		
Said. Flow (RTOR) Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 1.00 1.00	Satd. Flow (perm)	877	1583	0	1863	1608	0	548	3529	0	376	3394	0
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 20.3 Peak Hour Factor 0.90	Right Turn on Red			No			No			No			No
Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 0.70 0	Satd. Flow (RTOR)												
Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 0.44 716 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 </td <td>Link Speed (mph)</td> <td></td> <td>35</td> <td></td> <td></td> <td>35</td> <td></td> <td></td> <td>35</td> <td></td> <td></td> <td>35</td> <td></td>	Link Speed (mph)		35			35			35			35	
Peak Hour Factor 0.90	Link Distance (ft)		1961			266			1652			1043	
Àdj. Flow (vph) 119 0 63 4 2 21 120 992 17 44 522 194 Shared Lane Traffic (%) Lane Group Flow (vph) 119 63 0 4 23 0 120 1009 0 44 716 0 Enter Blocked Intersection No	Travel Time (s)		38.2			5.2			32.2			20.3	
Shared Lane Traffic (%) Lane Group Flow (vph) 119 63 0 4 23 0 120 1009 0 44 716 0 Enter Blocked Intersection No	Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%) Lane Group Flow (vph) 119 63 0 4 23 0 120 1009 0 44 716 0 Enter Blocked Intersection No	Adj. Flow (vph)	119	0	63	4	2	21	120	992	17	44	522	194
Lane Group Flow (vph) 119 63 0 4 23 0 120 1009 0 44 716 0 Enter Blocked Intersection No N													
Enter Blocked Intersection No No <th< td=""><td>· · · · · · · · · · · · · · · · · · ·</td><td>119</td><td>63</td><td>0</td><td>4</td><td>23</td><td>0</td><td>120</td><td>1009</td><td>0</td><td>44</td><td>716</td><td>0</td></th<>	· · · · · · · · · · · · · · · · · · ·	119	63	0	4	23	0	120	1009	0	44	716	0
Median Width(ft) 12 12 12 12 12 12 12 Link Offset(ft) 0	,	No	No	No	No	No	No	No	No	No	No	No	No
Median Width(ft) 12 12 12 12 12 12 Link Offset(ft) 0	Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Crosswalk Width(ft) 16 16 16 16 Two way Left Tum Lane	-		12	Ū		12	Ū		12	Ū		12	Ū
Crosswalk Width(ft) 16 16 16 16 Two way Left Tum Lane	Link Offset(ft)		0			0			0			0	
Headway Factor 1.00			16			16			16			16	
Headway Factor 1.00	Two way Left Turn Lane											Yes	
Turning Speed (mph) 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 9 15 15 15 15 15 15 15 15 15 15 15 15 15 15 15 15 15 15 15 16 16 Permitted Phases 4 8 2 6 10.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 10.0 7.0 10.0 7.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 <td< td=""><td></td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td><td>1.00</td></td<>		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turn Type pm+pt NA pm+pt NA pm+pt NA pm+pt NA Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.		15		9	15		9	15		9	15		9
Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 <td< td=""><td></td><td>pm+pt</td><td>NA</td><td></td><td>pm+pt</td><td>NA</td><td></td><td>pm+pt</td><td>NA</td><td></td><td>pm+pt</td><td>NA</td><td></td></td<>		pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 2.5 3.3 2.5 Lost Time (s)	Protected Phases		4			8			2			6	
Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 1.0	Permitted Phases	4			8			2			6		
Minimum Initial (s)7.07.07.07.07.010.07.010.0Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time (s)5.05.05.05.05.05.05.05.0Loat Time (s)5.05.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Detector Phase	7	4		3	8		5	2		1	6	
Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.03.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Switch Phase												
Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 <t< td=""><td>Total Split (s)</td><td>16.0</td><td>24.0</td><td></td><td>14.0</td><td>22.0</td><td></td><td>16.0</td><td>66.0</td><td></td><td>16.0</td><td>66.0</td><td></td></t<>	Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min	Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0	Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	
Total Lost Time (s)5.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.0Recall ModeNoneNoneNoneNoneMinNoneMin		2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Lead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin		5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Recall Mode None None None None Min None Min	Lead-Lag Optimize?	Yes						Yes			Yes		
	Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Act Effct Green (s) 13.8 12.2 10.7 8.9 34.8 32.3 33.4 28.8	Recall Mode	None	None		None			None			None	Min	
	Act Effct Green (s)	13.8	12.2		10.7	8.9		34.8	32.3		33.4	28.8	

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Pineville Industrial TIA atrial Drivo/Driv OGG: N. Dolle Street/Dine ille Deed 9 Ind ... _

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Actuated g/C Ratio	0.24	0.21		0.19	0.15		0.60	0.56		0.58	0.50	
v/c Ratio	0.33	0.19		0.01	0.09		0.23	0.51		0.10	0.42	
Control Delay	23.1	26.0		20.5	32.0		7.8	15.1		7.3	15.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	23.1	26.0		20.5	32.0		7.8	15.1		7.3	15.6	
LOS	С	С		С	С		А	В		А	В	
Approach Delay		24.1			30.3			14.3			15.1	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)	34	18		1	6		12	137		4	85	
Queue Length 95th (ft)	89	67		9	34		48	282		22	196	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	436	609		387	536		604	3197		531	3075	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.27	0.10		0.01	0.04		0.20	0.32		0.08	0.23	
Intersection Summary												
31	Other											
Cycle Length: 120												
Actuated Cycle Length: 57.8												
Natural Cycle: 75												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 0.51												
Intersection Signal Delay: 15					tersectior							
Intersection Capacity Utilizat Analysis Period (min) 15	ion 56.1%			IC	U Level o	of Service	В					

Ø1		√ Ø3	ø₄
16 s	66 s	14 s	24 s
▲ ø5		∕ ∕ ø7	₩ Ø8
16 s	66 s	16 s	22 s

	→	\mathbf{r}	4	←	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f,			र्स	Y	
Traffic Volume (veh/h)	34	43	10	43	43	4
Future Volume (Veh/h)	34	43	10	43	43	4
Sign Control	Free			Free	Stop	
Grade	2%			-1%	5%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	38	48	11	48	48	4
Pedestrians						·
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)				110110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			86		132	62
vC1, stage 1 conf vol			00		102	02
vC2, stage 2 conf vol						
vCu, unblocked vol			86		132	62
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					5.1	5.2
tF (s)			2.2		3.5	3.3
p0 queue free %			99		94	100
cM capacity (veh/h)			1510		855	1003
					000	1000
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	86	59	52			
Volume Left	0	11	48			
Volume Right	48	0	4			
cSH	1700	1510	865			
Volume to Capacity	0.05	0.01	0.06			
Queue Length 95th (ft)	0	1	5			
Control Delay (s)	0.0	1.4	9.4			
Lane LOS		А	А			
Approach Delay (s)	0.0	1.4	9.4			
Approach LOS			А			
Intersection Summary						
Average Delay			2.9			
Intersection Capacity Utiliz	zation		19.5%	IC	U Level o	of Service
Analysis Period (min)	-		15			
Analysis Period (min)			15			

966: N Polk Street/Pineville Road & Industrial Drive/Driveway												
	الحر	-	\mathbf{r}	4	+	×	1	Ť	1	1	ŧ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	¢î		<u>۲</u>	¢î		۲.	≜ ⊅		ሻ	≜ ⊅	
Traffic Volume (vph)	197	4	95	9	1	42	44	793	10	7	1162	127
Future Volume (vph)	197	4	95	9	1	42	44	793	10	7	1162	127
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.855			0.853			0.998			0.985	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1593	0	1770	1589	0	1770	3532	0	1770	3486	0
Flt Permitted	0.431			0.686			0.082			0.278		
Satd. Flow (perm)	803	1593	0	1278	1589	0	153	3532	0	518	3486	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	219	4	106	10	0.00	47	49	881	11	8	1291	141
Shared Lane Traffic (%)	215	т	100	10		1	75	001		0	1231	171
Lane Group Flow (vph)	219	110	0	10	48	0	49	892	0	8	1432	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Leit	12	Night	Len	12	rugiit	Leit	12	Night	Len	12	Tugitt
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	1.00	1.00	1.00	9	1.00	1.00	1.00	1.00	1.00	9
Turn Type	pm+pt	NA	9	pm+pt	NA	9	pm+pt	NA	9	pm+pt	NA	9
Protected Phases	ρπ+ρι 7	4		рш+рі 3	8		5 pint-pi	2		ριπ+ρι 1	6	
Permitted Phases	4	4		8	0		2	2		6	0	
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase	I	4		5	0		J	2		1	0	
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
	14.0	20.0		14.0	20.0		14.0	66.0		14.0	66.0	
Total Split (s)		24.0		14.0	18.3%			55.0%			55.0%	
Total Split (%)	13.3% 10.2			8.6	16.3%		13.3%	55.0% 59.7		13.3% 9.7	55.0% 59.7	
Maximum Green (s)	3.0	18.2 3.8		0.0 3.0	3.8		10.4 3.0	59.7 3.8		9.7 3.0	3.8	
Yellow Time (s)	2.8	3.0 2.0		2.4	3.0 2.0		2.6	3.8 2.5		3.3	3.0 2.5	
All-Red Time (s)	2.0 -0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Lost Time Adjust (s)								-1.3 5.0				
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0			5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	19.4	17.8		13.8	10.5		51.0	49.7		48.2	42.1	

2019 Background PM Peak Hour Timmons Group

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Pineville Industrial TIA atrial Driv

11/02/2017

966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Actuated g/C Ratio	0.23	0.21		0.17	0.13		0.62	0.60		0.58	0.51	
v/c Ratio	0.67	0.32		0.04	0.24		0.18	0.42		0.02	0.81	
Control Delay	43.3	37.2		29.9	45.1		8.2	10.6		7.1	22.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	43.3	37.2		29.9	45.1		8.2	10.6		7.1	22.7	
LOS	D	D		С	D		А	В		А	С	
Approach Delay		41.3			42.5			10.4			22.6	
Approach LOS		D			D			В			С	
Queue Length 50th (ft)	105	50		4	26		10	123		2	363	
Queue Length 95th (ft)	#238	133		20	71		26	261		8	527	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	346	466		304	380		345	2611		507	2555	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.63	0.24		0.03	0.13		0.14	0.34		0.02	0.56	
Intersection Summary												
71	Other											
Cycle Length: 120												
Actuated Cycle Length: 82.9												
Natural Cycle: 90												
Control Type: Actuated-Unco	pordinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 21					tersectior		_					
Intersection Capacity Utilizat	ion 62.5%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
# 95th percentile volume e Queue shown is maximur			eue may l	be longer								

Ø1	™ ø2	🗸 Ø3	<u> </u>
16 s	66 s	14 s	24 s
★ ø5	↓ ø ₆		★ ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

	→	$\mathbf{\hat{z}}$	4	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ţ.			र्स	Y		
Traffic Volume (veh/h)	75	43	3	83	45	10	
Future Volume (Veh/h)	75	43	3	83	45	10	
Sign Control	Free			Free	Stop		
Grade	2%			-1%	5%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	83	48	3	92	50	11	
Pedestrians			Ū				
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)	Nono			None			
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			131		205	107	
vC1, stage 1 conf vol			101		200	107	
vC2, stage 2 conf vol							
vCu, unblocked vol			131		205	107	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)			7.1		. .	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		94	99	
cM capacity (veh/h)			1454		781	947	
					701	577	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	131	95	61				
Volume Left	0	3	50				
Volume Right	48	0	11				
cSH	1700	1454	807				
Volume to Capacity	0.08	0.00	0.08				
Queue Length 95th (ft)	0	0	6				
Control Delay (s)	0.0	0.3	9.8				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.3	9.8				
Approach LOS			А				
Intersection Summary							
Average Delay			2.2				
Intersection Capacity Utiliz	zation		2.2 16.8%			of Service	
	auon			iU		I SEIVICE	
Analysis Period (min)			15				

2024 Phase II Background Traffic Volumes

Lane Group EBL EBL EBR WBL WBT WBR NBL NBT NBT NBT SBL SBT SBR Lane Configurations 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 0 1 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	966: N Polk Street/I	Pinevill	e Road	d & Inc	lustrial	Drive/	Drivev	vay				11/0)3/2017
Lane Configurations Y P Y PD Y PD Y PD Traffic Volume (vph) 134 0 71 5 2 21 186 986 16 44 519 238 Ideal Flow (vph) 1900 1800 100 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 <th></th> <th>٦</th> <th>-</th> <th>\mathbf{r}</th> <th>4</th> <th>+</th> <th>•</th> <th>1</th> <th>Ť</th> <th>1</th> <th>1</th> <th>Ļ</th> <th>-</th>		٦	-	\mathbf{r}	4	+	•	1	Ť	1	1	Ļ	-
Traffic Volume (vph) 134 0 71 5 2 21 186 986 16 44 519 238 Future Volume (vph) 1900	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph) 134 0 71 5 2 21 186 986 16 44 519 238 Future Volume (vph) 1900	Lane Configurations	۲	4Î		۲	4Î		ሻ	∱ î≽		٦	∱ î≽	
ideal Flow (rph) 1900	Traffic Volume (vph)	134	0	71	5	2	21	186	986	16	44	519	238
Storage Length (ft) 150 0 1 1 1 0 1 1 0 1	Future Volume (vph)	134	0	71	5	2	21	186	986	16	44	519	238
Storage Lanes 1 0 1 1 1 1 0 0 0 0 0 0 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 <th1< th=""> 1 1 <t< td=""><td>Ideal Flow (vphpl)</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td></t<></th1<>	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Shorage Lanes 1 0 1 1 1 1 0 1 0 1 Taper Length (ft) 25 233 373 0 1 1 1 1 1 1 1 0 1 373 0 1 <td>Storage Length (ft)</td> <td>150</td> <td></td> <td>0</td> <td>0</td> <td></td> <td>75</td> <td>100</td> <td></td> <td>0</td> <td>165</td> <td></td> <td>0</td>	Storage Length (ft)	150		0	0		75	100		0	165		0
Taper Length (th) 25 25 25 25 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 0.95 0.95 0.95 0.95 0.950 0.90 0.		1		0	1		1	1		0	1		0
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 1.00 0.95 0.95 Frt Protected 0.950 0.950 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1583 0 1770 1606 0 374 3373 0 Right Turn on Red 361 1583 0 1863 1606 0 346 3532 0 1770 3532 Satd. Flow (prot) 355 35 35 35 35 35 35 35 1043 1044 104 104 104 104 104 104 1044 1044 <td></td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td> <td>25</td> <td></td> <td></td>		25			25			25			25		
Fit Protected 0.950 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1583 0 1770 166 0 1770 3373 0 Righ Turn on Red No No <td></td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>0.95</td> <td>1.00</td> <td>0.95</td> <td>0.95</td>		1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Satd. Flow (prot) 1770 1583 0 1770 1606 0 1770 3532 0 1770 3373 0 FIP Permitted 0.449 0 1606 0 346 5532 0 332 3373 0 Right Turn on Red No	Frt		0.850			0.862			0.998			0.953	
Fit Permitted 0.449 0.186 0.178 Satd. Flow (perm) 836 1583 0 1863 1606 0 346 352 0 332 3373 0 Satd. Flow (RTOR) No No No No No No No Link Dstance (ft) 1961 266 1652 1043 1043 Travel Time (s) 38.2 5.2 32.2 20.3 20.3 Peak Hour Factor 0.90 <t< td=""><td>Flt Protected</td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td><td>0.950</td><td></td><td></td></t<>	Flt Protected	0.950			0.950			0.950			0.950		
Fit Permitted 0.449 0.186 0.178 Satd. Flow (perm) 836 1583 0 1863 1606 0 346 3532 0 332 3373 0 Satd. Flow (RTOR) No No No No No No No No Link Spead (mph) 35 35 35 35 35 20.3 20.3 Travel Time (s) 38.2 5.2 32.2 20.3 20.3 20.4 Aj, Flow (vph) 149 0.90	Satd. Flow (prot)	1770	1583	0	1770	1606	0	1770	3532	0	1770	3373	0
Right Turn on Red No No No No No No Satd. Flow (RTOR) Link Speed (mph) 35 35 35 35 35 Link Speed (mph) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 1.90 1.00 1.00 1.00 1.00		0.449						0.186			0.178		
Right Turn on Red No No No No No No No No Stadt, Flow (RTOR) 1165 Speed (mph) 35 35 35 35 35 Link Distance (th) 1961 266 1652 1043 1043 Pravel Hour Factor 0.90 1.60 1.60	Satd. Flow (perm)	836	1583	0	1863	1606	0	346	3532	0	332	3373	0
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90				No			No			No			No
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90	Satd. Flow (RTOR)												
Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 0.70 0.70 0.70 0.70 0.00 0.00 0.00 0.00	, ,		35			35			35			35	
Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 0.			1961									1043	
Peak Hour Factor 0.90	()		38.2			5.2			32.2			20.3	
Adj. Flow (vph) 149 0 79 6 2 23 207 1096 18 49 577 264 Shared Lane Traffic (%) Lane Group Flow (vph) 149 79 0 6 25 0 207 1114 0 49 841 0 Lane Group Flow (vph) 149 79 0 6 25 0 207 1114 0 49 841 0 Lane Group Flow (vph) 149 79 0 6 25 0 207 1114 0 49 841 0 Lane Alignment Left Left Right Left R		0.90		0.90	0.90	0.90	0.90	0.90		0.90	0.90		0.90
Shared Lane Traffic (%) Lane Group Flow (vph) 149 79 0 6 25 0 207 1114 0 49 841 0 Enter Blocked Intersection No	Adj. Flow (vph)	149		79	6	2	23	207	1096	18	49	577	
Lane Group Flow (vph) 149 79 0 6 25 0 207 1114 0 49 841 0 Enter Blocked Intersection No N	2 (1)												
Enter Blocked Intersection No No <th< td=""><td></td><td>149</td><td>79</td><td>0</td><td>6</td><td>25</td><td>0</td><td>207</td><td>1114</td><td>0</td><td>49</td><td>841</td><td>0</td></th<>		149	79	0	6	25	0	207	1114	0	49	841	0
Lane Alignment Left Left Right			No	No	No	No	No		No	No	No	No	
Median Width(ft) 12 12 12 12 12 Link Offset(ft) 0	Lane Alignment	Left	Left	Right	Left	Left	Right	Left		Right	Left	Left	Right
Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane Yes Yes Yes Headway Factor 1.00 <td>-</td> <td></td> <td></td> <td>5</td> <td></td> <td></td> <td>Ū</td> <td></td> <td></td> <td>0</td> <td></td> <td></td> <td>J</td>	-			5			Ū			0			J
Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane Yes Headway Factor 1.00	. ,		0			0			0			0	
Headway Factor 1.00	Crosswalk Width(ft)		16			16			16			16	
Headway Factor 1.00<	()											Yes	
Turning Speed (mph) 15 9 15 16 Protected Phases 7 4 3 8 5 2 1 6 Detector Phase 7 4 3 8 5 2 1 6 6 Switch Phase 7 7.0 7.0 7.0 7.0 10.0 7.0 10.0 7.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 <td></td> <td>1.00</td>		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turn Type pm+pt NA pm+pt NA pm+pt NA pm+pt NA Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8	Turning Speed (mph)	15		9	15		9	15		9	15		9
Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 5 2 1 6 5 3 8 5 2 1 6 5 3 6 5 2 1 6 5 5 3 6 6 7 4 3 8 5 2 1 6 6 6 6 6 6 6 7 10.0 7.0 10.0 7.0 10.0 7.0 10.0 7.0 10.0 7.0 10.0 14.0 21.0 14.0 21.0 14.0 21.0 13.3% 55.0% 13.3% 55.0% 13.3% 55.0% 13.3% 55.0% 13.3%		pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Initial (s) 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 5.0 <td></td>													
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Initial (s) 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 5.0 <td>Permitted Phases</td> <td>4</td> <td></td> <td></td> <td>8</td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td> <td></td> <td></td>	Permitted Phases	4			8			2			6		
Minimum Initial (s)7.07.07.07.07.010.07.010.0Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.03.0Recall ModeNoneNoneNoneNoneNoneNoneMinNoneMin	Detector Phase	7	4			8		5	2		1	6	
Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead/Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Switch Phase												
Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin	Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.3 1.4 1.4 1.4 <t< td=""><td></td><td>16.0</td><td>24.0</td><td></td><td>14.0</td><td>22.0</td><td></td><td>16.0</td><td>66.0</td><td></td><td>16.0</td><td>66.0</td><td></td></t<>		16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0	Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min		10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min		3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	
Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0								2.6					
Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin					-0.4								
Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin		5.0			5.0			5.0			5.0		
Lead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	.,												
Vehicle Extension (s) 2.0 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min	•												
Recall Mode None None None None Min None Min	•				2.0								
					None	None		None			None		
	Act Effct Green (s)	14.5	12.9		10.7			39.0	33.5		33.6	24.7	

2024 Background AM Peak Hour Timmons Group

Pineville Industrial TIA

Synchro 9 Report . Page 1

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966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/201
	٦	-	\mathbf{r}	*	-	•	1	Ť	1	\	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Actuated g/C Ratio	0.22	0.20		0.16	0.13		0.59	0.51		0.51	0.38	
v/c Ratio	0.45	0.26		0.02	0.12		0.48	0.62		0.13	0.66	
Control Delay	28.0	29.2		23.0	34.6		10.8	16.3		7.7	20.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	28.0	29.2		23.0	34.6		10.8	16.3		7.7	20.3	
LOS	С	С		С	С		В	В		А	С	
Approach Delay		28.4			32.4			15.4			19.6	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)	53	27		2	9		24	168		5	123	
Queue Length 95th (ft)	117	84		12	37		81	331		24	248	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	370	511		338	447		462	3095		443	2956	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.40	0.15		0.02	0.06		0.45	0.36		0.11	0.28	
Intersection Summary												
	Other											
Cycle Length: 120												
Actuated Cycle Length: 65.8												
Natural Cycle: 80												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.66												
Intersection Signal Delay: 18					tersection							
Intersection Capacity Utilizat	ion 60.2%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Ø1	™ ¶ ø2	√ Ø3	ø₄
16 s	66 s	14 s	24 s
▲ Ø5	▶ø6	▶ _{Ø7}	₩ Ø8
16 s	66 s	16 s	22 s

	→	\mathbf{r}	4	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			र्स	Y		
Traffic Volume (veh/h)	38	52	15	47	48	5	
Future Volume (Veh/h)	38	52	15	47	48	5	
Sign Control	Free			Free	Stop		
Grade	2%			-1%	5%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	42	58	17	52	53	6	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			100		157	71	
vC1, stage 1 conf vol			100		101		
vC2, stage 2 conf vol							
vCu, unblocked vol			100		157	71	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)					2.1	5.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		94	99	
cM capacity (veh/h)			1493		824	991	
Direction, Lane #	EB 1	WB 1	NB 1		•= ·	•••	
Volume Total	100	69	59				
Volume Left	0	17	53				
Volume Right	58	0	6				
cSH	1700	1493	839				
Volume to Capacity	0.06	0.01	0.07				
Queue Length 95th (ft)	0.00	1	6				
Control Delay (s)	0.0	1.9	9.6				
Lane LOS	0.0	1.9 A	9.0 A				
Approach Delay (s)	0.0	1.9	9.6				
Approach LOS	0.0	1.5	9.0 A				
			~				
Intersection Summary							
Average Delay			3.1				
Intersection Capacity Utiliza	ation		20.0%	IC	U Level o	of Service)
Analysis Period (min)			15				

Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

	٦	\mathbf{F}	•	Ť	Ļ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	1 2	
Traffic Volume (veh/h)	2	30	112	281	124	9
Future Volume (Veh/h)	2	30	112	281	124	9
Sign Control	Stop		=	Free	Free	•
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	2	33	124	312	138	10
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				Tionio	Tiono	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	703	143	148			
vC1, stage 1 conf vol	100	110	110			
vC2, stage 2 conf vol						
vCu, unblocked vol	703	143	148			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	96	91			
cM capacity (veh/h)	369	905	1434			
,						
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	35	436	148			
Volume Left	2	124	0			
Volume Right	33	0	10			
cSH	835	1434	1700			
Volume to Capacity	0.04	0.09	0.09			
Queue Length 95th (ft)	3	7	0			
Control Delay (s)	9.5	2.8	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.5	2.8	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utiliz	ation		41.4%	IC	CU Level c	of Service
Analysis Period (min)	-		15			
			10			

966: N Polk Street/I	Pinevill	e Road	d & Inc	lustrial	Drive/	Drivev	vay				11/()3/2017
	۶	-	$\mathbf{\hat{v}}$	4	←	×	1	Ť	۲	1	ŧ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	4		<u>۲</u>	4î		<u>۲</u>	≜ ⊅		<u>۲</u>	≜ ⊅	
Traffic Volume (vph)	265	5	166	10	1	46	63	875	11	8	1283	158
Future Volume (vph)	265	5	166	10	1	46	63	875	11	8	1283	158
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.855			0.853			0.998			0.984	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1593	0	1770	1589	0	1770	3532	0	1770	3483	0
Flt Permitted	0.488			0.638			0.070			0.242		
Satd. Flow (perm)	909	1593	0	1188	1589	0	130	3532	0	451	3483	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	294	6	184	11	1	51	70	972	12	9	1426	176
Shared Lane Traffic (%)		-			-			••=		-		
Lane Group Flow (vph)	294	190	0	11	52	0	70	984	0	9	1602	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	J -		12	J -		12	J -		12	J -
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane											Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	pm+pt	NA	-	pm+pt	NA	-	pm+pt	NA		pm+pt	NA	-
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	
All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	23.1	21.3		15.7	10.6		59.5	57.7		55.7	49.7	
	20.1	21.0		10.1	10.0		00.0	01.1		00.1	10.1	

2024 Background PM Peak Hour Timmons Group

Pineville Industrial TIA

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966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	-	\mathbf{r}	4	-	•	•	Ť	1	1	Ļ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Actuated g/C Ratio	0.24	0.23		0.17	0.11		0.63	0.61		0.59	0.53	
v/c Ratio	0.88	0.53		0.04	0.29		0.28	0.46		0.02	0.87	
Control Delay	64.5	44.3		32.9	50.6		10.3	11.4		7.0	27.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	64.5	44.3		32.9	50.6		10.3	11.4		7.0	27.5	
LOS	Е	D		С	D		В	В		А	С	
Approach Delay		56.5			47.5			11.3			27.4	
Approach LOS		Е			D			В			С	
Queue Length 50th (ft)	175	107		6	33		15	143		2	458	
Queue Length 95th (ft)	#279	#248		21	76		38	300		8	674	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	335	415		282	321		296	2490		448	2304	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.88	0.46		0.04	0.16		0.24	0.40		0.02	0.70	
Intersection Summary												
	Other											
Cycle Length: 120												
Actuated Cycle Length: 94.4												
Natural Cycle: 90												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.88												
Intersection Signal Delay: 26					tersectior							
Intersection Capacity Utilization	tion 80.2%			IC	U Level o	of Service	D					
Analysis Period (min) 15												
# 95th percentile volume e Queue shown is maximul			eue may l	be longer.								

Ø1	1 Ø2	√ Ø3	_ ▲ _{Ø4}
16 s	66 s	14 s	24 s
* ø5	▶ Ø6		★ ø8
16 s	66 s	16 s	22 s

Item 7.	
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Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

	٦	\mathbf{r}	1	Ť	Ļ	~
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	4	
Traffic Volume (veh/h)	10	112	37	112	403	4
Future Volume (Veh/h)	10	112	37	112	403	4
Sign Control	Stop		-	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	124	41	124	448	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	656	450	452			
vC1, stage 1 conf vol	000	100	102			
vC2, stage 2 conf vol						
vCu, unblocked vol	656	450	452			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	80	96			
cM capacity (veh/h)	414	609	1109			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	135	165	452			
Volume Left	11	41	0			
Volume Right	124	0	4			
cSH	587	1109	1700			
Volume to Capacity	0.23	0.04	0.27			
Queue Length 95th (ft)	22	3	0			
Control Delay (s)	13.0	2.3	0.0			
Lane LOS	В	А				
Approach Delay (s)	13.0	2.3	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utilizati	tion		46.9%	IC	CU Level o	f Service
Analysis Period (min)			15			

2019 Phase I Build Traffic Volumes

966: N Polk Street/	Pinevill	e Road	d & Inc	lustrial	Drive/	Drivev	vay				11/0)3/2017
	٦	-	\rightarrow	4	←	•	•	1	1	>	Ļ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ef 👘		<u>۲</u>	4		ሻ	≜ ⊅		۳	≜ ⊅	
Traffic Volume (vph)	127	0	67	4	2	19	175	893	15	40	470	221
Future Volume (vph)	127	0	67	4	2	19	175	893	15	40	470	221
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.850			0.863			0.997			0.952	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1583	0	1770	1608	0	1770	3529	0	1770	3369	0
Flt Permitted	0.465						0.215			0.232		
Satd. Flow (perm)	866	1583	0	1863	1608	0	400	3529	0	432	3369	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	141	0	74	4	2	21	194	992	17	44	522	246
Shared Lane Traffic (%)		•			_							
Lane Group Flow (vph)	141	74	0	4	23	0	194	1009	0	44	768	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane											Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	pm+pt	NA	-	pm+pt	NA	-	pm+pt	NA	-	pm+pt	NA	-
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	
All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	14.3	12.8		10.9	9.1		36.1	33.0		31.3	21.7	
	11.0	12.0		10.0	V.1		00.1	00.0		01.0	£	

2019 Build AM Peak Hour Timmons Group

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Pineville Industrial TIA 066: N Polk Street/Pineville Peed & Industrial Drive/Driveway

966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	-	$\mathbf{\hat{v}}$	∢	-	•	1	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Actuated g/C Ratio	0.24	0.22		0.19	0.16		0.61	0.56		0.53	0.37	
v/c Ratio	0.38	0.22		0.01	0.09		0.40	0.51		0.10	0.62	
Control Delay	24.1	26.3		20.8	32.0		9.4	15.2		7.5	19.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	24.1	26.3		20.8	32.0		9.4	15.2		7.5	19.2	
LOS	С	С		С	С		А	В		А	В	
Approach Delay		24.8			30.3			14.3			18.5	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)	41	21		1	7		22	142		5	102	
Queue Length 95th (ft)	104	76		9	34		76	286		22	220	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	434	606		388	533		543	3188		539	3043	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.32	0.12		0.01	0.04		0.36	0.32		0.08	0.25	
Intersection Summary												
71	Other											
Cycle Length: 120												
Actuated Cycle Length: 58.7												
Natural Cycle: 75												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.62												
Intersection Signal Delay: 17					tersectior							
Intersection Capacity Utilizat	ion 57.2%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Ø1	™ ø2	√ Ø3	ø₄
16 s	66 s	14 s	24 s
* ø5	↓ ø ₆	∕ ≯ _{Ø7}	₩ Ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

Item	7.

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<u>بور</u> ۴			<u>احد،</u>	Y		
Traffic Volume (veh/h)	34	48	14	43	45	5	
Future Volume (Veh/h)	34	48	14	43	45	5	
Sign Control	Free			Free	Stop	•	
Grade	2%			-1%	5%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	38	53	16	48	50	6	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			91		144	64	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			91		144	64	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		94	99	
cM capacity (veh/h)			1504		839	1000	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	91	64	56				
Volume Left	0	16	50				
Volume Right	53	0	6				
cSH	1700	1504	853				
Volume to Capacity	0.05	0.01	0.07				
Queue Length 95th (ft)	0	1	5				
Control Delay (s)	0.0	1.9	9.5				
Lane LOS		А	А				
Approach Delay (s)	0.0	1.9	9.5				
Approach LOS			А				
Intersection Summary							
Average Delay			3.1				
Intersection Capacity Utiliza	ation		19.7%	IC	U Level c	of Service	;
Analysis Period (min)			15				

Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

11/03/2017

	٦	\mathbf{F}	1	1	Ļ	~
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			स	f,	
Traffic Volume (veh/h)	2	30	112	255	112	9
Future Volume (Veh/h)	2	30	112	255	112	9
Sign Control	Stop			Free	Free	Ŭ
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	2	33	124	283	124	10
Pedestrians	2	55	124	205	124	10
Lane Width (ft)						
()						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)				Maria	Maria	
Median type				None	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	660	129	134			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	660	129	134			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	96	91			
cM capacity (veh/h)	391	921	1451			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	35	407	134			
Volume Left	2	124	0			
Volume Right	33	0	10			
cSH	855	1451	1700			
Volume to Capacity	0.04	0.09	0.08			
Queue Length 95th (ft)	3	7	0			
Control Delay (s)	9.4	2.9	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.4	2.9	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utiliz	zation		39.4%	IC	CU Level o	of Service
Analysis Period (min)			15			
			10			

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966: N Polk Street/I	Pinevill	e Road	l & Inc	lustrial	Drive/	Drivev	vay				11/0)3/2017
	٦	→	\mathbf{r}	4	+	×	1	Ť	1	1	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	4Î		۲	eî.		۲	≜ î≽		٦	<u></u> ↑1≽	
Traffic Volume (vph)	247	4	157	9	1	42	59	793	10	7	1162	150
Future Volume (vph)	247	4	157	9	1	42	59	793	10	7	1162	150
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.853			0.853			0.998			0.983	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1589	0	1770	1589	0	1770	3532	0	1770	3479	0
Flt Permitted	0.435			0.645			0.079			0.280		-
Satd. Flow (perm)	810	1589	0	1201	1589	0	147	3532	0	522	3479	0
Right Turn on Red			No			No			No	•	0110	No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	274	4	174	10	0.50	47	66	881	11	8	1291	167
Shared Lane Traffic (%)	217	т	174	10		77	00	001		0	1231	107
Lane Group Flow (vph)	274	178	0	10	48	0	66	892	0	8	1458	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Leit	12	Night	Len	12	Night	Leit	12	Night	Len	12	rugin
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		10			10			10			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	9	1.00	1.00	9	1.00	1.00	1.00	1.00	1.00	9
Turn Type		NA	9	pm+pt	NA	9		NA	9		NA	9
Protected Phases	pm+pt 7	4		рш+рі 3	8		pm+pt 5	2		pm+pt 1	6	
Permitted Phases	1	4		8	0		2	2		6	0	
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase	1	4		5	0		J	2		1	0	
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
.,	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	66.0		14.0	66.0	
Total Split (s)	13.3%	24.0		14.0	22.0 18.3%		13.3%	55.0%		13.3%	55.0%	
Total Split (%)										13.3% 9.7		
Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7			59.7	
Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	_
All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	20.6	19.1		13.9	10.4		53.1	51.5		49.4	43.3	

2019 Build PM Peak Hour Timmons Group

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Pineville Industrial TIA atrial Driv

966: N Polk Street/	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	→	\mathbf{r}	4	+	•	1	Ť	1	1	Ļ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Actuated g/C Ratio	0.24	0.22		0.16	0.12		0.62	0.60		0.58	0.51	
v/c Ratio	0.81	0.50		0.04	0.25		0.24	0.42		0.02	0.83	
Control Delay	53.3	41.5		30.7	46.2		8.9	10.7		7.1	24.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	53.3	41.5		30.7	46.2		8.9	10.7		7.1	24.3	
LOS	D	D		С	D		А	В		А	С	
Approach Delay		48.6			43.5			10.5			24.3	
Approach LOS		D			D			В			С	
Queue Length 50th (ft)	140	87		4	27		13	123		2	380	
Queue Length 95th (ft)	#267	#224		20	72		33	265		8	571	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	340	441		288	360		328	2588		495	2501	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.81	0.40		0.03	0.13		0.20	0.34		0.02	0.58	
Intersection Summary												
51	Other											
Cycle Length: 120												
Actuated Cycle Length: 85.7												
Natural Cycle: 90												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 0.83												
Intersection Signal Delay: 23					tersectior							
Intersection Capacity Utilization	tion 75.6%			IC	U Level o	of Service	D					
Analysis Period (min) 15												
# 95th percentile volume e Queue shown is maximu			eue may l	be longer.								

Ø1	1 Ø2	√ Ø3	ø₄
16 s	66 s	14 s	24 s
▲ Ø5			₩ Ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

Item 7	7
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	-	\mathbf{F}	∢	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>بر ا</u>			<u>्</u>	Y	
Traffic Volume (veh/h)	75	45	5	83	51	15
Future Volume (Veh/h)	75	45	5	83	51	15
Sign Control	Free		-	Free	Stop	
Grade	2%			-1%	5%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	83	50	6	92	57	17
Pedestrians			-		••	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	Homo			1 tonio		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			133		212	108
vC1, stage 1 conf vol			100		212	100
vC2, stage 2 conf vol						
vCu, unblocked vol			133		212	108
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)					0.1	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			100		93	98
cM capacity (veh/h)			1452		773	946
Direction, Lane #	EB 1	WB 1	NB 1			• • •
Volume Total	133	98	74			
Volume Left	0	6	57			
Volume Right	50	0	17			
cSH	1700	1452	807			
Volume to Capacity	0.08	0.00	0.09			
Queue Length 95th (ft)	0.00	0.00	0.03			
Control Delay (s)	0.0	0.5	9.9			
Lane LOS	0.0	0.5 A	9.9 A			
	0.0	0.5	9.9			
Approach Delay (s)	0.0	0.5	9.9 A			
Approach LOS			A			
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utiliza	ation		18.9%	IC	U Level c	of Service
Analysis Period (min)			15			

Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

11/03/2017

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	ţ,	
Traffic Volume (veh/h)	10	112	37	102	366	4
Future Volume (Veh/h)	10	112	37	102	366	4
Sign Control	Stop		•.	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	124	41	113	407	4
Pedestrians		127	- 1	110	101	T
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
				None	None	
Median type				NONE	None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked	004	400	444			
vC, conflicting volume	604	409	411			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	00.4	(00				
vCu, unblocked vol	604	409	411			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	81	96			
cM capacity (veh/h)	445	642	1148			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	135	154	411			
Volume Left	11	41	0			
Volume Right	124	0	4			
cSH	620	1148	1700			
Volume to Capacity	0.22	0.04	0.24			
Queue Length 95th (ft)	21	3	0			
Control Delay (s)	12.4	2.4	0.0			
Lane LOS	В	А				
Approach Delay (s)	12.4	2.4	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			2.9			
Intersection Capacity Utiliza	ation		44.4%	IC	U Level c	of Service
Analysis Period (min)			15			
			10			

Synchro 9 Report Page 1

2024 Phase II Build Traffic Volumes

966: N Polk Street/I	Pinevill	e Road	d & Inc	lustrial	Drive/	Drivev	vay				11/0)3/2017
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	€.		<u>۲</u>	₽		٦	≜ ⊅		<u>۲</u>	≜ ⊅⊱	
Traffic Volume (vph)	156	0	83	5	2	21	338	986	16	44	519	340
Future Volume (vph)	156	0	83	5	2	21	338	986	16	44	519	340
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	0		75	100		0	165		0
Storage Lanes	1		0	1		1	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.850			0.862			0.998			0.941	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1583	0	1770	1606	0	1770	3532	0	1770	3330	0
Flt Permitted	0.435						0.141			0.191		
Satd. Flow (perm)	810	1583	0	1863	1606	0	263	3532	0	356	3330	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		1961			266			1652			1043	
Travel Time (s)		38.2			5.2			32.2			20.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	173	0	92	6	2	23	376	1096	18	49	577	378
Shared Lane Traffic (%)		-		-	_							
Lane Group Flow (vph)	173	92	0	6	25	0	376	1114	0	49	955	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	J -		12	J -		12	J -		12	J
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane											Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Minimum Split (s)	14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Total Split (s)	16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Total Split (%)	13.3%	20.0%		11.7%	18.3%		13.3%	55.0%		13.3%	55.0%	
Maximum Green (s)	10.2	18.2		8.6	16.2		10.4	59.7		9.7	59.7	
Yellow Time (s)	3.0	3.8		3.0	3.8		3.0	3.8		3.0	3.8	
All-Red Time (s)	2.8	2.0		2.4	2.0		2.6	2.5		3.3	2.5	
Lost Time Adjust (s)	-0.8	-0.8		-0.4	-0.8		-0.6	-1.3		-1.3	-1.3	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	3.0		2.0	3.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Act Effct Green (s)	14.8	13.2		10.8	8.7		42.2	36.3		35.3	26.6	

2024 Build AM Peak Hour Timmons Group Synchro 9 Report Page 1

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Pineville Industrial TIA
966: N Polk Street/Pineville Road & Industrial Drive/Driveway

966: N Polk Street/I	Pineville	e Road	& Ind	ustrial	Drive/	Drivew	ay				11/0	3/2017
	٦	-	\mathbf{r}	∢	-	•	1	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Actuated g/C Ratio	0.21	0.19		0.16	0.13		0.61	0.53		0.51	0.39	
v/c Ratio	0.54	0.30		0.02	0.12		0.91	0.60		0.14	0.75	
Control Delay	30.9	30.3		23.2	34.9		46.7	16.0		7.8	22.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	30.9	30.3		23.2	34.9		46.7	16.0		7.8	22.8	
LOS	С	С		С	С		D	В		А	С	
Approach Delay		30.7			32.6			23.8			22.1	
Approach LOS		С			С			С			С	
Queue Length 50th (ft)	64	33		2	9		88	168		5	151	
Queue Length 95th (ft)	136	96		12	37		#363	338		25	301	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	348	478		324	416		414	3073		434	2897	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.50	0.19		0.02	0.06		0.91	0.36		0.11	0.33	
Intersection Summary												
· · · / · ·	Other											
Cycle Length: 120												
Actuated Cycle Length: 69												
Natural Cycle: 90												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.91												
Intersection Signal Delay: 23					tersectior							
Intersection Capacity Utilizat	ion 71.8%			IC	U Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume e Queue shown is maximur			eue may l	be longer.								

Ø1	1 Ø2	√ Ø3	ø₄
16 s	66 s	14 s	24 s
▲ Ø5			₩ Ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

	→	\mathbf{r}	∢	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4			<u>्</u>	Y	
Traffic Volume (veh/h)	38	63	23	47	50	6
Future Volume (Veh/h)	38	63	23	47	50	6
Sign Control	Free			Free	Stop	-
Grade	2%			-1%	5%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	42	70	26	52	56	7
Pedestrians			•	•=		•
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)				110110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			112		181	77
vC1, stage 1 conf vol			112		101	
vC2, stage 2 conf vol						
vCu, unblocked vol			112		181	77
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			98		93	99
cM capacity (veh/h)			1478		794	984
					7 94	904
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	112	78	63			
Volume Left	0	26	56			
Volume Right	70	0	7			
cSH	1700	1478	811			
Volume to Capacity	0.07	0.02	0.08			
Queue Length 95th (ft)	0	1	6			
Control Delay (s)	0.0	2.6	9.8			
Lane LOS		А	А			
Approach Delay (s)	0.0	2.6	9.8			
Approach LOS			А			
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utiliz	zation		20.4%	IC		of Service
Analysis Period (min)			20.4 %	10	O Level C	
Analysis Fellou (IIIIII)			15			

Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

11/03/2017

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	۲			र्स	¢.	
Traffic Volume (veh/h)	5	64	367	281	124	28
Future Volume (Veh/h)	5	64	367	281	124	28
Sign Control	Stop	• ·		Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	71	408	312	138	31
Pedestrians			100	0.12	100	01
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None	None	
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	1282	154	169			
vC1, stage 1 conf vol	1202	134	109			
vC2, stage 2 conf vol						
vCu, unblocked vol	1282	154	169			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	4.1			
	3.5	3.3	2.2			
tF (s)	3.5 95	3.3 92	Z.Z 71			
p0 queue free %						
cM capacity (veh/h)	130	892	1409			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	77	720	169			
Volume Left	6	408	0			
Volume Right	71	0	31			
cSH	612	1409	1700			
Volume to Capacity	0.13	0.29	0.10			
Queue Length 95th (ft)	11	30	0			
Control Delay (s)	11.7	6.2	0.0			
Lane LOS	В	А				
Approach Delay (s)	11.7	6.2	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			5.5			
Intersection Capacity Utiliz	zation		57.6%	IC	CU Level o	of Service
Analysis Period (min)			15			
			10			

Synchro 9 Report Page 1

Lane Group EBL EBT EBR WBL WBT WBR NBI NBT NBT SBL SBT SBR Lane Configurations 1 1 1 46 77 875 11 8 1233 180 Future Volume (vph) 383 5 312 10 1 46 77 875 11 8 1233 180 Storage Length (r) 1500 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 1000 111 1 1 0 1177 352 0 177 352 0 100 100 100 100 100 100 100 100 100	966: N Polk Street/	Pinevill	e Road	d & Inc	lustrial	Drive/	Drivev	vay				11/()3/2017
Lane Configurations T P T P T P T P		٦	-	\rightarrow	4	+	•	•	1	1	1	Ŧ	-
Traffic Volume (vph) 383 5 312 10 1 46 77 875 11 8 1283 180 Future Volume (vph) 380 5 312 10 1 46 77 875 11 8 1283 180 Gideal Flow (vph) 1900 190 190 190 </th <th>Lane Group</th> <th>EBL</th> <th>EBT</th> <th>EBR</th> <th>WBL</th> <th>WBT</th> <th>WBR</th> <th>NBL</th> <th></th> <th>NBR</th> <th>SBL</th> <th></th> <th>SBR</th>	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL		NBR	SBL		SBR
Future (vph) 383 5 312 10 1 46 77 875 11 8 1283 180 ideal Flow (vphp) 1900 19	Lane Configurations	<u>۲</u>	4î		ሻ	4Î		٦	≜ ⊅		ሻ	≜ ⊅	
ideal Flow (vph) 1900	Traffic Volume (vph)	383	5	312	10	1	46	77	875	11	8	1283	180
Storage Langth (ft) 150 0 0 75 100 0 165 0 Storage Lanes 1 0 1 1 1 0 0	Future Volume (vph)	383	5	312	10	1	46	77	875	11	8	1283	180
Storage Lance 1 0 1 1 1 1 0 1 0 Taper Length (ft) 25 26 170 3476 0 1770 3476 0 1770 3476 0 1770 3476 0 1770 3476 0 No	Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Taper Length (th) 25 25 25 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 0.950 0.90	Storage Length (ft)	150		0	0		75	100		0	165		0
Lame Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 1.00 0.95 0.95 Frt 0.853 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.950 0.942 Stat. Flow (prot) 1770 1589 0 1770 1589 0 1770 3532 0 1770 3476 0 Right Turn on Red No	Storage Lanes	1		0	1		1	1		0	1		0
Frt 0.853 0.853 0.998 0.990 990 FIP Orlected 0.950 0.952 0.921 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.924 0.90		25			25			25			25		
Fit Protected 0.950 0.950 0.950 0.950 0.950 Satd. Flow (prot) 1770 1589 0 1770 3532 0 1770 3476 0 Righ Turn on Red No No No No No No No No Satd. Flow (prot) 920 1589 0 745 1589 0 125 3532 0 451 3476 0 Righ Turn on Red No	Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Satd. Flow (prot) 1770 1589 0 1770 1589 0 1770 3532 0 1770 3476 0 FI Permitted 0.494 0.400 0.067 0.242 0 451 347 0 125 3532 0 451 3476 0 Right Turn on Red No <	Frt		0.853			0.853			0.998			0.982	
Fit Permitted 0.494 0.400 0.067 0.242 Satd. Flow (perm) 920 1589 0 745 1589 0 125 352 0 451 3476 0 Right Turn on Red No No No No No No No Link Distance (th) 1961 266 1652 1043 1143 17avel Time (s) 38.2 5.2 32.2 20.3 20.3 Peak Hour Factor 0.90	Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (perm) 920 1589 0 745 1589 0 125 3532 0 451 3476 0 Right Turn on Red No No No No No No No Satd. Flow (RTOR) 1961 266 1652 1043 Travel Time (s) 352 52 232.2 20.3 Peak Hour Factor 0.90 <t< td=""><td>Satd. Flow (prot)</td><td>1770</td><td>1589</td><td>0</td><td>1770</td><td>1589</td><td>0</td><td>1770</td><td>3532</td><td>0</td><td>1770</td><td>3476</td><td>0</td></t<>	Satd. Flow (prot)	1770	1589	0	1770	1589	0	1770	3532	0	1770	3476	0
Right Turn on Red No No No No No No No Satic Flow (RTOR) 111 1961 266 1652 1043 1043 Link Speed (mph) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 1.40 1.00	Flt Permitted	0.494			0.400			0.067			0.242		
Right Turn on Red No No No No No No Satd. Flow (RTOR) 111 266 1652 1043 1043 Link Speed (mph) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 1.00 1.00	Satd. Flow (perm)	920	1589	0	745	1589	0		3532	0	451	3476	0
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90				No			No			No			No
Link Speed (mph) 35 35 35 35 35 Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90	Satd. Flow (RTOR)												
Link Distance (ft) 1961 266 1652 1043 Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90 D.90			35			35			35			35	
Travel Time (s) 38.2 5.2 32.2 20.3 Peak Hour Factor 0.90			1961									1043	
Peak Hour Factor 0.90	()		38.2			5.2			32.2			20.3	
Adj, Flow (vph) 426 6 347 11 1 51 86 972 12 9 1426 200 Shared Lane Traffic (%) Lane Group Flow (vph) 426 353 0 11 52 0 86 984 0 9 1626 0 Lane Group Flow (vph) 426 353 0 11 52 0 86 984 0 9 1626 0 Lane Alignment Left Left Right Left Left Right Left Right </td <td>· · · · · · · · · · · · · · · · · · ·</td> <td>0.90</td> <td></td> <td>0.90</td> <td>0.90</td> <td></td> <td>0.90</td> <td>0.90</td> <td></td> <td>0.90</td> <td>0.90</td> <td></td> <td>0.90</td>	· · · · · · · · · · · · · · · · · · ·	0.90		0.90	0.90		0.90	0.90		0.90	0.90		0.90
Shared Lane Traffic (%) Lane Group Flow (vph) 426 353 0 11 52 0 86 984 0 9 1626 0 Enter Blocked Intersection No													
Lane Group Flow (vph) 426 353 0 11 52 0 86 984 0 9 1626 0 Enter Blocked Intersection No N	, , ,		-	-					-		-		
Enter Blocked Intersection No No <th< td=""><td>()</td><td>426</td><td>353</td><td>0</td><td>11</td><td>52</td><td>0</td><td>86</td><td>984</td><td>0</td><td>9</td><td>1626</td><td>0</td></th<>	()	426	353	0	11	52	0	86	984	0	9	1626	0
Lane Alignment Left Left Right	,			No						No	No		
Median Width(ft) 12 13 13 13 13 13 13 13 13 13 13 13 13 13 <th13< th=""> 13 13</th13<>													
Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane Yes Yes Yes Yes Headway Factor 1.00<	-			5			Ū			0			0
Crosswalk Width(ft) 16 16 16 16 16 Two way Left Turn Lane Yes Headway Factor 1.00 Protected Phases 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 66.0 66.0 66.0 10.0 7.0 7.0 7.0 7.0 10.0 7.0 10.0 </td <td>. ,</td> <td></td> <td>0</td> <td></td>	. ,		0										
Two way Left Turn Lane Yes Headway Factor 1.00 Protected Phases 7 4 3 8 5 2 1 6 1.00			16			16			16			16	
Headway Factor 1.00<												Yes	
Turning Speed (mph) 15 9 15 15 16		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turn Type pm+pt NA pm+pt NA pm+pt NA pm+pt NA Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 6 Detector Phase 7 4 3 8 5 2 1 6 Switch Phase 7 4 3 8 5 2 1 6 Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (s) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 3.0 3.8 3.0 3.8 3.0 3.8	,	15		9	15		9	15		9	15		9
Protected Phases 7 4 3 8 5 2 1 6 Permitted Phases 4 8 2 6 10.0 7.0 7.0 7.0 7.0 7.0 10.0 7.0 10.0 7.0 10.0 7.0 10.0 14.0 21.0 14.0 21.0 14.0 21.0 14.0 21.0 10.1 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% 13.3% 55.0% 13.3% 55.0% 13.3% 55.0% 14.0 21.0 2.6 2.5 3.3 2.5 </td <td></td> <td>pm+pt</td> <td>NA</td> <td></td> <td>pm+pt</td> <td>NA</td> <td></td> <td>pm+pt</td> <td>NA</td> <td></td> <td>pm+pt</td> <td>NA</td> <td></td>		pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Initial (s) 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 5.0 <td></td>													
Detector Phase 7 4 3 8 5 2 1 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Initial (s) 7.0 7.0 7.0 10.0 7.0 10.0 Minimum Split (s) 14.0 20.0 14.0 20.0 14.0 21.0 Total Split (s) 16.0 24.0 14.0 22.0 16.0 66.0 16.0 66.0 Total Split (%) 13.3% 20.0% 11.7% 18.3% 13.3% 55.0% 13.3% 55.0% Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 3.0 5.0 <td>Permitted Phases</td> <td>4</td> <td></td> <td></td> <td>8</td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td> <td></td> <td></td>	Permitted Phases	4			8			2			6		
Minimum Initial (s)7.07.07.07.07.010.07.010.0Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Detector Phase	7	4			8		5	2		1	6	
Minimum Split (s)14.020.014.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.03.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Switch Phase												
Minimum Split (s)14.020.014.021.014.021.0Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.03.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	Minimum Initial (s)	7.0	7.0		7.0	7.0		7.0	10.0		7.0	10.0	
Total Split (s)16.024.014.022.016.066.016.066.0Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin		14.0	20.0		14.0	20.0		14.0	21.0		14.0	21.0	
Total Split (%)13.3%20.0%11.7%18.3%13.3%55.0%13.3%55.0%Maximum Green (s)10.218.28.616.210.459.79.759.7Yellow Time (s)3.03.83.03.83.03.83.03.83.03.8All-Red Time (s)2.82.02.42.02.62.53.32.5Lost Time Adjust (s)-0.8-0.8-0.4-0.8-0.6-1.3-1.3-1.3Total Lost Time (s)5.05.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesYesVehicle Extension (s)2.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneMinNoneMin		16.0	24.0		14.0	22.0		16.0	66.0		16.0	66.0	
Maximum Green (s) 10.2 18.2 8.6 16.2 10.4 59.7 9.7 59.7 Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0								13.3%					
Yellow Time (s) 3.0 3.8 3.0 3.8 3.0 3.8 3.0 3.8 All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0													
All-Red Time (s) 2.8 2.0 2.4 2.0 2.6 2.5 3.3 2.5 Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None Min None Min								3.0			3.0		
Lost Time Adjust (s) -0.8 -0.8 -0.4 -0.8 -0.6 -1.3 -1.3 -1.3 Total Lost Time (s) 5.0													
Total Lost Time (s)5.05.05.05.05.05.05.0Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin	()												
Lead/LagLeadLagLeadLagLeadLagLead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin													
Lead-Lag Optimize?YesYesYesYesYesYesVehicle Extension (s)2.02.02.02.02.03.02.03.0Recall ModeNoneNoneNoneNoneNoneMinNoneMin													
Vehicle Extension (s) 2.0 2.0 2.0 2.0 2.0 3.0 2.0 3.0 Recall Mode None None None None None Min None Min													
Recall Mode None None None None Min None Min	• •												
	Act Effct Green (s)	25.4	23.5		15.4	10.2		62.0	60.1		57.6	51.5	

2024 Build PM Peak Hour Timmons Group

Pineville Industrial TIA

Synchro 9 Report Page 1

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966: N Polk Street/				uotinai	DIIVC	Direction						3/2017
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Actuated g/C Ratio	0.26	0.24		0.16	0.10		0.63	0.61		0.58	0.52	
v/c Ratio	1.18	0.93		0.06	0.32		0.36	0.46		0.02	0.89	
Control Delay	142.8	73.9		33.6	52.2		14.5	11.6		7.0	29.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	142.8	73.9		33.6	52.2		14.5	11.6		7.0	29.7	
LOS	F	Е		С	D		В	В		А	С	
Approach Delay		111.6			49.0			11.9			29.6	
Approach LOS		F			D			В			С	
Queue Length 50th (ft)	~390	238		6	34		18	143		2	483	
Queue Length 95th (ft)	#508	#546		21	76		57	300		8	#697	
Internal Link Dist (ft)		1881			186			1572			963	
Turn Bay Length (ft)	150						100			165		
Base Capacity (vph)	360	378		229	291		273	2500		429	2269	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	1.18	0.93		0.05	0.18		0.32	0.39		0.02	0.72	
Intersection Summary												
· · · / · ·	Other											
Cycle Length: 120												
Actuated Cycle Length: 98.6	6											
Natural Cycle: 110												
Control Type: Actuated-Unc	oordinated											
Maximum v/c Ratio: 1.18												
Intersection Signal Delay: 42					tersectior							
Intersection Capacity Utiliza	tion 87.4%			IC	U Level o	of Service	E					
Analysis Period (min) 15												
 Volume exceeds capacity 			ally infinit	e.								
Queue shown is maximu												
# 95th percentile volume e Queue shown is maximu			eue may l	be longer								

Ø1	™ [†] <i>ø</i> 2	√ Ø3	
16 s	66 s	14 s	24 s
▲ ø5	↓ Ø6	∕ ≯ _{Ø7}	₩ Ø8
16 s	66 s	16 s	22 s

Pineville Industrial TIA <u>1: Industrial Drive & Rodney Street</u>

Item	7

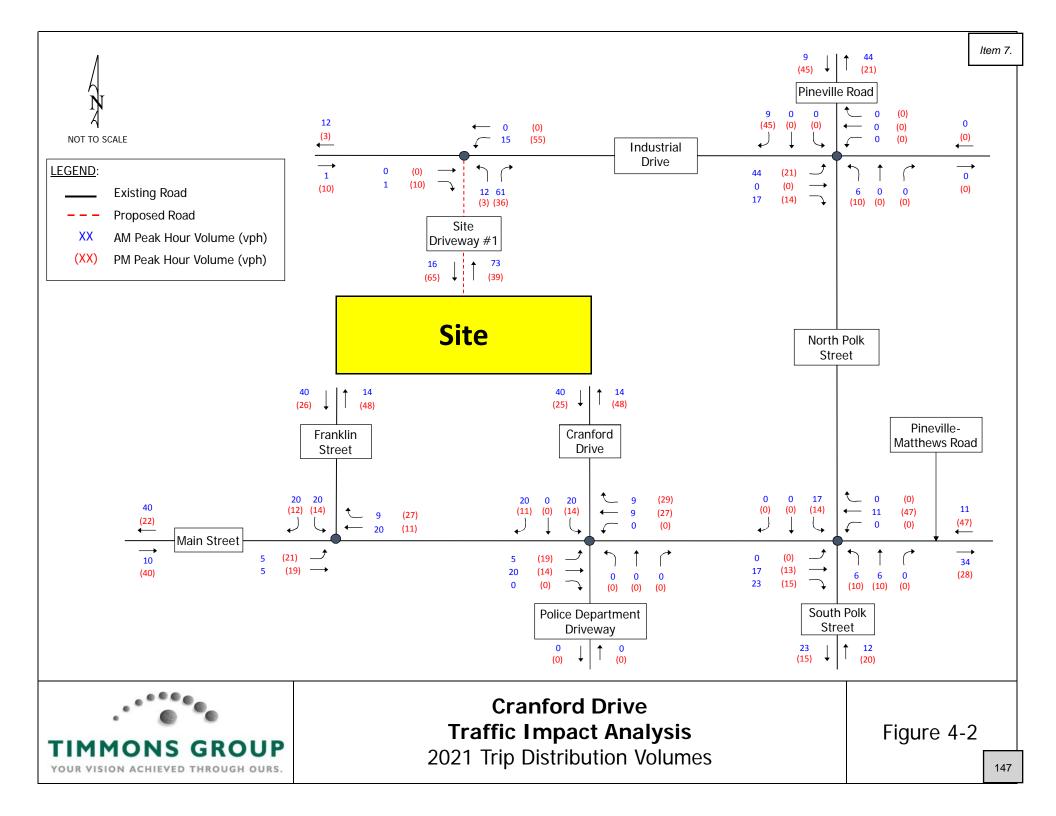
	→	$\mathbf{\hat{F}}$	¥	+	•	*	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	₽ P			ب ا	Y		
Traffic Volume (veh/h)	83	51	6	92	70	26	
Future Volume (Veh/h)	83	51	6	92	70	26	
Sign Control	Free			Free	Stop		
Grade	2%			-1%	5%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	92	57	7	102	78	29	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			149		236	120	
vC1, stage 1 conf vol			110		200	120	
vC2, stage 2 conf vol							
vCu, unblocked vol			149		236	120	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)			7.1		0.4	0.2	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		90	97	
cM capacity (veh/h)			1432		748	931	
					740	301	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	149	109	107				
Volume Left	0	7	78				
Volume Right	57	0	29				
cSH	1700	1432	790				
Volume to Capacity	0.09	0.00	0.14				
Queue Length 95th (ft)	0	0	12				
Control Delay (s)	0.0	0.5	10.3				
Lane LOS		А	В				
Approach Delay (s)	0.0	0.5	10.3				
Approach LOS			В				
Intersection Summary							
Average Delay 3.2							
Intersection Capacity Utilization			21.9%		U Level c	f Service	А
Analysis Period (min)			21.9 <i>%</i>	10			~
			10				

Pineville Industrial TIA 3: Industrial Drive & Site Driveway #1

11/03/2017

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	1	
Traffic Volume (veh/h)	35	376	73	112	403	7
Future Volume (Veh/h)	35	376	73	112	403	7
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	39	418	81	124	448	8
Pedestrians	00	110	01	121	110	Ŭ
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
				NUTE	NOLIG	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked	700	450	450			
vC, conflicting volume	738	452	456			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	700	450	450			
vCu, unblocked vol	738	452	456			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	89	31	93			
cM capacity (veh/h)	357	608	1105			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	457	205	456			
Volume Left	39	81	0			
Volume Right	418	0	8			
cSH	573	1105	1700			
Volume to Capacity	0.80	0.07	0.27			
Queue Length 95th (ft)	193	6	0			
Control Delay (s)	31.5	3.8	0.0			
Lane LOS	D	А				
Approach Delay (s)	31.5	3.8	0.0			
Approach LOS	D					
Intersection Summary						
Average Delay			13.6			
Intersection Capacity Utiliz	zation		66.7%	IC	CU Level c	of Service
Analysis Period (min)	Lation		15	IC.		
			15			

Appendix E – Approved Developments





MEETING DATE: 02/11/2025

Agenda Title/Category:	New Employee Handbook Pay Policies							
Staff Contact/Presenter:	Linda Gaddy							
Meets Strategic Initiative or Approved Plan:	YesNoIf yes,Xlist:							
Background:	New pay plan designs were approved based on the compensation study just conducted. Pay Policy revisions have been revised to match.							
Discussion:	Approval of the proposed policy changes in that were presented at the last workshop meeting.							
Fiscal impact:	Within budget, these changes do not increase costs, and one reduces costs							
Attachments:	 PowerPoint summary of policy changes and final classification and pay plans Employee Handbook page updated Grid of Compensation Policies 2025 Classification and Pay plans 							
Recommended Motion to be made by Council:	Approval of Employee Handbook pay policy changes and final pay plans							

COMPENSATION 2025 POLICY CHANGES & PLANS





COMPENSATION PLAN 3.5.2025



Pineville TOWN OF PINEVILLE'S EMPLOYEE HANDBOOK

PROMOTION EFFECTS ON SALARY RATE

Promotions: Employees shall receive a pay increase to recognize and compensate the employee for their knowledge, skills and ability and their increased responsibility and duties. With the exception of Sworn Officers and Firefighters, when there is opportunity for promotion, the pay increase is a minimum of 5% or the minimum of that pay grade, whichever is greater. The increase should be between five(5) and 10% for most promotions, based on a 5% increase per pay grade increase, and consistent with internal equity. All increases must be approved by Human Resources and Town Manager.

Promoted employees will complete a promotion introductory period of 6 months, but no pay increase is due at this time. They will be included in the next annual review date. For Sworn Officers, the employee's new anniversary date will be one year from the date of promotion. For Sworn Officers and Firefighters, the incumbent will move to the step in the new grade that aligns with the promotion. From Police Officer to Corporal is one step lower on the new grade. For Officer to Sergeant two steps lower on the new grade. Other moves follow a similar pattern dependent on how may grades they are moving up. If the increase is less than the minimum salary for the new position, the incumbent will move to Step 1 of the new grade. In no event, however, shall the new salary exceed the maximum rate of the new salary range.

Police Officer to Detective and Detective to Police Officer considered Lateral move. Move to same Step on new Grade.

Demotions: Demotion is the movement of an employee from one position to a position in a class assigned to a lower salary range. An employee being demoted, must show promise of performing satisfactorily in another position. When an employee is demoted to a position for which they are qualified for other than for disciplinary or performance-based reasons, the employee will be placed at a pay-rate within the new pay grade based on knowledge, skills and ability as related to the new position and equitable to others within the same classification. If the current salary is within the new range, the employee's salary may be retained at the previous rate if appropriate. If the demotion is the result of discipline or the demotion is from a position with supervisory or leadership responsibilities, the salary shall be decreased at minimum 5% or to the midpoint of the new range and no merit increase will be granted for twelve months. In no event, however, shall the new salary exceed the maximum rate of the new salary range.

Revised pay plan plans have been finalized and the policies and procedures surrounding compensation have been evaluated and revised to match the new pay structures.

The policy changes which were shared with you at the Work Session have been incorporated into the Employee Handbook policies on p. 13.

The major change was that the 6-month introductory period pay increases are no longer needed. That section was deleted. The next section on promotion was revised and updated.

PAY POLICY REVIEW

	CURRENT	NEW		
Pay premiums				
Premiums for certs	Police sworn: One step for Intermeditate Law Enforcement Certificate, two steps for Advanced (or only one additonal step if already received credit for Intermediate)	\$1250/yr Intermediate \$2500/yr Advanced La *if credit already give only \$1250 wil be add granted every year at a		
Premiums for educ	General: none Police sworn: Two steps for Associates Degree, two more or 4 total for bachleors degree	General: none Police sworn: 5% for 10% for Bachelors* *if credit already give 5% for a total of 10%		
Premiums for language	5% for general employees, 2 steps for Police Officers	5% premium for one l premium for Police O		
	CURRENT	NEW		
Promotion Practices	includes 6 month introductory period	-		
General	5% to 10% unless moving greater than 2 grades considered on an individual basis	5% to 10% unless mov grades considered on and consistent with ir shoud genreally be at grade movement.		
	Incumbent is promoted to a higher grade, they receive the equivalent of a 7.5%	Move to the new grad Officer to Corporal, 1 to Sergeant. Other me		

In your packet is a workbook of Incentive Pay, Hiring and Promotion practices outlining the old vs. the new recommended polices.

Some only apply to the Police Sworn officers who receive additional premium pay for certifications and degree on top of their base pay (their Step).

INCENTIVE PREMIUMS

	CURRENT	NEW	Grandfather in?	Handbook Policy
Pay premiums				
Premiums for certs	Certificate, two steps for Advanced (or only one additional step if already	\$1250/yr Intermediate Law Certificate \$2500/yr Advanced Law Certificate* *if credit already given for Intermediate, only \$1250 will be added for total \$2500 granted every year at annual review	Yes 14 people	Add: flat amount awarded once per year at annual evaluation
Premiums for educ	Police sworn: Two steps for Associates Degree, two more or 4 total for bachelors degree	General: none Police sworn: 5% for Associates 10% for Bachelors* *if credit already given for Associates, only 5% for a total of 10%	N/A	in each paycheck Add: Sworn officers will receive a premium for Associates or Bachelors degrees completed added to their Grade/Step pay rate.
Premiums for language		5% premium for one language only, 5% premium for Police Officers	N/A	in each paycheck

PROMOTION PRACTICES

	CURRENT	NEW	Grandfather?	Handbook Policy
Promotion Practices	includes 6 month introductory period			
General	5% to 10% unless moving greater than 2 grades considered on an individual basis	5% to 10% unless moving more than 2 grades considered on an individual basis, and consistent with internal equity ** should generally be at 5% increase for each grade movement.	N/A	NEW EMPLOYEE INTRODUCTORY PAY INCREASES p12 Delete NA. Add: Promotion evaluation at 6 mons w/out pay increase. Annual eval is next Aug
Police	Incumbent is promoted to a higher grade, they receive the equivalent of a 7.5% increase and placed into the corresponding step of the new grade. If the 7.5% increase is less than the minimum salary for the new position, the incumbent will move to Step 1 of the new grade. At the completion of the introductory period in the new position, he/she is eligible for another one step increase (for a total max increase of 10% at the conclusion of the introductory process). *Police Officer to Detective and Detective to Police Officer considered Lateral Move and will move to the corresponding step to keep the same pay.	Move to the new grade one step lower for Officer to Corporal, 2 steps lower for Officer to Sergeant. Other moves follow similar pattern dependent on how many grades they are moving up. If the increase is less than the minimum salary for the new position, the incumbent will move to Step 1 of the new grade. *Police Officer to Detective and Detective to Police Officer considered Lateral move. Move to same Step on new Grade. **A six-month introductory period will apply to promotions. A performance evaluation will be conducted at six months in the new position, but no pay increase is granted.	N/A	EFFECTS ON SALARY RATE p 12 Promotion Evaluation conducted at 6 months without pay increase. Annual eval date is one year from last annual review date with eligibility to move one step with acceptable performance review score.

PROMOTION PRACTICES

	CURRENT	NEW	Grandfather ?	Handbook Policy
Promotion Practices	includes 6 month introductory period			
Fi	At the completion of the introductory period in the new position, he/she is eligible for another one step increase (for a total max increase of 10% at the conclusion of the introductory process).	**A six-month introductory period will apply to promotions. A performance evaluation will be conducted at six months in the new position, but no pay increase is granted.	N/A	Annual evaluation will be conducted at 12 months with eligible for a one-step increase with acceptable evaluation score.
	process).			score.

HIRING PRACTICES

	CURRENT	NEW	Grandfather in?	Handbook Policy
Hiring Guidelines				
Genera	Approx 2.5% per year of prior directly relevant experience up to the mid-point	Approx 2.5% per year of prior directly relevant experience up to the mid-point. Above mid- point will need justification and prior approval by Town Manager	N/A	New: Add to hiring practices. Initial 6 month evaluation without a pay increase.
Police	Step 1 No experience &/or no Degree Step 3 Two (2) years of experience &/ or Associate Degree Step 5 Four (4) years of experience &/or Bachelor's Degree Five(5)+ years of experience evaluated on a case-by-case basis	Step 1 No sworn experience &/or no Degree Step 3 Two (2) years of experience Step 5 Four (4) years of experience Five(5)+ years of experience evaluated on a case-by-case basis. Possesses an education degree and/or Law Enforcement Certificate=additional premium pay.	N/A	Anniversary date for evals is 12 months from sworn in date. No eval at 6 months. All officers have an FTO period. Completing FTO will replace 6 month eval.
	none	Probationary Officer: (new) Less than one year sworn exper. completing initial Field Training usually for 4 to 6 months. During this time they are accompanied by and are receiving field training from experienced trainers in order to become an independent Police Officer.	N/A	Pay rate is 5% below Step 1 Police Officer. At completion of FTO will promote to Police Officer Step 1. Anniversary date for evals is 12 months from sworn in date. No eval at 6 months.

HIRING PRACTICES

	CURRENT	NEW	Grandfather in?	Handbook Policy
Hiring Guidelines				
Fire	Start at Step 1, Upon successful completion of a six (6) month introductory period, incumbents will be eligible to receive a one (1) step increase	Start at Step 1, Upon successful completion of a twelve 12) month introductory period, incumbents will be eligible to receive a one (1) step increase with acceptable performance review score. Consideration for above Step 1 may be given for many years of experience in a tight hiring market with approval of Town Manager.	N/A	At bottom of published Fire pay plan

PAY POLICY TRANSITION TO NEW PLANS

The policy revisions need to be approved.

New policies will take effect on March 5th along with the new pay plans.

PAY PLANS



					Effective Ma	rch 5, 2025					
Grade	Project Title	FLSA Status	Min	Mid	Max	Grade	Project Title	FLSA Status	Min	Mid	Max
14	Customer Service Representative	NE	\$43,285	\$49,778	\$56,271	20	Assistant Telecommunications Supervisor	NE	\$55,589	\$66,707	\$77,82
14	Maintenance Technician	NE	\$43,285	\$49,778	\$56,271	20	Community Outreach Specialist	NE	\$55,589	\$66,707	\$77,82
14	Park Maintenance Technician I	NE	\$43,285	\$49,778	\$56,271	20	Parks Maintenance Supervisor	NE	\$55,589	\$66,707	\$77,82
14	Storm Water Technician	NE	\$43,285	\$49,778	\$56,271	20	Public Works Supervisor	NE	\$55,589	\$66,707	\$77,82
14	911 Telecommunicator	NE	\$43,285	\$49,778	\$56,271	20	Senior Systems Technician/Assistant Supervisor	NE	\$55,589	\$66,707	\$77,82
14	Admin Assistant/Receptionist	NE	\$43,285	\$49,778	\$56,271						
						21	Building Maintenance Supervisor	NE	\$58,368	\$70,042	\$81,71
15	Administrative Assistant	NE	\$45,450	\$52,267	\$59,085	21	Crime Analyst	NE	\$58,368	\$70,042	\$81,71
15	Equipment Operator	NE	\$45,450	\$52,267	\$59,085	21	Records & Accreditation Manager	NE	\$58,368	\$70,042	\$81,71
15	Senior Customer Service Representative	NE	\$45,450	\$52,267	\$59,085						
15	Senior Storm Water Technician	NE	\$45,450	\$52,267	\$59,085	22	Community Relations & Communications Specialist	NE	\$61,287	\$73,544	\$85,80
15	Parks Maintenance Technician II	NE	\$45,450	\$52,267	\$59,085	22	Accountant	Exempt	\$61,287	\$73,544	\$85,8
15	911 Telecommunicator II	NE	\$45,450	\$52,267	\$59,085						
						23	Central Office Database Technician	NE	\$64,351	\$77,221	\$90,09
16	Accounting Technician II	NE	\$47,722	\$54,880	\$62,039	23	Network Database Technician	NE	\$64,351	\$77,221	\$90,09
16	Administrative Technician	NE	\$47,722	\$54,880	\$62,039	23	Systems Technician Supervisor	NE	\$64,351	\$77,221	\$90,09
16	Billing & Collections Coordinator	NE	\$47,722	\$54,880	\$62,039						
16	Property & Evidence Technician	NE	\$47,722	\$54,880	\$62,039	24	Telecommunications Supervisor 911	Exempt	\$67,568	\$81,082	\$94,5
16	Senior Parks Maintenance Technician	NE	\$47,722	\$54,880	\$62,039						
16	Building Maintenance Technician	NE	\$47,722	\$54,880	\$62,039	25	Town Clerk	Exempt	\$70,947	\$85,136	\$99,32
16	911 Telecommunicator III	NE	\$47,722	\$54,880	\$62,039			-			
						30	Human Resource Director	Exempt	\$94,593	\$118,241	\$141,8
17	Human Resource Assistant	NE	\$50,108	\$57,624	\$65,140	30	Parks & Recreation Director	Exempt	\$94,593	\$118,241	\$141,8
17	Fleet Manager	NE	\$50,108	\$57,624	\$65,140	30	Public Works Director	Exempt	\$94,593	\$118,241	\$141,8
17	Code Enforcement Officer	NE	\$50,108	\$57,624	\$65,140			-			
17	911 Senior Telecommunicator	NE	\$50,108	\$57,624	\$65,140	31	Finance Director	Exempt	\$102,160	\$127,700	\$153,2
						31	Planning Director	Exempt	\$102,160	\$127,700	\$153,2
18	Athletic Coordinator	NE	\$50,421	\$60,505	\$70,589	31	Telephone/Utility Director	Exempt	\$102,160	\$127,700	\$153,2
18	Programs/Events Coordinator	NE	\$50,421	\$60,505	\$70,589						
18	Special Events Coordinator	NE	\$50,421	\$60,505	\$70,589	33	Assistant Town Manager	Exempt	\$119,949	\$148,949	\$178,7
18	Systems Technician	NE	\$50,421	\$60,505	\$70,589						
					, 	36	Town Manager	Exempt	\$150,106	\$187,633	\$225,:

						Effective M	1arch 5, 2025						
Grade	Position		1	2	3	4	5	6	7	8	9	10	11
	BLET	\$18.00											
	Probationary	\$55,556.55											
	40	\$26.71											
	42	\$25.44											
PO1	Police Of	ficer	\$58,334.38	\$59,792.74	\$61,287.56	\$62,819.75	\$64,390.24	\$66,000.00	\$67,650.00	\$69,341.25	\$71,074.78	\$72,851.65	\$74,672.94
		42	\$26.71	\$27.38	\$28.06	\$28.76	\$29.48	\$30.22	\$30.98	\$31.75	\$32.54	\$33.36	\$34.19
PO2	Detecti	ve	\$60,667.76	\$62,184.45	\$63,739.06	\$65,332.54	\$66,965.85	\$68,640.00	\$70,356.00	\$72,114.90	\$73,917.77	\$75,765.72	\$77,659.86
		40	\$29.17	\$29.90	\$30.64	\$31.41	\$32.20	\$33.00	\$33.83	\$34.67	\$35.54	\$36.43	\$37.34
PO3	Corpor	al	\$64,914.50	\$66,537.36	\$68,200.80	\$69,905.82	\$71,653.46	\$73,444.80	\$75,280.92	\$77,162.94	\$79,092.02	\$81,069.32	\$83,096.05
		40	\$31.21	\$31.99	\$32.79	\$33.61	\$34.45	\$35.31	\$36.19	\$37.10	\$38.03	\$38.98	\$39.95
		42	\$29.72	\$30.47	\$31.23	\$32.01	\$32.81	\$33.63	\$34.47	\$35.33	\$36.21	\$37.12	\$38.05
PO4	Sergea	nt	\$72,704.24	\$74,521.85	\$76,384.89	\$78,294.52	\$80,251.88	\$82,258.18	\$84,314.63	\$86,422.50	\$88,583.06	\$90,797.64	\$93,067.58
		40	\$34.95	\$35.83	\$36.72	\$37.64	\$38.58	\$39.55	\$40.54	\$41.55	\$42.59	\$43.65	\$44.74
		42	\$33.29	\$34.12	\$34.97	\$35.85	\$36.75	\$37.66	\$38.61	\$39.57	\$40.56	\$41.57	\$42.61
PO5	Lieutena	ant	\$82,155.79	\$84,209.69	\$86,314.93	\$88,472.80	\$90,684.62	\$92,951.74	\$95,275.53	\$97,657.42	\$100,098.86	\$102,601.33	\$105,166.3
PO6	Police Ca	otain	\$94,068.38	\$96,420.09	\$98,830.59	\$101,301.36	\$103,833.89	\$106,429.74	\$109,090.48	\$111,817.75	\$114,613.19	\$117,478.52	\$120,415.4
PO7	Police C	hief	\$122,103.62	\$125,766.72	\$129,539.73	\$133,425.92	\$137,428.69	\$141,551.56	\$145,798.10	\$150,172.05	\$154,677.21	\$159,317.52	\$164,097.0
Vew H	ire Guidelines			Promotion G	uidelines				** Police Offi	cer to Detectiv			
Step 1	No sworn experi	ence		Move to the s	tep in the new	grade that alig	gns with the		Detective to Police Officer is considered				
Step 3	Two (2) years of	experience		promotion. Fr	om Police Offi	icer to Corpora	il is one step		a lateral move		same step		
Step 5	Four (4) years of	experience		lower on the r	new grade. For	Officer to Ser	geant two		on new grade				
*	Premium pay for A	ssociates				le. Other move							
	or Bachelors degr					n how may gra							
	Intermediate or Ad					s less than the							
	Law certificate, w	ill be added		-	-	the incumbent	t will move to						
	to step pay rate			Step 1 of the	new grade.								

FIRE PAY PLAN

						Town of		ire Classifi ective 3/5/202	cation & Pa	y Plan						
							EIII	ecuve 3/3/20	20							
Position	Grade	Step 1	Step 2	Step 3	Step 4	Step 5	Step 6	Step 7	Step 8	Step 9	Step 10	Step 11	Step 12	Step 13	Step 14	Step 15
Firefighter Driver	20	\$58,317.17	\$59,775.10	\$61,269.48	\$62,801.21	\$64,371.25	\$65,980.53	\$67,630.04	\$69,320.79	\$71,053.81	\$72,830.16	\$74,650.91	\$76,517.18	\$78,430.11	\$80,390.86	\$82,400.64
IOURLY (For HR Only)		\$ 19.64	\$ 20.13	\$ 20.63	\$ 21.15	\$ 21.67	\$ 22.22	\$ 22.77	\$ 23.34	\$ 23.92	\$ 24.52	\$ 25.13	\$ 25.76	\$ 26.41	\$ 27.07	\$ 27.74
Introductory Period		fatwelve (12)	month introdu	ctopy period in	cumbents will l	ne eligible to re	ceive a one (1)	sten increase								

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Policies for Compensation 2025

	CURRENT	NEW	Grandfather in?	Handook Policy
Pay premiums				
Premiums for certs	Police sworn : One step for Intermeditate Law Enforcement Certificate, two steps for Advanced (or only one additonal step if already received credit for Intermediate)	 \$1250/yr Intermediate Law Certificate \$2500/yr Advanced Law Certificate* *if credit already given for Intermediate, only \$1250 wil be added for total \$2500 granted every year at annual review 	Yes 14 people	<i>Add: flat amount awarded once per year at annual evaluation</i>
Premiums for educ	General: nonePolicesworn: Two steps for Associates Degree, twomore or 4 total for bachleors degree	General: nonePolice sworn: 5% for Associates10%for Bachelors**ifcredit already given for Associates, only 5% for atotal of 10%	N/A	<i>in each paycheck</i> Add: Sworn officers will receive a premium for Associates or Bachelors degrees completed added to their Grade/Step pay rate.
Premiums for language	5% for general employees, 2 steps for Police Officers	5% premium for one language only, 5% premium for Police Officers	N/A	in each paycheck
Promotion Practices	includes 6 month introductory period			
General	5% to 10% unless moving greater than 2 grades considered on an individual basis	5% to 10% unless moving more than 2 grades considered on an individual basis, and consistent with internal equity** shoud genreally be at 5% increase for each grade movement.	N/A	NEW EMPLOYEE INTRODUCTORY PAY INCREASES p12 Delete N/A. Add: Promotion evaluation at 6 months without pay increase. Annual eval date is next Aug
Police	grade. At the completion of the introductory	Move to the new grade one step lower for Officer to Corporal, 1 step lower for Officer to Sergeant. Other moves follow similiar pattern dependent on how many grades they are moving up. If the increase is less than the minimum salary for the new position, the incumbent will move to Step 1 of the new grade. *Police Officer to Detective and Detective to Police Officer considered Lateral move. Move to same Step on new Grade. **A six-month introductory period will apply to promotions. A performance evaluation will be conducted at six months in the new positon, but no pay increase is granted.	N/A	EFFECTS ON SALARY RATE p 12 Promotion Evaluation conducted at 6 months without pay increase. Annual eval date is one year from sworn in date with elgibility to move one step with acceptable performance review score.
Fire	At the completion of the introductory period in the new position, he/she is eligible for another one step increase (for a total max increase of 10% at the conclusion of the introductory process).	**A six-month introductory period will apply to promotions. A performance evaluation will be conducted at six months in the new positon, but no pay increase is granted.	N/A	Annual evaluation will be conducted at 12 months with eligible for a one-step increase with acceptable evaluation score.

Item 10.

iring Guidelines				
General	Approx 2.5% per year of prior directly relevant experience up to the mid-point	Approx 2.5% per year of prior directly relevant experience up to the mid-point. Above mid-point will need justification and prior approval by Town Manager	N/A	New: Add to hiring practices. Intial 6 month evaultion without a pay increase.
Police	Step 1 No experience &/or no Degree Step 3 Two (2) years of experience &/or Associate Degree Step 5 Four (4) years of experience &/or Bachelor's Degree Five(5)+ years of experience evaluated on a case-by-case basis	Step 1 No sworn experience &/or no Degree Step3 Two (2) years of experienceStep 5Four (4) years of experienceFive(5)+ years of experience evaluated on a case-by-case basis. Posesses an education degreeand/or Law Enforcement Certificate=additionalpremium pay.	N/A	Anniversary date for evals is 12 months from sworn in date. No eval at 6 months.
	none	Probationary Officer: (new) Less than one year sworn exper. completing initial Field Training usually for 4 to 6 months. During this time they are accompanied by and are receiving field training from experienced trainers in order to become an independent Police Officer.	N/A	Pay rate is 5% below Step 1 Police Officer. At completion of FTO will promote to Police Officer Step 1. Anniversary date for evals is 12 months from sworn in date. No eval at 6 months.
Fire	Start at Step 1, Upon successful completion of a six (6) month introductory period, incumbents will be eligible to receive a one (1) step increase	Start at Step 1, Upon successful completion of a twelve 12) month introductory period, incumbents will be eligible to receive a one (1) step increase with acceptable performace review score	N/A	At bottom of published Fire pay plan

Item 10.

PINEVILLE'S EMPLOYEE HANDBOOK

COMPENSATION

ADMINISTRATION AND MAINTENANCE

The Town Manager, assisted by the Human Resources Director, shall be responsible for the administration and the maintenance of the pay plan. All employees covered by the pay shall be paid at a rate within the salary range established for the respective position classification, with the exception of employees in trainee status or employees whose existing salaries are above the maximum rate established for their respective classes when transitioning to a new salary plan. Those employees shall have their salaries maintained at that salary level with no increases until such time as the employees' salary range (in the new salary plan) is increased above the employees' current salary.

The pay plan is intended to provide equitable compensation for all positions, reflecting differences in the duties and responsibilities, the comparable rates of pay for positions in the public sector, changes in cost of living, and the financial conditions of the Town along with other factors. Periodically, at the direction of the Town Manager, the Human Resources Director will contract with an outside source to conduct comparative compensation studies of all factors affecting the level of salary ranges and make minor adjustments in the positions to salary grades as deemed necessary. When major adjustments affect a number of positions or a general adjustment is needed to re-align the salary structure, the Town Manager shall recommend such changes to the Town Council for consideration.

STARTING SALARIES

Persons hired for a position approved in the position classification plan, shall be compensated at up to the midpoint of the salary range based on their experience within the respective classification in which they are employed; however on the recommendation of the Department Head and the Human Resources Director and the written approval of the Town Manager, exceptionally well qualified applicants may be employed above the midpoint of the established minimum salary.

TRAINEE/APPRENTICE DESIGNATION

An applicant or current employee will be classified as "TRAINEE/APPRENTICE" status if the applicant shows promise of success in a position, but does not meet all of the requirements for the position under consideration; or Town employees, who do not meet all of the requirements for a position. A training plan must be established for employees in trainee/apprentice status by their Department Head to ensure they successfully meet all the requirements of the position in a reasonable period of time. An employee in trainee/apprentice status shall be regarded as an introductory employee and compensation shall not be more than ten percent below the minimum salary established for the position for which the employee is being trained.

If the trainee/apprentice does not successfully complete the requirements by the determined date they may be transferred, demoted, or dismissed. If the trainee has successfully completed all the requirements, the employee shall be paid at least the minimum rate established for the position following the trainee period.

Pineville TOWN OF PINEVILLE'S EMPLOYEE HANDBOOK

PROMOTION EFFECTS ON SALARY RATE

Promotions: Employees shall receive a pay increase to recognize and compensate the employee for their knowledge, skills and ability and their increased responsibility and duties. With the exception of Sworn Officers and Firefighters, when there is opportunity for promotion, the pay increase is a minimum of 5% or the minimum of that pay grade, whichever is greater. The increase should be between five(5) and 10% for most promotions, based on a 5% increase per pay grade increase, and consistent with internal equity. All increases must be approved by Human Resources and Town Manager.

Promoted employees will complete a promotion introductory period of 6 months, but no pay increase is due at this time. They will be included in the next annual review date. For Sworn Officers, the employee's new anniversary date will be one year from the date of promotion. For Sworn Officers and Firefighters, the incumbent will move to the step in the new grade that aligns with the promotion. From Police Officer to Corporal is one step lower on the new grade. For Officer to Sergeant two steps lower on the new grade. Other moves follow a similar pattern dependent on how may grades they are moving up. If the increase is less than the minimum salary for the new position, the incumbent will move to Step 1 of the new grade. In no event, however, shall the new salary exceed the maximum rate of the new salary range.

Police Officer to Detective and Detective to Police Officer considered Lateral move. Move to same Step on new Grade.

Demotions: Demotion is the movement of an employee from one position to a position in a class assigned to a lower salary range. An employee being demoted, must show promise of performing satisfactorily in another position. When an employee is demoted to a position for which they are qualified for other than for disciplinary or performance-based reasons, the employee will be placed at a pay-rate within the new pay grade based on knowledge, skills and ability as related to the new position and equitable to others within the same classification. If the current salary is within the new range, the employee's salary may be retained at the previous rate if appropriate. If the demotion is the result of discipline or the demotion is from a position with supervisory or leadership responsibilities, the salary shall be decreased at minimum 5% or to the midpoint of the new range and no merit increase will be granted for twelve months. In no event, however, shall the new salary exceed the maximum rate of the new salary range.

Transfers: The salary of an employee reassigned to a position in the same class or to a position in a different class within the same salary range shall not be changed as a result of the transfer.

Reclassification: The Town Manager may reclassify an employee's position to a higher salary range if there is a significant change in the knowledge, skills, abilities, and responsibilities required. The employee may be eligible for a salary increase commensurate with the reclassification if recommended by the Department Head and the Human Resources Director and approved by the Town Manager.

If the Town Manager determines the position shall be reclassified to a lower salary range, the employee's salary shall remain the same. The only exception where an employee's actual salary will be affected as a result of a lower classification is if the employee's salary is above the maximum salary for the new range, the employee shall have their salary adjusted to the maximum salary of the new salary range.

Town of Pineville Classification & Pay Plan Effective March 5, 2025

		i 5, 2025			
Grade	Project Title	FLSA Status	Min	Mid	Max
14	Customer Service Representative	NE	\$43,285	\$49,778	\$56,271
14	Maintenance Technician	NE	\$43,285	\$49,778	\$56,271
14	Park Maintenance Technician I	NE	\$43,285	\$49,778	\$56,271
14	Storm Water Technician	NE	\$43,285	\$49,778	\$56,271
14	911 Telecommunicator	NE	\$43,285	\$49,778	\$56,271
14	Admin Assistant/Receptionist	NE	\$43,285	\$49,778	\$56,271
			¢ :0,200	<i>Q</i> 10,110	\$00 <u>,</u> 211
15	Administrative Assistant	NE	\$45,450	\$52,267	\$59,085
15	Equipment Operator	NE	\$45,450	\$52,267	\$59,085
15	Senior Customer Service Representative	NE	\$45,450	\$52,267	\$59,085
15	Senior Storm Water Technician	NE	\$45,450	\$52,267	\$59,085
15	Parks Maintenance Technician II	NE	\$45,450	\$52,267	\$59,085
15	911 Telecommunicator II	NE	\$45,450	\$52,267	\$59,085
10			φ10,100	φ02,207	<i>\</i> 000,000
16	Accounting Technician II	NE	\$47,722	\$54,880	\$62,039
16	Administrative Technician	NE	\$47,722	\$54,880	\$62,039
16	Billing & Collections Coordinator	NE	\$47,722	\$54,880	\$62,039
16	Property & Evidence Technician	NE	\$47,722	\$54,880	\$62,039
	Senior Parks Maintenance Technician				
16		NE	\$47,722	\$54,880	\$62,039
<u>16</u>	Building Maintenance Technician 911 Telecommunicator III	NE NE	\$47,722	\$54,880	\$62,039
16	911 Telecommunicator III	NE	\$47,722	\$54,880	\$62,039
			A =0.400	A 57.004	*•••••••••••••
17	Human Resource Assistant	NE	\$50,108	\$57,624	\$65,140
17	Fleet Manager	NE	\$50,108	\$57,624	\$65,140
17	Code Enforcement Officer	NE	\$50,108	\$57,624	\$65,140
17	911 Senior Telecommunicator	NE	\$50,108	\$57,624	\$65,140
18	Athletic Coordinator	NE	\$50,421	\$60,505	\$70,589
18	Programs/Events Coordinator	NE	\$50,421	\$60,505	\$70,589
18	Special Events Coordinator	NE	\$50,421	\$60,505	\$70,589
18	Systems Technician	NE	\$50,421	\$60,505	\$70,589
20	Assistant Telecommunications Supervisor	NE	\$55,589	\$66,707	\$77,825
20	Community Outreach Specialist	NE	\$55,589	\$66,707	\$77,825
20	Parks Maintenance Supervisor	NE	\$55,589	\$66,707	\$77,825
20	Public Works Supervisor	NE	\$55,589	\$66,707	\$77,825
20	Senior Systems Technician/Assistant Supervisor	NE	\$55,589	\$66,707	\$77,825
21	Building Maintenance Supervisor	NE	\$58,368	\$70,042	\$81,715
21	Crime Analyst	NE	\$58,368	\$70,042	\$81,715
21	Records & Accreditation Manager	NE	\$58,368	\$70,042	\$81,715
22	Community Relations & Communications Specialist	NE	\$61,287	\$73,544	\$85,802
22	Accountant	Exempt	\$61,287	\$73,544	\$85,802
			¥ - 1 -	+ - / -	<i>+)</i>
23	Central Office Database Technician	NE	\$64,351	\$77,221	\$90,091
23	Network Database Technician	NE	\$64,351	\$77,221	\$90,091
23	Systems Technician Supervisor	NE	\$64,351	\$77,221	\$90,091
			ψο 1,00 1	Ψ''', ΖΖ'	ψου,ουτ
24	Telecommunications Supervisor 911	Exempt	\$67,568	\$81,082	\$94,595
		Exempt	<i>401,000</i>	\$01,00Z	<i>40</i> 1,000
25	Town Clerk	Exempt	\$70,947	\$85,136	\$99,326
			* - / -	+,	+ ,
	Human Resource Director	Exempt	\$94,593	\$118,241	\$141,890
30		Exempt	\$94,593	\$118,241	\$141,890
30 30	Parks & Recreation Director				
30			\$94,593	\$118.241	\$141.890
	Parks & Recreation Director Public Works Director	Exempt	\$94,593	\$118,241	\$141,890
30 30	Public Works Director	Exempt	• •		
30 30 31	Public Works Director Finance Director	Exempt Exempt	\$102,160	\$127,700	\$153,240
30 30 31 31	Public Works Director Finance Director Planning Director	Exempt Exempt Exempt	\$102,160 \$102,160	\$127,700 \$127,700	\$153,240 \$153,240
30 30 31	Public Works Director Finance Director	Exempt Exempt	\$102,160	\$127,700	\$153,240 \$153,240
30 30 31 31 31 31	Public Works Director Finance Director Planning Director Telephone/Utility Director	Exempt Exempt Exempt Exempt	\$102,160 \$102,160 \$102,160	\$127,700 \$127,700 \$127,700	\$153,240 \$153,240 \$153,240
30 30 31 31	Public Works Director Finance Director Planning Director	Exempt Exempt Exempt	\$102,160 \$102,160	\$127,700 \$127,700	\$141,890 \$153,240 \$153,240 \$153,240 \$178,739

Town of Pineville Police Classification & Step Pay Plan Effective March 5, 2025

Grade	Position		1	2	3	4	5	6	7	8	9	10	11
	BLET	\$18.00											
	Probationary	\$55,417.66											
	40	\$26.64											
	42	\$25.37											
PO1	Police Of	ficer	\$58,334.38	\$59,792.74	\$61,287.56	\$62,819.75	\$64,390.24	\$66,000.00	\$67,650.00	\$69,341.25	\$71,074.78	\$72,851.65	\$74,672.94
		42	\$26.71	\$27.38	\$28.06	\$28.76	\$29.48	\$30.22	\$30.98	\$31.75	\$32.54	\$33.36	\$34.19
PO2	Detecti	ve	\$60,667.76	\$62,184.45	\$63,739.06	\$65,332.54	\$66,965.85	\$68,640.00	\$70,356.00	\$72,114.90	\$73,917.77	\$75,765.72	\$77,659.86
		40	\$29.17	\$29.90	\$30.64	\$31.41	\$32.20	\$33.00	\$33.83	\$34.67	\$35.54	\$36.43	\$37.34
PO3	Corpor	al	\$64,914.50	\$66,537.36	\$68,200.80	\$69,905.82	\$71,653.46	\$73,444.80	\$75,280.92	\$77,162.94	\$79,092.02	\$81,069.32	\$83,096.05
		40	\$31.21	\$31.99	\$32.79	\$33.61	\$34.45	\$35.31	\$36.19	\$37.10	\$38.03	\$38.98	\$39.95
		42	\$29.72	\$30.47	\$31.23	\$32.01	\$32.81	\$33.63	\$34.47	\$35.33	\$36.21	\$37.12	\$38.05
PO4	Sergea	ant	\$72,704.24	\$74,521.85	\$76,384.89	\$78,294.52	\$80,251.88	\$82,258.18	\$84,314.63	\$86,422.50	\$88,583.06	\$90,797.64	\$93,067.58
		40	\$34.95	\$35.83	\$36.72	\$37.64	\$38.58	\$39.55	\$40.54	\$41.55	\$42.59	\$43.65	\$44.74
		42	\$33.29	\$34.12	\$34.97	\$35.85	\$36.75	\$37.66	\$38.61	\$39.57	\$40.56	\$41.57	\$42.61
PO5	Lieuten	ant	\$82,155.79	\$84,209.69	\$86,314.93	\$88,472.80	\$90,684.62	\$92,951.74	\$95,275.53	\$97,657.42	\$100,098.86	\$102,601.33	\$105,166.36
PO6	Police Ca	ptain	\$94,068.38	\$96,420.09	\$98,830.59	\$101,301.36	\$103,833.89	\$106,429.74	\$109,090.48	\$111,817.75	\$114,613.19	\$117,478.52	\$120,415.48
PO7	Police C	hief	\$122,103.62	\$125,766.72	\$129,539.73	\$133,425.92	\$137,428.69	\$141,551.56	\$145,798.10	\$150,172.05	\$154,677.21	\$159,317.52	\$164,097.05

Add Policies - Hiring and promotion

Town of Pineville Fire Classification & Pay Plan Effective 3/5/2025

Position	Grade	Step	1	Step 2	Step 3	Step 4	Step 5	Step 6	Step 7	Step 8	Step 9	Step 10	Step 11	Step 12	Step 13	Step 14	Step 15
Firefighter Driver	20	\$ 58,31	7.17	\$ 59,775.10	\$ 61,269.48	\$ 62,801.21	\$ 64,371.2	5 \$ 65,980.53	\$ \$ 67,630.04	\$ 69,320.79	\$ 71,053.81	\$ 72,830.16	\$ 74,650.91	\$ 76,517.18	\$ 78,430.11	\$ 80,390.86	\$ 82,400.64
HOURLY (For HR Only)		\$ 1	9.64	\$ 20.13	\$ 20.63	3 \$ 21.15	5\$ 21.6	7 \$ 22.22	22.77 \$	\$ 23.34	\$ 23.92	\$ 24.52	\$ 25.13	\$ 25.76	\$ 26.41	\$ 27.07	\$ 27.74
												_					
Introductory Period																	
Upon successful comp	oletion of	a twelve (12) mo	onth introduct	ory period, inc	umbents will be	eligible to rec	eive a one (1) s	tep increase								



MEETING DATE: February 11, 2025

Agenda - Title/Category:	Awa	rd Pu	rchase o	of Underground		
	Cabl	es for	Miller F	arm Subdivision		
Staff Contact/Presenter:	Davi	d Luc	ore			
Meets Strategic Initiative or	Yes	No	If yes,	System Expansion		
Approved Plan:	X		list:			
Background:	unde	ergrou		solicited for the es needed for the vision.		
Discussion:	Four companies submitted bids with the lowest cost bid supplied by Border States.					
Fiscal impact:	\$212	2,640.	00			
Attachments:	Form	nal Bio	d Summ	ary		
Recommended Motion to be made by Council:	unde	ergrou	he purc und cabl livision.	hase of es for the Miller		

	l Bid Summary Town of Pineville, NC									Iter
Under	ground Distribution Cable	Bid Opening:		1/28/2025 @	2:00 PM					
		Location:		505 Main St,	Pineville, NC	28134				
Bid Op	bening Location:			505 Main Str	eet, Pineville,	, NC 28134				
	Item	American Wire	Group	WESCO		Border	States Kerite	Border State	es Prysmian	
1	15 kV, 750 AL, STR, JCN, 1/3 N, 220 mil								-	
	Cost/Foot	NO BID		\$	13.21	\$	11.18	\$	11.	08
	QTY				9,000		15,000		10,0	00
	Total Cost			\$	118,890	\$	167,700	\$	110,8	00
	Delivery			20-22 Week	S	20 Wee	ks	20 Weeks		
2	15 kV, 1/0 AL, SOL, JCN, Full N 220 Mil									
	Cost/Foot	NO BID		\$	3.92		3.62		4	.26
	QTY				19,500		20,000		20,0	00
	Total Cost			\$	76,440.00	\$	72,400.00	\$	85,200.	00
	Delivery			1-2 Weeks		10-12 W	/eeks	20 Weeks		
3	600V, CONVERSE, 2/0 AL TPX, URD									
	Cost/Foot		1.32		1.38		1.28		1	.28
	QTY		22500		23000		23000		230	000
	Total Cost	\$	29,700.00	\$	31,740.00	\$	29,440.00	\$	29,440.	00
	Delivery	1 Week		1-2 Weeks		STOCK		STOCK		
4	600V, WESLEYAN, 350 AL TPX, URD									
	Cost/Foot		3.1		2.99		3.07		3	8.07
	QTY		10000		10000		10000		100	000
	Total Cost	\$	31,000.00	\$	29,900.00	\$	30,700.00	\$	30,700.	00
	Delivery	1 Week		1-2 Weeks		STOCK		STOCK		
		Border States						\$	212,640.	00
		WESCO		\$	29,900.00					





PUBLIC WORKS

To: Town CouncilFrom: Chip HillDate: February 1, 2025Re: Public Works Updates

Johnston Drive Alignment: The contractor has worked primarily on storm water drainage systems in January in phase 2. Three masonry drainage structures were completed in January, with the connecting concrete pipe run between them. The contractor plans to continue working on the drainage structures outside of the roadway until February 17th, when the road closure will allow the water & drainage work in the roadway. Town staff is working with Charlotte Water personnel to limit disturbances of service to business and residences in the vicinity. The Town engineer is working on the final draft of the detour map. When it is completed and approved it will be posted on the Town website.

Main Street Crosswalks: The Town staff had a meeting with town engineers regarding cabinet placement and directional drilling alignment. Some changes are needed. Constructional documents revisions to NCDOT are ongoing. A request was made for the soft digs cost for locating underground utilities.

Sidewalks on S Polk: A total of 40cy have been placed in total for sidewalk. Precast storm water structures have been delivered and are in the process of being installed.

Huntley Glen: Most interior work is completed. True Homes was directed to complete design for the front sidewalk running adjacent to Dorman Road and submit to Mecklenburg County.

Huntley Glen Townhouses: Video was completed. We are waiting for the engineer's storm drain assessment report.

Parkway Crossing: Pipe repairs are underway and could last 6 months due to the amount of pipe repairs that are needed. We may start looking at phasing external markups and repairs as we get closer to spring.

Preston Park: Prefinal complete. Developer instructed they will need to petition town separately for the alleys to be taken over. All ADA ramps will need to be compliant with the most up to date ramp standards.

McCullough: The developer is organizing the storm drain video footage to submit to the Town and county in the appropriate form.

Miller Farms: Verified and confirmed that densities were taken on pipe install. Densities are good and pipe install will continue. This project is on track and progressing as a normal project. The developer is coming with an engineered soils profile, due to the moisture content of the soils. This will have to be reviewed and approved prior to implementation.

Chadwick Park: Video has been approved. As-Builts approved. The prefinal walk has been completed and the site marked up. ADA ramps will need to meet the latest requirements. Alleys have been deemed to be taken over by the Town so those will need to be installed per the plan, with repairs. Developer will need to petition the Town for those, in person, and separate from the roadways. They can start making repairs to all items on the prefinal, just no top lift of asphalt.

Coventry: Pipe certification letter and report sent. Still waiting on the roadway certification. They are working on the water line and NCDOT entrances.

Carolina Logistics Park: As-builts are approved. Working on getting the final video in correct format. Once that is all approved Prefinal will be done. The section of sidewalk along Downs Road is not complete.

*see attached spreadsheet for permits pending/issued

PERMITS ISSUED/PENDING COMPANY Fiscal Year 2025

Ashley Northup/AT&T Zach Pellicone/Charlotte Water Paul Tatsis/PNG Ashley Northup/AT&T O'brien Walls/Charlotte Water AT&T/SourceOne/Rosita Villavicencio AT&T/Ashley Northup Charlotte Water/Samuel Yuhas Charlotte Water/Samuel Yuhas AT&T/SourceOne/Rosita Villavicencio Spectrum/STS Cable Services/Tracey Kendall AT&T/SourceOne/Rosita Villavicencio Spectrum/ACP Fiber Services/Ryan McCumber Charlotte Water/Cirilo Saba

LOCATION

625 Eagleton Downs
10112 Industrial Drive
307 College Street
10810 Park Crossing Dr
109 N Polk Street
12026 Carolina Logistics Drive
10901 Downs Rd
10496 Park Road
12031 Lancaster Hwy/Carolina Place
12020 Carolina Logistics Drive
11925 Carolina Logistics Drive
10230 Pineville Distribution/Industrial Dr
Miller Road
265 Eden Circle/Cone Avenue

STATUS PERMIT NO

Issued

Pending

PW20240812EAGLETONDOWNS625 PW20240807INDUSTRIAL10112 PW20240729COLLEGE307 PW20240806PARKCROSSING10810 PW20240731NPOLK109 PW20241011CAROLINALOGISTICS12026 Canceled PW20241008PARKROAD10496 PW20241010LANCASTERHWY12031

PW20241017CAROLINALOGISTICS12020 PW20241024CAROLINALOGISTICS11925 PW20250130PINEVILLEDISTRIBUTIONST10230 PW20250115MILLER



Human Resources

Linda Gaddy, PHR SHRM-CP MSHR lgaddy@pinevillenc.gov (704) 889-2362

 To:
 Ryan Spitzer, Town Manager

 Members of the Town Council

From: Linda Gaddy

Date: 2/5/2025

Re: Human Resources Monthly Report

Ryan,

Enclosed is the Human Resources Department Monthly Report for the month of January 2025.

<u>New Hires:</u> Logan Hulst, rehire Police Officer

<u>Resignation/Termination:</u> Elginn Britt, Police Officer

Retirements: none

Transfers: none

Promotions: none

<u>Current Openings:</u> Police Officer: 2 openings for lateral hires B.L.E.T. trainees, 2 sponsored currently 911 Telecommunicator, 1 opening, accepting applications, reviewing

Departmental Update:

Employee Appreciation and events:

Another round of submissions deemed eligible for "Caught in the Act" awards were recognized in the newsletter and awarded certificates and a lunch voucher. A committee screened entries for those staff recognized by their peers for going above and beyond or for giving exemplary service to each other or to the public.

Safety:

Three minor incidents to report this month due to both preventable and non-preventable normal police officer operations, most resulted in minor property damage.

Annual online safety training courses have been assigned to all employees to be completed in February, along with annual anti-harassment prevention training.

Annual First Aid/CPR/AED class is scheduled and live trainings on safe equipment operation are coming up in March/April.

Recruiting:

We are still seeking experienced Police Officers, B.L.E.T. police trainees, and one more 911 Telecommunicator now that we have a trainer available again. All other departments are fully staffed.

Police Promotions:

The promotional process for Police Corporal is almost completed. Three candidates applied for two open positions (one for later in the year). Assessment center exercises were completed in January. Chief's interviews are in progress. Candidates that score acceptable and are recommended for promotion by their current supervisor will be ready for promotion at any time in the next year should an additional position for a Corporal come open.

Compensation Study:

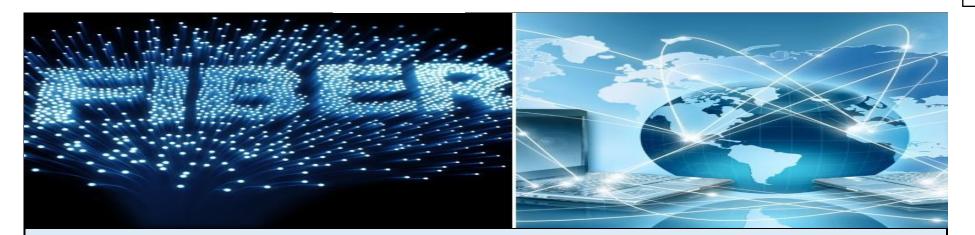
Baker Tilly consultants completed a compensation study for the entire Town. The Baker Tilly team has presented their final report, and recommenations to leaders and to Council. They have aslo presented the final report to employees. The recommended scenarios and options have been approved and have moved to the planning and execution stage. Policies and procedures surronding the changes to compensation plans have been evaluated for any needed revisions and are ready to present to managers. Final classification and pay plans have been reviewed again internally and will be published very soon. Internal staff have been trained in maintenance of the pay structures. New pay plans are effective March 5, 2025.

Performance Management:

Supervisors are conducting mid-year performance check-ins with their staff reviewing progress toward goals and performance for the first half of the year.



	PINEV	ILLE	COMM	UNIC	CATIO	N SYST	'EMS			
	1	NTERNET I	RESULTS FO	R MON	TH ENDING	<i>6 01-31-2025</i>				
REVENUE AREA	MONTH ENDING 11- 30-2024	INSTALLS SOLD AND COMPLETED IN DEC.	DISCONNECTS TAKEN AND EXECUTED IN DEC.	MONTH ENDING 12- 31-2024	INSTALLS SOLD AND COMPLETED IN JAN.	DISCONNECTS TAKEN AND EXECUTED IN JAN.	MONTH ENDING 01- 31-2025	SOLD IN JAN ON SCHEDULE FOR INSTALLATION AFTER BILLING OR IN FEB	TOTAL INTERNET FOR MONTH ENDING 01-31-2025	TOTAL AS OF 01-31- 2025
ILEC	593	4	-5	592	7	-11	588	0	588	588
CLEC	637	7	-8	636	10	-7	639	0	639	639
TOTAL	1230	11	-13	1228	17	-18	1227	0	1227	1227



100M to 1 GIG SPEED OFFERING TAKE RATE TO DATE

INTERNET							
RESULTS FOR MONTH ENDING 01-31-			SERVICE	RES OR		NET GROWTH/LOSS FROM	
2025	Dec-24	Jar	-25 AREA	BUS	SPEED	PREVIOUS MONTH	
	216	214	CLEC	RES	300M		
	85	88	CLEC	RES	600M		
	202	200	CLEC	RES	1 GIG		
	6	7	CLEC	BUS	100M	1	
	8	8	CLEC	BUS	1 GIG		
	3	3	CLEC	BUS	200M	0	
	3	4	CLEC	BUS	400M		
	33	32	ILEC	BUS	100M	-1	
	5	5	ILEC	BUS	200M	0	
	8	7	ILEC	BUS	400M	-1	
	26	26	ILEC	BUS	1 GIG	0	
	139	136	ILEC	RES	1 GIG	-3	
	235	233	ILEC	RES	300M	-2	
	49	52	ILEC	RES	600M	3	
TOTAL	1018	1015				-3	
1227							
82% of our Internet subscribers now su	ubscriber to 1001	M or higher					



LINE COUNT AS OF 01-31-2025

LINE COUNT	LINE COUNT MONTH	LINE COUNT MONTH	
FOR MONTH ENDING 01-31-2025	ENDING 12-31-2024	ENDING 01-25-2025	
BUS	36	36	0
RES	85	83	-2
CLEC SUBTOTAL	121	119	-2
	LINE COUNT MONTH	LINE COUNT MONTH	
ILEC LINE COUNT	ENDING 12-31-2024	ENDING 01-25-2025	
BUS	285	283	-2
RES	102	99	-3
	387	382	-5
COMBINED LINE COUNT	508	501	-7

January 2025

Our Senior Field Trip visited the NC Music Hall of Fame in Kannapolis on January 21st. 10 seniors enjoyed a morning learning about NC music. Youth basketball continued practice and games in January. Pineville Elementary and the Belle Johnston Center are filled on Saturdays with boys and girls shooting, dribbling, passing and competing. It is a true joy to watch these children have fun and enjoy the game of basketball. Shi had a fun game day with our Senior Nutrition program. Seniors engaged in multiple fun activities to lighten their day. We are preparing for our youth soccer and baseball season at Jack Hughes. The maintenance team is preparing our fields to be in great shape for all who participate. We are excited that our newest Pineville program, girls youth volleyball will begin in March. 39, hopefully lucky winners enjoyed Bingo Night on Friday, January 17th. We also had a winter Grab n' Go for kids for kids in January – 86 kids received a goodie bag 💬





January 2025





Item 12.

January 2025

General Programming – Belle Johnston

Pickleball: Open Pickleball times are Mondays and Friday from 9am-12pm and Wednesdays from 1:30pm-4:30pm. 50 participants

Asap Pickleball – Thursday at 9am – 16 participants

Karate: They hold classes on Wednesdays. 25 participants

Pre School Open Gym – Wednesday morning from 9a – 12p – 15 participants

National Pie Day – January 23 - 67 participants

Fitness Dance – Wednesdays at 3pm - 15 participants

Paint Class – January 13 - 9 participants

Sound Bath Meditation Class — 9 participants

Field Trip – January 21 - NC Music Hall of Fame – January 18 – 10 participants

Family Bingo Night – January 17 – 39 participants

January Winter Grab n Go – January 15 – 82 participants

Senior Game Day – January 24th – 25 participants

Youth Basketball – January – 245 participants

Cookie Decorating Class – January 23 – 9 participants

Lake Park

Bootcamp with Lia – Bootcamp meets 5:45am – 6:45am M/W/F in Lake Park. 60 participated

Tai Chi – Saturdays – 11 participants

The Hut

Senior Fit – Senior Fit takes place at the Hut M – Thursdays. 192 participants

Yoga – 102 participants

Cardio Funk: Lem holds class on Tuesdays at 6:30pm. 12 participants

January 2025

Facility Rentals

Facility Rentals

The Hut: 1 Rentals The BJCC Dining Room: 3 Rentals The BJCC Gym: 0 rental Large Shelter: 0 Rentals Medium Shelter: 1 Rentals Tot Lot at Lake Park: 0 Rentals Shelter 1 at JH: 0 Rentals Shelter 2 at JH: 0 Rentals

Jack Hughes

Jack Hughes Special Events

No tournaments or events in January.

Baseball Field Usage

No field usage in January.

Multipurpose Field Usage

No field usage in January.

Park Maintenance Update

Lake Park/Belle

Daily park check Monthly building inspection Remove Christmas decorations | Blow walkways, shelters and playgrounds as needed

Hut

Cut as needed Trash removal as needed Monthly building inspection

January 2025

Cemetery

Blew leaves as needed Limb removal as needed Daily check

Dog park

Removed limbs as needed Blew entrance as needed Cut up fallen tree

Jack Hughes

Daily park check Monthly building inspections Cut fields as needed Safety classes Monroe CEU training for spray license Equipment maintenance as needed Cut out base lines on sod field 2 Applied turface fields 1,2,4 Put out fertilizer all 4 fields Repaired L screens all fields Pressure washed stadium backstop and dug outs Pressure washed field 1 dug outs Repaired foul pole field 2

Town Hall/PD

Removed Christmas tree Daily check and trash removal Fountain deicing Repaired solenoid on fountain

Social Media

Facebook

Post Reach: 3,867	Post Engagements: 181	
New Page Likes: +23	Total Page Likes: 4,998	Total Page Followers: 6,178

Instagram

New Followers: +29

Total Followers: 3,020

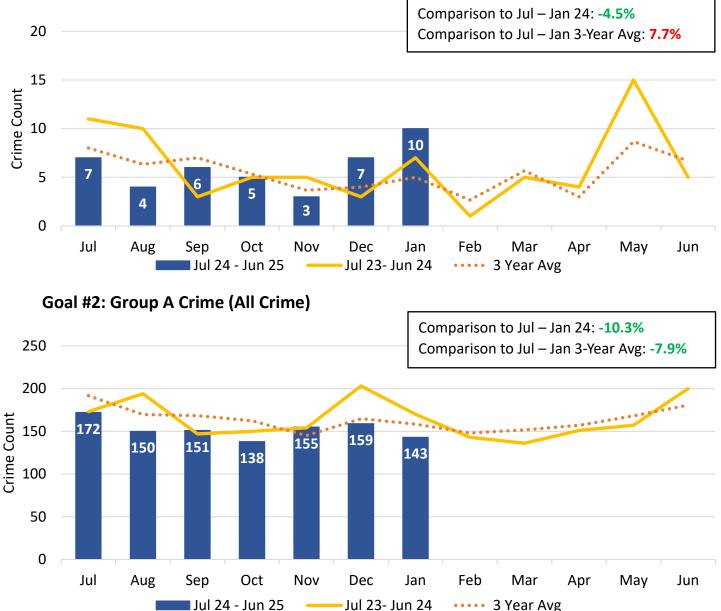


PINEVILLE POLICE DEPARTMENT

MONTHLY REPORT January 2025

Crime Goals

Below is the evaluation of the police department's crime goals. Goals are measured for 12 months based on the fiscal year. For the year of July 2024 – June 2025, the goal is to reduce violent crime and reduce all crime by 5%.



Goal #1: Violent Crime

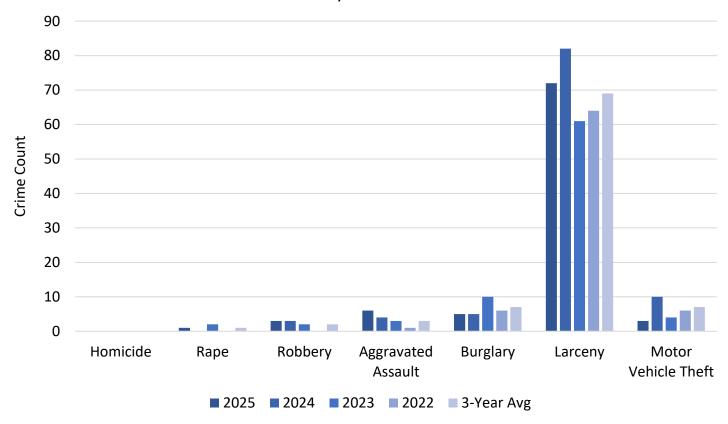
* data is subject to change; unfounded removed

Monthly Crime Statistics

Below is a table and bar graph of the counts for Part 1 Crimes in January. For comparison, the same is shown for the past 3 years. The average of the 3 years was calculated.

	January Crime Statistics Part 1 Offenses											
	2025	2024	2023	2022	3-Year Average (2022-2024)	ETJ						
Homicide	0	0	0	0	0	0						
Rape	1	0	2	0	1	0						
Robbery	3	3	2	0	2	0						
Aggravated Assault	6	4	3	1	3	2						
Burglary	5	5	10	6	7	0						
Larceny	72	82	61	64	69	1						
Motor Vehicle Theft	3	10	4	6	7	0						

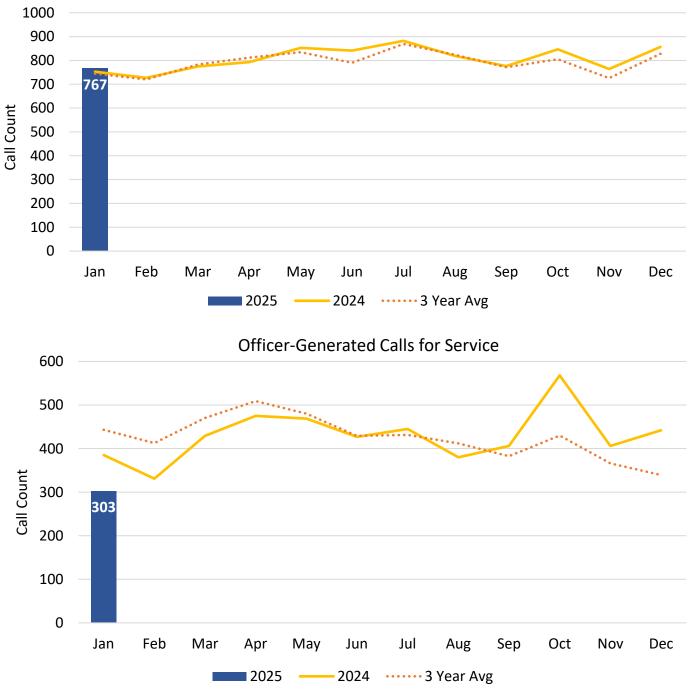
* ETJ statistics included in total number of offenses



January Part 1 Crimes

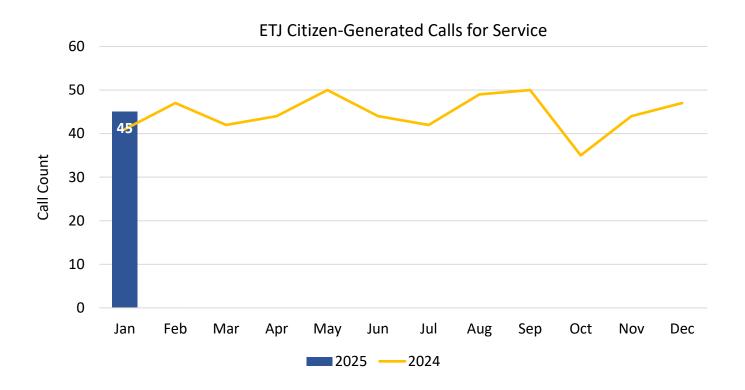
Calls for Service

The graphs below display the number of calls for service in comparison to previous months, year, and 3-year average. The first graph is citizen-generated calls. The second graph is officer-generated calls. The third graph is the ETJ.



Citizen-Generated Calls for Service

*zone checks and foot patrols removed



January Traffic Enforcement

Traffic Enforcement Type and Dispositions

Enforcement	Count
Traffic Stop	155
Citation Issued	47
Warning	86
Report Taken	4

*Officer-generated traffic stops; unfounded removed

Locations of Traffic Enforcement

Street Name	Count
PINEVILLE-MATTHEWS RD	36
MAIN ST	30
POLK ST	17
CAROLINA PLACE PKY	16
PARK RD	9
TOWNE CENTRE BLVD	6
ROCK HILL-PINEVILLE RD	4
JOHNSTON DR	4
LANCASTER HWY	3
DOVER ST	3

DOWNS CIR	3
COLLEGE ST	2
REID LN	2
CRANFORD DR	2
I-485 INNER HWY	2
LEE ST	2
PINEVILLE RD	2
SOUTH BLVD	2
PARK CEDAR DR	1
RODNEY ST	1
INDUSTRIAL DR	1
CHURCH ST	1
JACK HUGHES LN	1
BLUE HERON	1
FRANKLIN ST	1
LOWRY ST	1
EDEN CIR	1
CENTRUM PKY	1
Grand Total	155

*based on location of stop in CAD

January Community Engagement

- Walk about Alley 51
- In service Training
- Training on generations in reference to recruiting
- Meeting with Harrison United Methodist Church
- Town Safety Meeting
- Assisted with Cpl. Assessment
- Worked on shooting call
- Budget Meeting
- Special Olympics Conference
- Yoga event with HSS
- Harrison Church event
- PD Social Media work
- Liaison with apartment complexes, hotels and HOA's
- Car Seat Checks

Department Update



To: Town CouncilFrom: Travis MorganDate: 2/11/2025

Re: Town Planning Updates

PLANNING:

Miller Farm: First plat of lots in the works along with the traffic signal install operational in the upcoming future.

Sportsplex: Vogue Tower is interested in renewing talks for a communication tower for better signal in the area

CODE ENFORCEMENT:

Prohibited Parking:	Trash Can:	Parking on the lawn:
11715 Carolina place	11957 Stratfield	301 park lake ct
·	11837 Stratfield	130 Olive
ADU/Accessory Structures:	11825 Stratfield	918 Lakeview
1005 cone	400 Mallard	229 Eden Cir
Community Appearance/Junk Vehicle:		
123 boatwright	Unsecured Building:	Zoning Violation:
1111 Cone	403 Dover St	10724 Pineville Rd
1106 cone		
408 fisher st	Minimum Housing:	Signs:
10860 park	423 Cone	332 Cranford
100 Mallard	1108 Cone	8500 P-M
235 Eden		10222 Johnston Rd
252 Eden	POD:	9540 Rodney
236 Eden	823 Windage Wy	618 N Polk
333 Cranford		10019 Plum Creek
10860 Park Rd		136 Main
11715 carolina place		Dumpster:
1106 Cone Av		10860 Park Rd
286 Eden Cir		323 Main St
945 Pelican Bay Dr		8706 Pineville-
10520 Park Rd		Matthews
Improved Surfaces:		
409 Johnston Dr		
907 Lakeview Dr		
315 College		

Item 13.

2025

Saturday	Friday	Thursday	Wednesday	Tuesday	Monday	Sunday
Clocks forward at midnight	7	6	5 Possible Budget Meeting with Davenport Advisors	4	3	2
1	14	13	12 Possible Budget Meeting with Davenport Advisors	11 Town Council 6:30 pm	10	9
2	21	20 CATS meeting 5:30 pm Town Hall	19	18	17	16
2	28	27	26	25	24 Work Session 6:00 pm	23
					31	30

